



YUMA COUNTY BOARD OF ADJUSTMENT MEETING PUBLIC NOTICE & AGENDA

Due to COVID-19 concerns, the meeting will be conducted remotely through technological means. The intent is to reduce the number of individuals at Aldrich Hall to the greatest extent possible. Board members, staff, and applicants will discuss their zoning cases by remote technological means, principally ZOOM.

While Aldrich Hall will be open for public participation, members of the public are encouraged to:

1. Send written comments to ddsplanning@yumacountyaz.gov no later than 45 minutes prior to the start of the scheduled meeting. Comments received timely will be read into the record when the referenced agenda item is discussed.
2. Arrange to participate remotely by contacting the following email address: ddsplanning@yumacountyaz.gov.

If you do appear at Aldrich Hall, you must maintain social distancing. The podium will have a laptop connected to the meeting through ZOOM. When the item you are appearing for is opened for public comment, you may proceed to the podium and participate by ZOOM.

DATE: June 16, 2020
TIME: 1:00 P.M.
PLACE: Aldrich Auditorium, 2351 West 26th Street, Yuma, Arizona

MEMBERS: Neil Tucker, Chairman, Dist. 3
Eric Saltzer, Vice-Chairman, Dist. 1
Joe Harper, Dist. 4
Rosalie Lines, Dist. 2
Tim Eisenmann, Dist. 5

STAFF: Maggie Castro, AICP, Planning Director
Juan Leal Rubio, Senior Planner
Amber Jardine, PZ Commission Admin Specialist

ADVISORS: Ed Feheley, Deputy County Attorney
Diana Gomez, Director, Yuma County Public Health Services District

1. **Call to Order and Roll Call.**
2. **Pledge of Allegiance.**
3. **Approval of the Board of Adjustment meeting minutes from May 19, 2020.**
4. **Variance Case No. 20-10:** Robert and Judy Satchell request a variance from the development standards of Martinez Lake Resort Unit No. 1 Planned Development to increase the height to 35.5 feet on a parcel 1,620 square feet in size zoned Planned Development (PD), Assessor's Parcel Number 459-50-025, located at 10377 East Tule Road, Yuma, Arizona.
5. **Interpretation Case No. 20-01:** Barry Olsen, agent for Terry W. Cameron, et. al., requests the following interpretations for the property owned by Tony L. & Kathleen Abbott and Bryan L. Abbott, located at 10343 East North Martinez Lake Road, Assessor's Parcel Number 459-50-078, zoned Rural Area-20 acre minimum (RA-20):

- Is the Abbott's proposed "New Garage" actually a dwelling unit, or an accessory dwelling unit, under section 202.00 of the Zoning Ordinance?
- Is the Abbott's proposed "New Garage" an impermissible expansion of a non-conforming lot/use under Sections 1001.00, et. seq., of the Zoning Ordinance.
- If the Abbott's proposed "New Garage" is permissible under Sections 602 and 1001, et. seq., of the Zoning Ordinance, what is the required side yard setback for the "New Garage" per Section 1011 of the Zoning Ordinance?

6. Discussion by the Board members and Planning Director of events attended, current events, and the schedule for future Board of Adjustment meetings.

7. Adjourn.

The Committee may vote to hold an Executive Session for the purpose of obtaining legal advice from the Board's attorney on any matter listed on the agenda, pursuant to Arizona Revised Statute §38-431.03(A)(3).

Persons with disabilities may request reasonable accommodations by contacting Shannon Gunderman, ADA Coordinator at 928.373.1137 or by email at shannon.gunderman@yumacountyaz.gov. Requests should be made as early as possible to allow time to arrange the accommodation.

Yuma County Board of Adjustment

Item No. 3

The Yuma County Board of Adjustment met in a regular session on May 19, 2020. The meeting was held via Zoom and opened to the public in Aldrich Auditorium at 2351 West 26th Street, Yuma, Arizona.

CALL TO ORDER: At 1:00 p.m., Chairman convened the Board of Adjustment meeting. Board Members present: Tim Eisenmann, Eric Saltzer, Neil Tucker and Rosalie Lines. Board Member Joe Harper was absent. Others present: Planning Director Maggie Castro, AICP; Senior Planner Fernando Villegas; Senior Planner Javier Barraza; Deputy County Attorney Ed Feheley; and PZ Commission Admin Specialist Amber Jardine.

ITEM No. 3: Approval of the Board of Adjustment regular meeting minutes of April 21, 2020.

MOTION (EISENMANN/SALTZER): Approve as presented.

ROLL CALL VOTE: Eisenmann – AYE; Tucker - AYE; Saltzer – AYE.
The motion carried 3-0.

Board Member Rosalie Lines entered the meeting at 1:07 p.m.

ITEM No. 4: Variance Case No. 20-06: Kevin Dahl, agent for James and Lavina Platt, requests a variance from the Yuma County Zoning Ordinance, Section 601.05- Principal Building Setbacks, to allow a front yard setback of 3 feet, a rear yard setback of 2 feet, and a side yard setback of 3 feet on a parcel 9,583 square feet in size zoned Rural Area-20 acre minimum (RA-20), Assessor's Parcel Number 470-02-001, located at 10444 East Swede Lane, Yuma, Arizona.

Senior Planner Fernando Villegas presented the staff report recommending approval of Variance Case No. 20-06 based on:

1. Staff finds are peculiar conditions which deprive the property of privilege enjoyed by other property owners in the zoning district.
2. Staff finds there are conditions or circumstances unique to the development of the property to warrant granting of a variance
3. Staff finds the condition is not self-imposed.

If the Board of Adjustment approves this Variance, staff suggests attaching the following conditions:

1. This variance is valid for the time limits outlined in Section 403.07 of the Zoning Ordinance.

Board member Tucker inquired about the square footage of the subject property.

Senior Planner Fernando Villegas stated the subject property was 9,583 square feet.

Chairman Tucker opened the public hearing.

Jim Platt, 10444 East Swede Lane, Yuma, Arizona, owner of the subject property, explained he approached the County for guidance in placing a house on the subject property and applied for the variance.

Kevin Dahl, 1560 S. 5th Avenue, Yuma, Arizona, agent, stated the owner did not have any other option but to obtain a variance for the setbacks. He explained the lots are tight and constrained. He stated the requested setbacks do not cause any visibility issues.

Board Members reviewed an aerial image of the subject property and the surrounding properties.

There being no one else to come forward, Chairman Tucker closed public meeting.

Board Member Eisenmann inquired about the letter of opposition that was submitted to staff in the morning prior to the meeting.

Senior Planner Fernando Villegas stated staff received two phone calls from the owners of lot 3 and 4 adjacent to lot 2.

MOTION (LINES/EISENMANN): Approve Variance Case No. 20-06 subject to staff recommendations and conditions that are imposed.

ROLL CALL VOTE: Eisenmann – AYE; Tucker - AYE; Saltzer – AYE; Lines- AYE.
The motion carried 4-0.

ITEM No. 5: Variance Case No. 20-08: Guillermina Fuentes, of A & F Homebuilders, LLC, requests a variance from the Yuma County Zoning Ordinance, Section 706.04—Noise Zones, to allow a single family residence on a parcel 2.74 gross acres in size zoned Suburban Site Built-2 acre minimum (SSB-2), Assessor's Parcel Number 724-34-016, located on the southeast corner of Avenue 4 ½ E and County 13th Street, Yuma, Arizona; located in the 65-69 dB noise zone.

Senior Planner Javier Barraza presented the staff report recommending approval of Variance Case No. 20-08 based on:

1. Granting the request would not be contrary to the public interest.
2. Granting this request would be in accordance with the spirit of the regulations and the Airport District.

If the Board of Adjustment approves this Variance, staff suggests attaching the following conditions:

1. This variance is valid for the time limits outlined in Section 710.08 of the Yuma County Zoning Ordinance.
2. An Avigation Disclosure Statement shall be recorded by the owner/agent within 60 days of approval by the Board of Adjustment.
3. Measures to achieve an outdoor to indoor noise reduction of 25 decibels must be incorporated into the design and construction of the residence, per Yuma County Board of Supervisors Ordinance 02-01—Sound Attenuation Standards for Yuma County and the Yuma County Comprehensive Building and Safety Code.

Planning Director Maggie Castro, AICP, stated the subject property is located in the old Joint Land Use Planning area. The Joint Land Use Plan was adopted in 1996 by the City of Yuma and Yuma County. She explained that MCAS, the City and the County agreed that a density of 2 acre minimum in a 65 dB noise zone was acceptable. The same designation carried over into the 2020 Comprehensive plan. Therefore, the subject property is in compliance with the goals and objectives of the 2020 Comprehensive Plan and the Joint Land Use Plan.

Chairman Tucker opened the public hearing.

There being no one to come forward, Chairman Tucker closed the public meeting.

MOTION (EISENMANN/LINES): Approve Variance Case No. 20-08 as submitted to include the requirements of the staff.

ROLL CALL VOTE: Eisenmann – AYE; Tucker - AYE; Saltzer – AYE; Lines- AYE.
The motion carried 4-0.

ITEM No. 6: Variance Case No. 20-09: Kevin Dahl, agent for TDI Holdings LLC, requests a variance from the Yuma County Zoning Ordinance, Section 706.04—Noise Zones, to allow a single family residence on a parcel 2.0 gross acres in size zoned Suburban Site Built -2 acre minimum (SSB-2), Assessor's Parcel Number 754-18-009, located 875 feet west of the intersection of Avenue B ½ and the alignment of County 16 ¾ Street, Somerton, Arizona; located in the arrival and departure corridor but outside the accident potential zone one, two and noise contour lines.

Senior Planner Javier Barraza presented the staff report recommending approval of Variance Case No. 20-09 based on:

1. Granting this request would not be contrary to the public interest.
2. Granting this request would be in accordance with the spirit of the regulations and the Airport District.

If the Board of Adjustment approves this Variance, staff suggests attaching the following conditions:

1. This variance is valid for the time limits outlined in Section 710.08 of the Yuma County Zoning Ordinance.
2. An Avigation Disclosure Statement shall be recorded by the owner/agent within 60 days of approval by the Board of Adjustment.

Board Member Eisenmann inquired about other residences that exist in the subject property area.

Senior Planner Javier Barraza displayed an aerial image and explained the location and lot numbers of the residences in the area.

Chairman Tucker opened the public hearing.

There being no one to come forward, Chairman Tucker closed the public meeting.

MOTION (LINES/SALTZER): Approve Variance Case No. 20-09 as staff recommends and any conditions that staff recommends.

ROLL CALL VOTE: Eisenmann – AYE; Tucker - AYE; Saltzer – AYE; Lines- AYE.
The motion carried 4-0.

ITEM No. 7: Discussion by the Board members and Planning Director of events attended, current events, and the schedule for future Board of Adjustment meetings.

There being no further business to come before the Board, the Chairman adjourned the meeting at 1:28 p.m.

Approved and accepted on this 16th day of June, 2020.

Neil Tucker, Chairman

ATTEST:

Maggie Castro, AICP, Planning Director

DRAFT

Yuma County Board of Adjustment

Item No. 4

AIR-9900

4.

BOA Agenda

Meeting Date: 06/16/2020

Submitted For: Maggie Castro

Submitted By: Juan Leal-Rubio

Department: Planning & Zoning Division - DDS

Information

1. REQUESTED ACTION:

Variance Case No. 20-10: Robert and Judy Satchell request a variance from the development standards of Martinez Lake Resort Unit No. 1 Planned Development to increase the height to 35.5 feet on a parcel 1,620 square feet in size zoned Planned Development (PD), Assessor's Parcel Number 459-50-025, located at 10377 East Tule Road, Yuma, Arizona.

2. INTENT:

To allow a maximum building height of 35.5 feet where 32 feet is the maximum allowed.

3. For detailed analysis see attached staff report

4. STAFF'S RECOMMENDATION:

Staff recommends approval of Variance Case No. 20-10 based on:

1. Staff finds approval of this variance may not have an adverse effect on public health, safety and welfare.
 2. Staff finds there are specific peculiar conditions applicable to this property to warrant granting of this variance.
 3. Staff finds approval of this request does not have a negative impact on the neighborhood.
-

Attachments

Staff Report

Case Map

Site Plan

Elevation Plans

Floor Plan

Internal Comments

External Comments

STAFF REPORT
Yuma County Planning and Zoning Division

Prepared for the Hearing of
June 16, 2020
Yuma County Board of Adjustment

CASE NUMBER: Variance Case No. 20-10
OWNER: Robert and Judy Satchell
CASE PLANNER: Juan Leal Rubio, Senior Planner
DATE PREPARED: May 08, 2020

DESCRIPTION OF REQUEST: Robert and Judy Satchell request a variance from the development standards of Martinez Lake Resort Unit No. 1 Planned Development to increase the height to 35.5 feet on a parcel 1,620 square feet in size zoned Planned Development (PD), Assessor’s Parcel Number 459-50-025, located at 10377 East Tule Road, Yuma, Arizona.

THE APPLICANT’S REASON FOR REQUESTING THIS VARIANCE: The intent is to build a residence 35.5 feet in height.

APPROVAL OF THIS REQUEST WOULD ALLOW:

The following deviations from the Martinez Lake Resort Unit No. 1 Planned Development:

1. A maximum building height of 35.5 feet where 32 feet is the maximum allowed.
-

BACKGROUND

The subject property is located within Martinez Lake Resort Unit No. 1 Planned Development (PD) consisting of 140 single family residential lots and 21 tracts. Additionally, the property is located in the Martinez Lake Planning Area of the 2020 Comprehensive Plan. The property was rezoned to PD with Rezoning Case No. 02-39 on January 18, 2005. There is an existing recreational vehicle on the subject property.

The purpose of the PD district is to encourage imaginative and innovative planning, particularly with respect to diversification in the land use and flexibility in site designs, respect to various features, including, but not limited to, spacing, heights, density, open space, circulation, preservation of natural features, and innovative development that results in the availability of a variety of residential/commercial/industrial

opportunities. Martinez Lake Resort Unit No. 1 subdivision was rezoned to PD due to the unusual shape, topography, and configuration of the lots. The development standards allow for development of the lots with a setback of zero feet in the side and rear yards provided construction of dwelling units complies with the adopted fire code.

The Yuma County Zoning Ordinance, Section 616.07, states that any minor extensions, alterations or modifications of existing buildings, structures or elements of the plan may be authorized by the Planning Director if they are consistent with the purposes and intent of the plan. All other changes in the Development Plan must be approved under the procedures authorized by the Zoning Ordinance.

STAFF ANALYSIS

Section 403.03 of the Zoning Ordinance:

- A. *Variiances under section 403.02 shall be granted only when, because of peculiar conditions applicable to the property, the strict application of the Zoning Ordinance deprives such property of privilege enjoyed by other property owners in the zoning district.*

Staff finds there are peculiar conditions applicable to the subject property. The development standards allow for development of the lots with a maximum height of 32 feet which does not provide enough room for three floors. The applicant is requesting a maximum height of 35.5 feet for the dwelling unit in order to accommodate ground level parking and two additional levels of habitable space.

- B. *Variiances are available only in cases where there is a hardship arising from conditions or circumstances unique to the development of a particular piece of land, not from personal considerations, personal convenience or financial hardships.*

Staff finds there is hardship arising from conditions or circumstances unique to the development of this property. The development standards of the PD limit development of the properties to units with parking on the ground floor and the dwelling above the parking. However, the maximum height of 32 feet does not allow construction of three floors. Increasing the maximum height by three and a half feet will enable the construction of a unit with three floors.

- C. *Any motion to grant a Variance by the Board of Adjustment shall include specific peculiar conditions applicable to the property, which exist to cause granting of a Variance.*

Staff finds there are specific peculiar conditions applicable to this property to warrant granting of this variance. The unique characteristics in the Martinez Lake Resort Unit No. 1 PD require that parking be provided on the first level of each unit thereby limiting the number of floors that can be used for habitable area. Increasing the maximum height to 35.5 feet will enable the construction of two floors above the parking area dedicated to habitable space.

- D. *A variance shall not be granted which will have an adverse effect on public health, safety and welfare.*

Staff finds approval of this request will not have adverse effects on public health, safety, and welfare. The increase in the maximum height will allow the property owner to improve the property with the ground floor used solely for parking and two floors for livable area. The intent is to remove the existing recreational vehicle from the subject property thereby improving the physical condition of the subject property.

E. A variance shall not be granted if, in granting the variances a special privilege not commonly enjoyed by others in the zoning district will be conferred, or have a negative impact on the neighborhood.

Staff finds granting this variance does not appear to confer a special privilege since increasing the maximum height to 35.5 feet will facilitate the construction of a unit with the ground floor used solely for parking and two floors above the parking area for livable space.

F. The Board of Adjustment may require appropriate conditions or safeguards on any granted variances so that public health, safety and welfare are not compromised.

Staff is recommending that if approved, this variance include the attached conditions as a means of addressing concerns of public health, safety, and welfare.

G. Variances shall not be granted if the condition is self-imposed or if a reasonable use of the land can be made in an alternative development scheme without the variance.

Staff finds the condition is self-imposed. The alternative is to amend the entire Martinez Lake Resort Unit No.1 Planned Development to allow a maximum height of 36 feet.

H. The fact that there are non-conforming uses of neighborhood lands, structures or buildings, in the same zoning district shall not be considered grounds for issuance of a variance.

Staff's recommendation is not based on non-conforming uses of neighborhood lands, structures or buildings in the same zoning district.

H. The fact that there are non-conforming uses of neighborhood lands, structures or buildings, in the same zoning district shall not be considered grounds for issuance of a variance.

Staff's recommendation is not based on non-conforming uses of neighborhood lands, structures or buildings in the same zoning district. The Board of Adjustment approved the following variances allowing a height of 36 feet in the Martinez Lake Resort PD subdivision:

- Variance Case No. 17-06 (05/16/17)
- Variance Case No. 18-03 (03/20/18)
- Variance Case No. 18-10 (07/17/18)
- Variance Case No. 18-13 (08/21/2018)
- Variance Case No. 19-11 (10/15/2019)
- Variance Case No. 19-12 (10/15/2019)
- Variance Case No. 19-14 (11/19/2019)
- Variance Case No. 20-04 (04/21/2020)

ADDITIONAL COMMENTS:

Yuma County Chief Building/Fire Code Official Pat Headington stated that exterior walls located within 3 feet of property lines shall meet the fire-resistance requirements of the 2018 International Residential Code; Sprinklers shall be installed in accordance with 2018 International Residential Code.

Wellton-Mohawk Irrigation and Drainage District's General Manager Elston Grubaugh, stated that electrical service was now all underground for the full length of Vista del Rio Street.

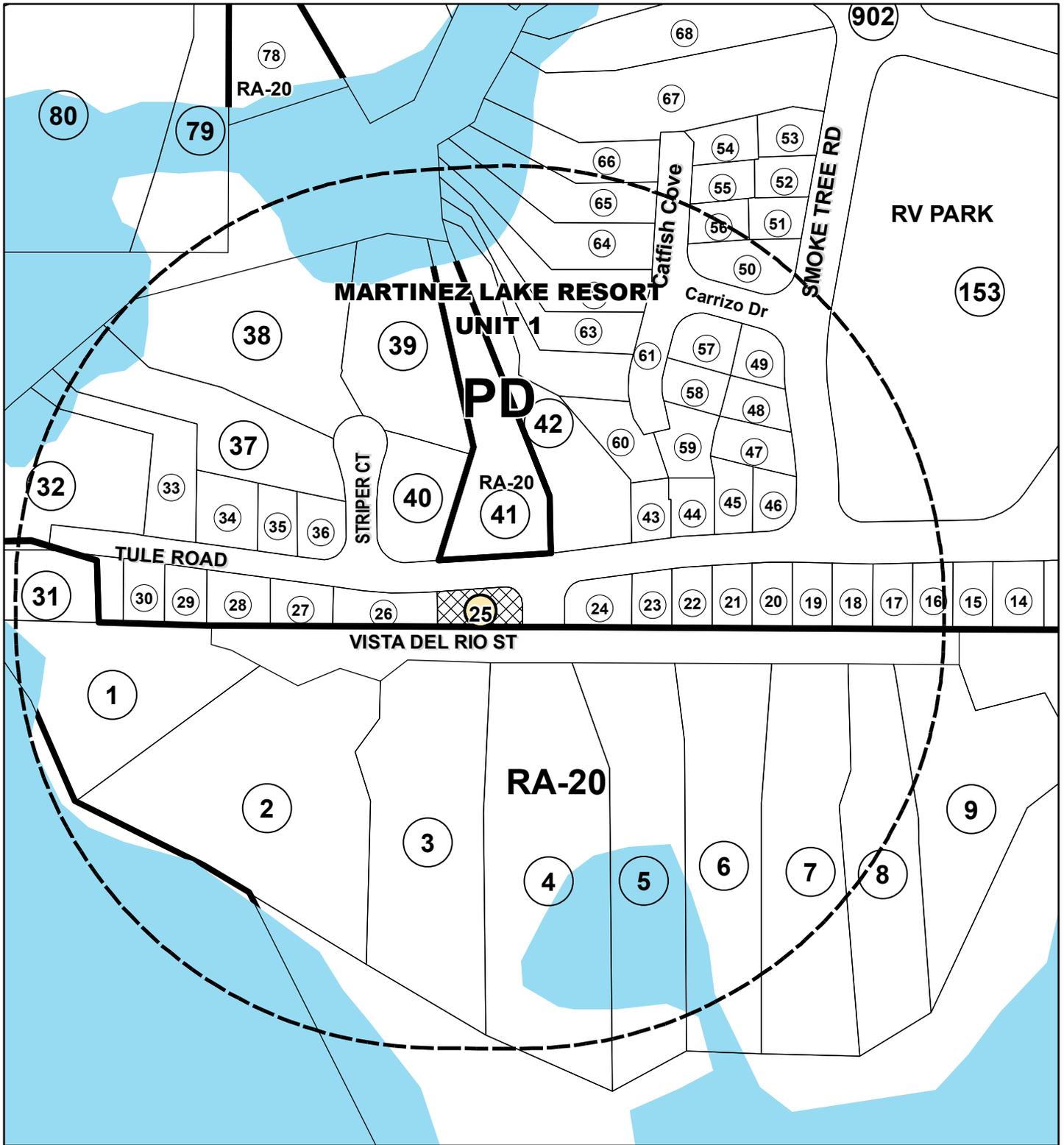
STAFF RECOMMENDATION

Staff recommends approval of Variance Case No. 20-10 based on:

1. Staff finds approval of this variance may not have an adverse effect on public health, safety and welfare.
2. Staff finds there are specific peculiar conditions applicable to this property to warrant granting of this variance.
3. Staff finds approval of this request does not have a negative impact on the neighborhood.

If the Board of Adjustment approves this Variance, staff suggests attaching the following condition:

1. This variance is valid for the time limits outlined in Section 403.07 of the Zoning Ordinance.




DEPARTMENT OF DEVELOPMENT SERVICES
PLANNING & ZONING DIVISION
 2351 W. 26TH STREET
 YUMA, AZ 85364

FOR INFORMATION ONLY - NO LIABILITY ASSUMED

CASE NO: V20-10
LOCATION: 10377 E. Tule Road
APN(s): 459-50-025

CASE PLANNER: Juan Leal Rubio
DATE DRAWN: 05-07-2020
REVIEWED BY: N/A

SCALE: 1" = 100'

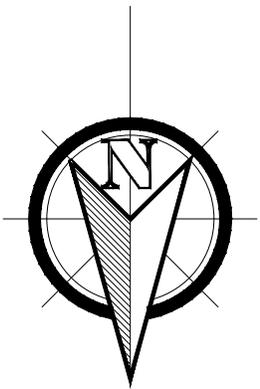
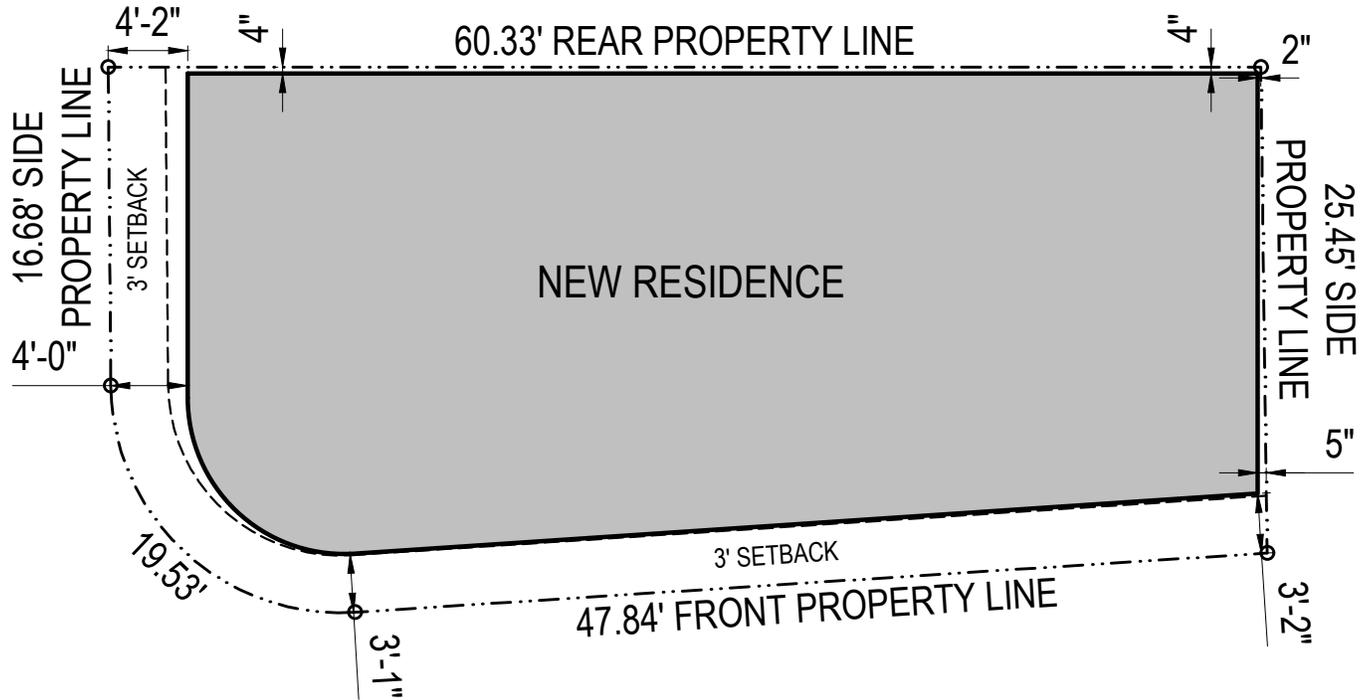
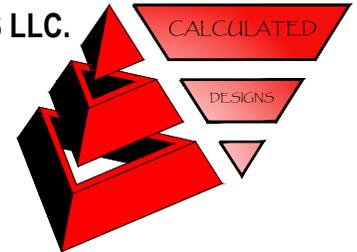


Legend

-  300' BUFFER
-  SUBJECT PROPERTY
-  Zoning Boundary

PROJECT	NEW RESIDENCE
NAME:	SATCHELL
ADDRESS	10377 E. TULE RD.
APN#	14-459-50-025
LOT#	24

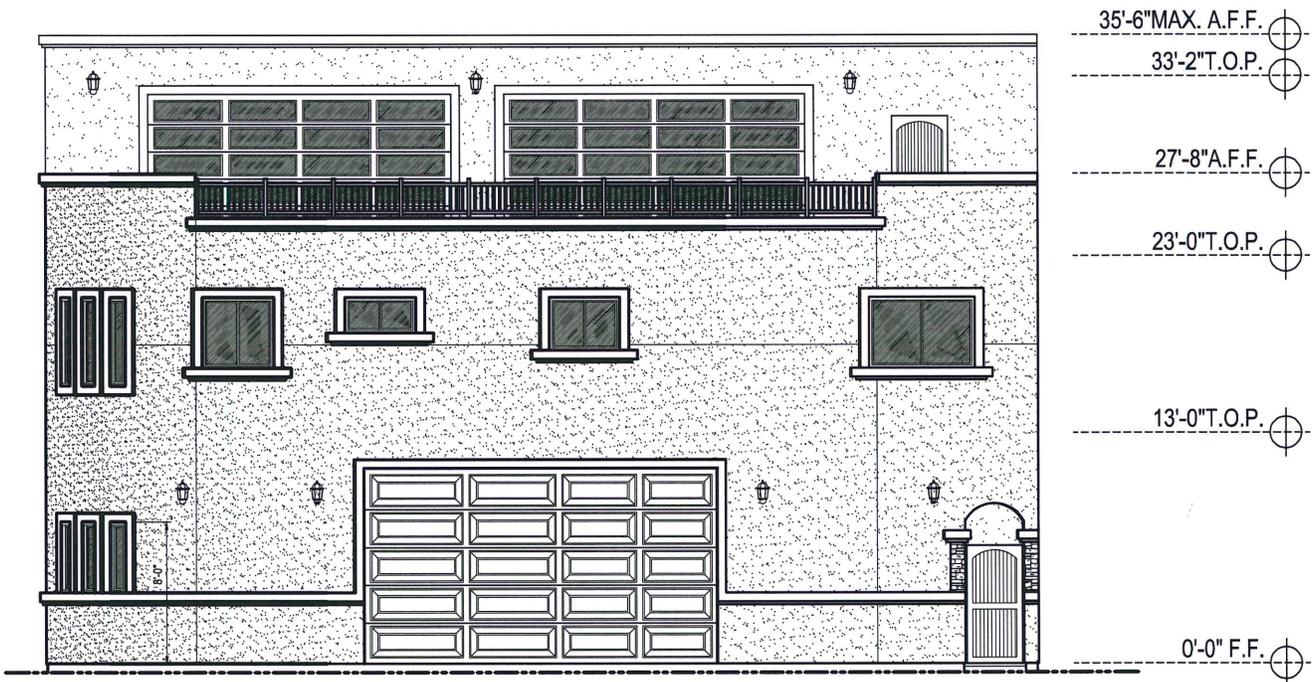
CALCULATED DESIGNS LLC.
 2615 E. 24TH ST. STE.5
 PHONE: 928.726.4600



SITE PLAN

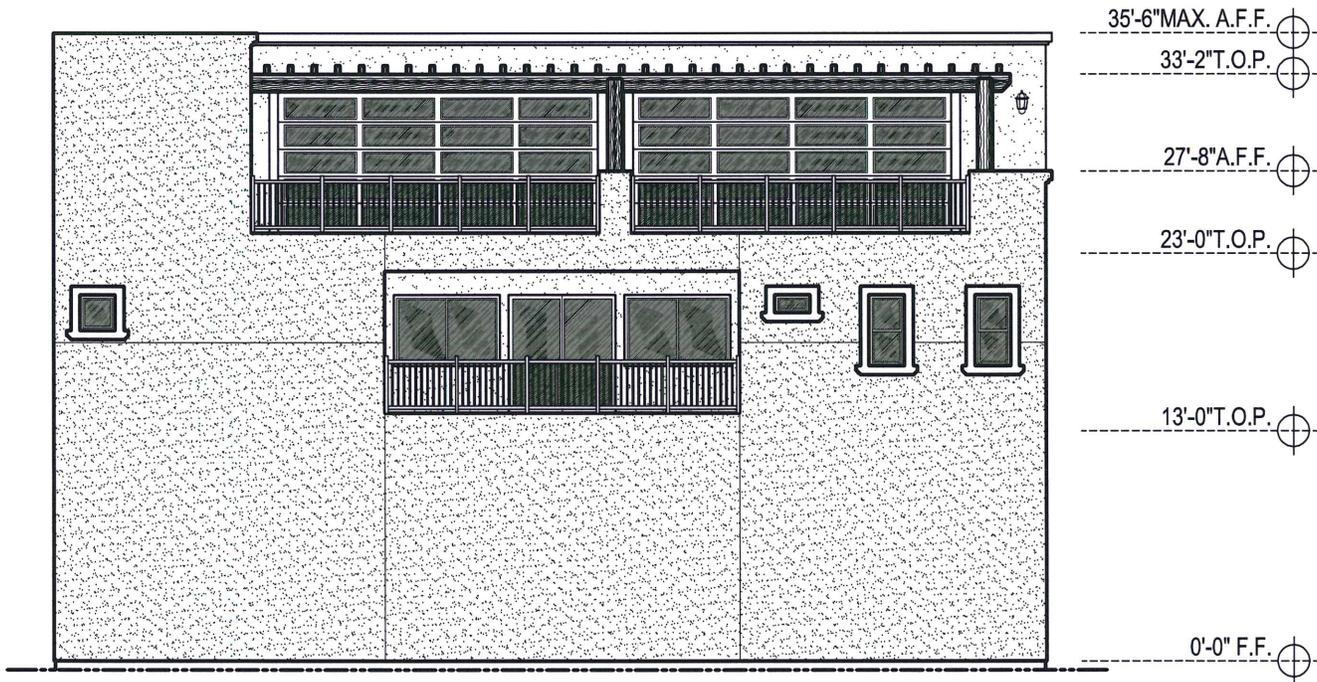
SCALE: 1"=10'

SQUARE FOOTAGE CALCULATIONS	
1ST FLOOR	
AREA NAME:	SQUARE FOOTAGE
GARAGE	1182.69 SQ. FT.
STAIRS	118.50 SQ. FT.
ENTRY	18.67 SQ. FT.
2ND FLOOR	
AREA NAME:	SQUARE FOOTAGE
LIVING AREA	1149.37 SQ. FT.
REAR COVERED BALCONY	70.00 SQ. FT.
STAIRS	100.50 SQ. FT.
3RD FLOOR	
AREA NAME:	SQUARE FOOTAGE
COVERED PATIO	580.64 SQ. FT.
PERGOLA	244.03 SQ. FT.
TOTAL LOT COVERED AREA	1319.87 SQ. FT.



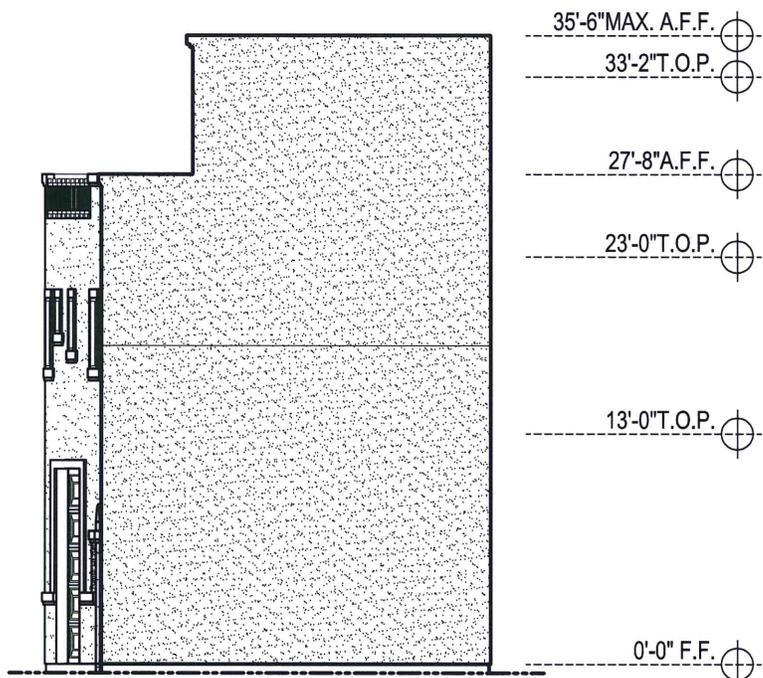
FRONT ELEVATION

SCALE: $1/8" = 1'-0"$



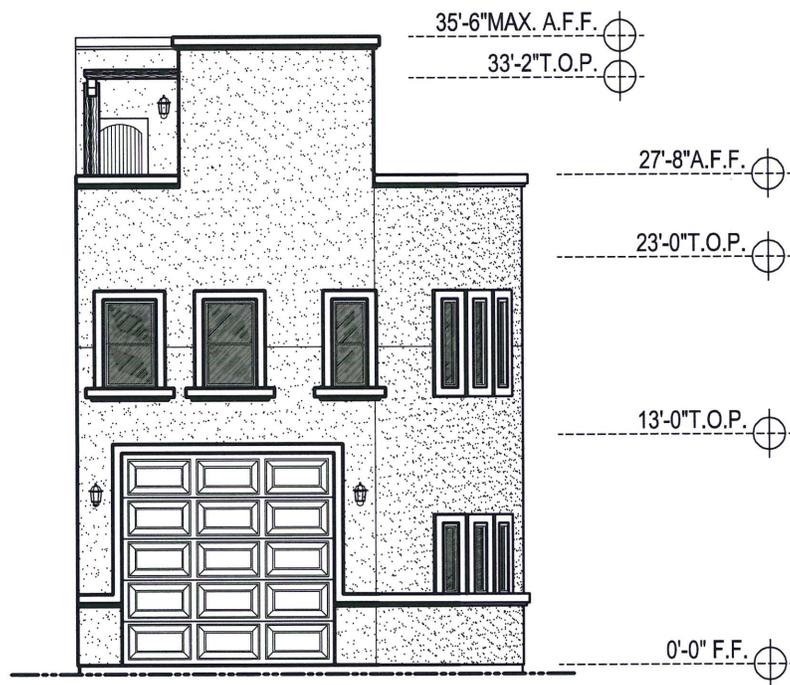
REAR ELEVATION

SCALE: $1/8" = 1'-0"$



RIGHT ELEVATION

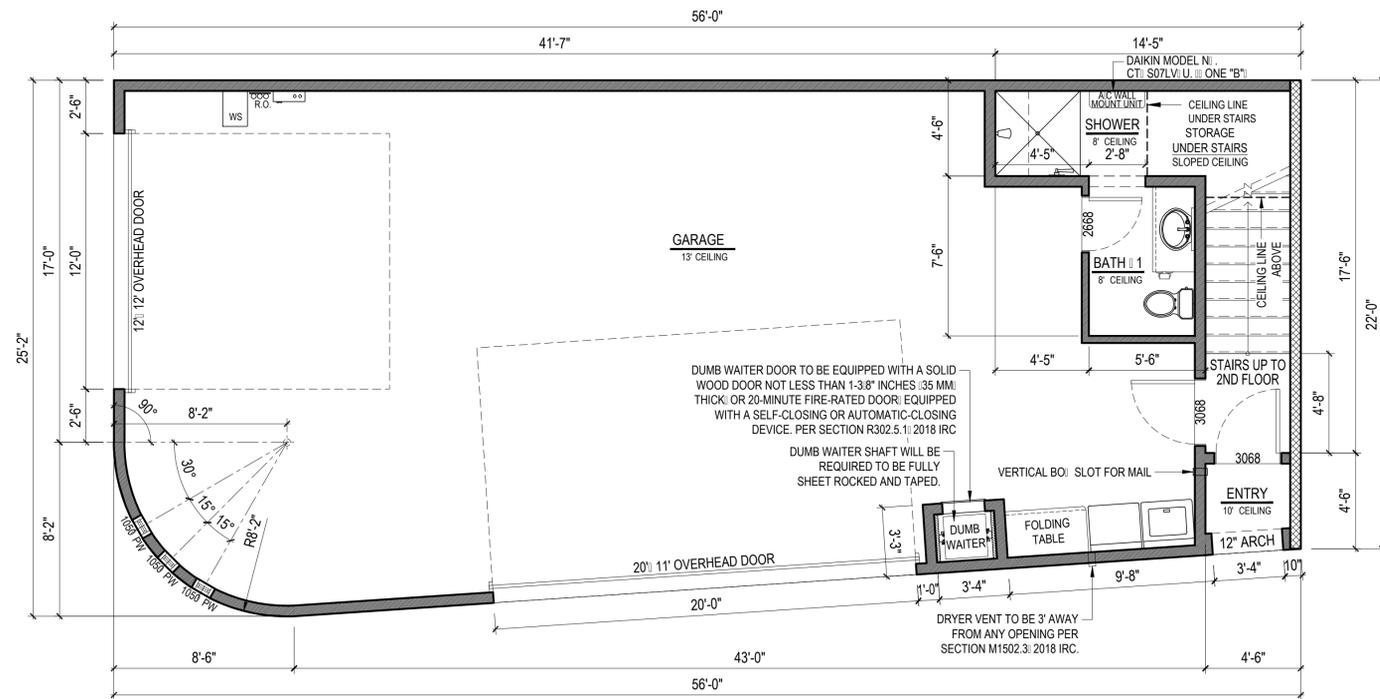
SCALE: $1/8" = 1'-0"$



LEFT ELEVATION

SCALE:

1/8" = 1'-0"



FLOOR PLAN 1ST FLOOR

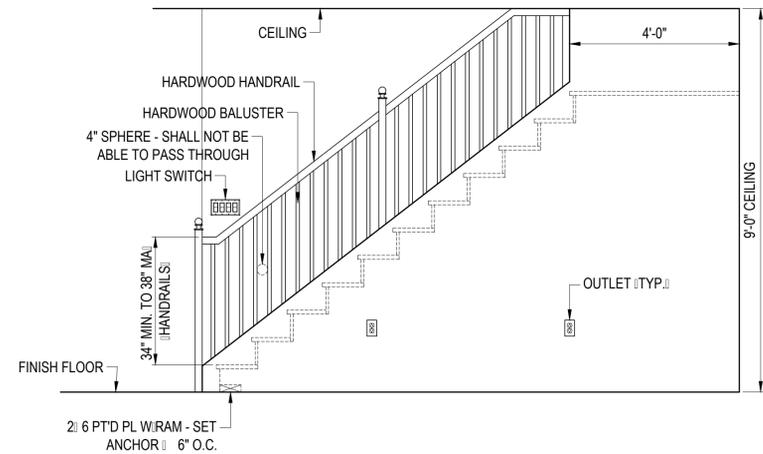
SCALE: 1/4" = 1'-0"

WALL LEGEND

STANDARD WALLS	
1-HR FIRE RATED WALLS	

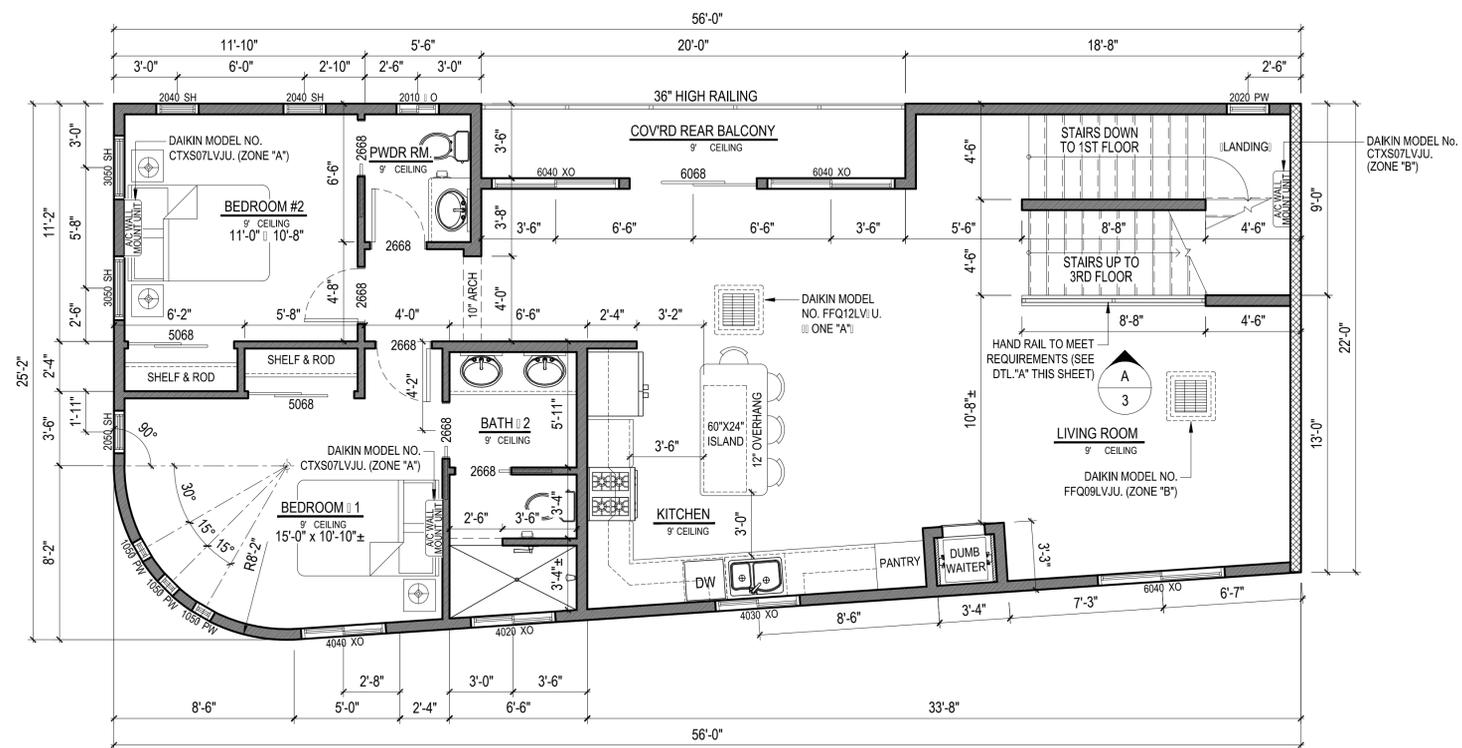
SQUARE FOOTAGE CALCULATIONS

1ST FLOOR	
AREA NAME:	SQUARE FOOTAGE
GARAGE	1182.69 SQ. FT.
STAIRS	118.50 SQ. FT.
ENTRY	18.67 SQ. FT.
2ND FLOOR	
AREA NAME:	SQUARE FOOTAGE
LIVING AREA	1149.37 SQ. FT.
REAR COVERED BALCONY	70.00 SQ. FT.
STAIRS	100.50 SQ. FT.
3RD FLOOR	
AREA NAME:	SQUARE FOOTAGE
COVERED PATIO	580.64 SQ. FT.
PERGOLA	244.03 SQ. FT.
TOTAL LOT COVERED AREA	1319.87 SQ. FT.



"A" RAIL DETAIL

SCALE: 1/2" = 1'-0"



FLOOR PLAN 2ND FLOOR

SCALE: 1/4" = 1'-0"

REVISIONS

NO.	DATE	DESCRIPTION

FLOOR PLAN

NEW PROJECT FOR: SATCHELL
 PROJECT ADDRESS: 10377 E. TULE RD.
 DRAWN BY: LC
 REVIEWED BY: STAFF
 LAST REVISION: N/A

APN: 14-459-50-025
 COMPLETED: 04-08-2020

NE RESIDENCE FOR SATCHELL

UMA ARIANA APN

CALCULATED DESIGNS LLC

2615 E. 24TH STREET STE. 5
 OFFICE: 928-726-4600

OF 8 SHEETS

Juan Leal Rubio

From: Joe Wehrle
Sent: Thursday, May 7, 2020 3:09 PM
To: Juan Leal Rubio
Subject: RE: RFC Variance Case No. 20-10

Juan,

The Assessor's Office has no comments or concerns for the proposed variance.

Joe Wehrle
Yuma County Assessor
192 South Maiden Lane
Yuma, AZ 85364
Joe.wehrle@yumacountyaz.gov
(928) 373-6073

From: Juan Leal Rubio
Sent: Thursday, May 7, 2020 11:08 AM
To: Anne.camacho@aps.com; grcopeland@antelopeunion.org; Wellton@town.wellton.az.us; BKnowles@azgfd.gov; rjism09@msn.com; Bfenske@azdot.gov; chuck.wullenjohn@us.army.mil; bkleee@craneschools.org; agui2400@yahoo.com; Gen@yumaairport.com; gramirez@azdot.gov; IGarcia@azdot.gov; Jeff_humphrey@fws.gov; jennifer.albers@yumaaz.gov; esperodriguez@ssd11.org; jcampa@cityofsanluis.org; JTheidrich@aol.com; mstraub@wmidd.org; sjohnson@mohawk17.org; MCASYUMA_CPLO@usmc.mil; rmolenaar@yumasun.com; Rick.rohrick@swgas.com; bryank@unitBIRR.com; ttyree@apscc.org; yid@mindspring.com; Pmorgan@ymidd.org; gthompson@yumaunion.org; Cgutierrez@ympo.org; daniel.m.steward.civ@mail.mil; Stephanie.Laborin@aps.com; Joe Wehrle <Joe.Wehrle@yumacountyaz.gov>; jmacdona@blm.gov; judith.e.movilla.civ@mail.mil; lkillman@town.wellton.az.us; info@ssd11.org; Robert.blevins@yumaaz.gov; sfajardo@mohawk17.org; tony.lomboy@charter.com; usarmy.ypg.atec.list.public-affairs-office@mail.mil; openunuri@ycwua.org; golivas@yumaunion.org; stephen.eckert@azwestern.edu; Javier Barraza <Javier.Barraza@yumacountyaz.gov>; Fernando Villegas <Fernando.Villegas@yumacountyaz.gov>; Arturo Alvarez <Arturo.Alvarez@yumacountyaz.gov>; Pat Headington <Pat.Headington@yumacountyaz.gov>; Rachel Stallworth <Rachel.Stallworth@yumacountyaz.gov>; George Amaya <George.Amaya@yumacountyaz.gov>; joseph.a.garcia46.civ@mail.mil; bobette.m.bauermann.civ@mail.mil; Mary.e.finch@usmc.mil; jperez@cityofsanluis.org; kleonard@azstateparks.gov; Ed.Alupay@CenturyLink.com; contact@ngvidd.com; egrubaugh@wmidd.org; LCVALDEZ@UP.COM; carmenjuarez@somertonaz.gov; antonio.martinez1@usmc.mil
Subject: RFC Variance Case No. 20-10

Please review and let me know if you have any questions, comments or concerns.

Regards,

Juan Leal Rubio, Senior Planner

Department of Development Services/Planning & Zoning Division
2351 W. 26th Street Yuma, AZ 85364
(928) 817-5176 | Fax (928) 817-5020

If you would like to provide comments regarding the service you received, please click the following link: [Yuma County : Customer Survey](#)

From: Rachel Stallworth
Sent: Thursday, May 7, 2020 11:11 AM
To: Juan Leal Rubio
Subject: RE: RFC Variance Case No. 20-10

No Comment on this one, Juan.

Rachel Stallworth

Improvement Districts

rachel.stallworth@yumacountyaz.gov

4343 S. Avenue 5 1/2E

Yuma, AZ 85365

Office: (928) 341-2500

Direct Line: (928) 341-2511

Mobile: (928) 581-5889

From: Juan Leal Rubio

Sent: Thursday, May 7, 2020 11:08 AM

To: Anne.camacho@aps.com; grcopeland@antelopeunion.org; Wellton@town.wellton.az.us; BKnowles@azgfd.gov; rjism09@msn.com; Bfenske@azdot.gov; chuck.wullenjohn@us.army.mil; bkleee@craneschools.org; agui2400@yahoo.com; Gen@yumaairport.com; gramirez@azdot.gov; IGarcia@azdot.gov; Jeff_humphrey@fws.gov; jennifer.albers@yumaaz.gov; esperodriguez@ssd11.org; icampa@cityofsanluis.org; JHeidrich@aol.com; mstraub@wmidd.org; sjohnson@mohawk17.org; MCASYUMA_CPLO@usmc.mil; rmolenar@yumasun.com; Rick.rohrick@swgas.com; bryank@unitBIRR.com; ttyree@apscc.org; yid@mindspring.com; Pmorgan@ymidd.org; gthompson@yumaunion.org; Cgutierrez@ympo.org; daniel.m.steward.civ@mail.mil; Stephanie.Laborin@aps.com; Joe Wehrle <Joe.Wehrle@yumacountyaz.gov>; jmacdona@blm.gov; judith.e.movilla.civ@mail.mil; lkillman@town.wellton.az.us; info@ssd11.org; Robert.blevins@yumaaz.gov; sfajardo@mohawk17.org; tony.lomboy@charter.com; usarmy.ypg.atec.list.public-affairs-office@mail.mil; openunuri@ycwua.org; golivas@yumaunion.org; stephen.eckert@azwestern.edu; Javier Barraza <Javier.Barraza@yumacountyaz.gov>; Fernando Villegas <Fernando.Villegas@yumacountyaz.gov>; Arturo Alvarez <Arturo.Alvarez@yumacountyaz.gov>; Pat Headington <Pat.Headington@yumacountyaz.gov>; Rachel Stallworth <Rachel.Stallworth@yumacountyaz.gov>; George Amaya <George.Amaya@yumacountyaz.gov>; joseph.a.garcia46.civ@mail.mil; bobette.m.bauermann.civ@mail.mil; Mary.e.finch@usmc.mil; jperez@cityofsanluis.org; kleonard@azstateparks.gov; Ed.Alupay@CenturyLink.com; contact@ngvidd.com; egrubough@wmidd.org; LCVALDEZ@UP.COM; carmenjuarez@somertonaz.gov; antonio.martinez1@usmc.mil

Subject: RFC Variance Case No. 20-10

Please review and let me know if you have any questions, comments or concerns.

Regards,

Juan Leal Rubio, Senior Planner

Department of Development Services/Planning & Zoning Division
2351 W. 26th Street Yuma, AZ 85364



YUMA COUNTY
Planning & Zoning Division
REQUEST FOR COMMENTS

May 7, 2020

CASE NUMBER: VARIANCE CASE NO. 20-10

Attached for your consideration is a **Variance**. The Planning & Zoning staff would appreciate your review of this proposal and any comments you may have. Please check the applicable response below and return this form to me along with your comments (if applicable) by the deadline below. You may also provide your response and comments (if any) by e-mail. If you have no comment, please provide a “no comment” response. If you cannot respond by the deadline, please contact me.

CASE SUMMARY: Variance Case No. 20-10: Robert and Judy Satchell request a variance from the development standards of Martinez Lake Resort Unit No. 1 Planned Development to increase the height to 35.6 feet on a parcel 1,620 square feet in size zoned Planned Development (PD), Assessor’s Parcel Number 459-50-025, located at 10377 East Tule Road, Yuma, Arizona.

Intent:

To allow the construction of a single family residence with a maximum building height of 35.6 feet where 32 feet is the maximum allowed.

PUBLIC HEARING: Tentatively scheduled for June 16, 2020

COMMENTS DUE: ASAP

COMMENT

NO COMMENT

DATE: 05/08/2020

PRINTED NAME: George Amaya, R.S.

AGENCY/DIVISION: Environmental Programs

RETURN TO: Juan Leal Rubio
2351 West 26th Street
Yuma, Arizona 85364
Fax: (928)817-5157
Juan.Leal-Rubio@yumacountyaz.gov

Attachments: Case Map and Site Plan



YUMA COUNTY
Planning & Zoning Division
REQUEST FOR COMMENTS

May 7, 2020

CASE NUMBER: VARIANCE CASE NO. 20-10

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PUBLIC HEARING: Tentatively scheduled for June 16, 2020

COMMENTS DUE: ASAP

COMMENT

NO COMMENT

DATE: 5/7/20 PRINTED NAME: Isabell Garcia

AGENCY/DIVISION: ADOT Southwest District

RETURN TO: Juan Leal Rubio
2351 West 26th Street
Yuma, Arizona 85364
Fax: (928)817-5157
Juan.Leal-Rubio@yumacountyaz.gov

Attachments: Case Map and Site Plan



YUMA COUNTY
Planning & Zoning Division
REQUEST FOR COMMENTS

May 7, 2020

CASE NUMBER: VARIANCE CASE NO. 20-10

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PUBLIC HEARING: Tentatively scheduled for June 16, 2020

COMMENTS DUE: ASAP

COMMENT NO COMMENT

DATE: 05/07/20 PRINTED NAME: Robert M. Blevins

AGENCY/DIVISION: City of Yuma, Community Planning

RETURN TO: Juan Leal Rubio
2351 West 26th Street
Yuma, Arizona 85364
Fax: (928)817-5157
Juan.Leal-Rubio@yumacountyaz.gov

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YUMA COUNTY
Planning & Zoning Division
REQUEST FOR COMMENTS

May 7, 2020

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PUBLIC HEARING: Tentatively scheduled for June 16, 2020

COMMENTS DUE: ASAP

 COMMENT X NO COMMENT

MCAS Yuma has reviewed the request for APN 459-50-025 and we have no objection to the case as requested. Thank you for the opportunity to review.

DATE: 7 May 2020 PRINTED NAME: Antonio Martinez, Community Liaison Specialist

AGENCY/DIVISION: MCAS Yuma

RETURN TO: Juan Leal Rubio
 2351 West 26th Street
 Yuma, Arizona 85364
 Fax: (928)817-5157
 Juan.Leal-Rubio@yumacountyaz.gov

Attachments: Case Map and Site Plan



YUMA COUNTY
Planning & Zoning Division
REQUEST FOR COMMENTS

May 7, 2020

CASE NUMBER: VARIANCE CASE NO. 20-10

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COMMENTS DUE: ASAP

 COMMENT X NO COMMENT

The District has completed its electrical system undergrounding project for the full length of Vista Del Rio Street.
The only wires still remaining on the poles is telecom (CenturyLink).

DATE: May 7, 2020 PRINTED NAME: Elston Grubaugh, Manager

AGENCY/DIVISION: WELLTON-MOHAWK IRRIGATION & DRAINAGE DISTRICT

RETURN TO: Juan Leal Rubio
 2351 West 26th Street
 Yuma, Arizona 85364
 Fax: (928)817-5157
 Juan.Leal-Rubio@yumacountyaz.gov

Attachments: Case Map and Site Plan



YUMA COUNTY
Planning & Zoning Division
REQUEST FOR COMMENTS

May 7, 2020

CASE NUMBER: VARIANCE CASE NO. 20-10

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PUBLIC HEARING: Tentatively scheduled for June 16, 2020

COMMENTS DUE: ASAP

COMMENT

NO COMMENT

DATE: 5/7/20 PRINTED NAME: Gen Grosse, Property/Community Relations

AGENCY/DIVISION: Yuma County Airport Authority

RETURN TO: Juan Leal Rubio
2351 West 26th Street
Yuma, Arizona 85364
Fax: (928)817-5157
Juan.Leal-Rubio@yumacountyaz.gov

Attachments: Case Map and Site Plan

From: YCWUA Planning <planning@ycwua.org>
Sent: Friday, May 8, 2020 6:28 AM
To: Juan Leal Rubio
Cc: planning@ycwua.org
Subject: Re: RFC Variance Case No. 20-10

Hello Mr. Juan,

The YCWUA has no comments about this. Thanks for the submittal and have a great Friday!

Omar Peñuñuri
Yuma County Water Users' Association

On 5/7/2020 11:08 AM, Juan Leal Rubio wrote:

Please review and let me know if you have any questions, comments or concerns.

Regards,

Juan Leal Rubio, Senior Planner

Department of Development Services/Planning & Zoning Division
2351 W. 26th Street Yuma, AZ 85364
(928) 817-5176 | Fax (928) 817-5020

If you would like to provide comments regarding the service you received, please click the following link: [Yuma County : Customer Survey](#)



From: Pat Morgan <pmorgan@ymidd.org>
Sent: Thursday, May 7, 2020 12:54 PM
To: Juan Leal Rubio
Subject: RE: RFC Variance Case No. 20-10

YMIDD has no Comment

From: Juan Leal Rubio [<mailto:Juan.Leal-Rubio@yumacountyaz.gov>]
Sent: Thursday, May 7, 2020 11:08 AM
To: Anne.camacho@aps.com; grcopeland@antelopeunion.org; Wellton@town.wellton.az.us; BKnowles@azgfd.gov; rjism09@msn.com; Bfenske@azdot.gov; chuck.wullenjohn@us.army.mil; bkleee@craneschools.org; agui2400@yahoo.com; Gen@yumaairport.com; gramirez@azdot.gov; IGarcia@azdot.gov; Jeff_humphrey@fws.gov; jennifer.albers@yumaaz.gov; esperodriguez@ssd11.org; icampa@cityofsanluis.org; JTheidrich@aol.com; mstraub@wmidd.org; sjohnson@mohawk17.org; MCASYUMA_CPLO@usmc.mil; rmolenar@yumasun.com; Rick.rohrick@swgas.com; bryank@unitBIRR.com; ttyree@apscc.org; vid@mindspring.com; Pmorgan@ymidd.org; gthompson@yumaunion.org; Cgutierrez@ympo.org; daniel.m.steward.civ@mail.mil; Stephanie.Laborin@aps.com; Joe Wehrle <Joe.Wehrle@yumacountyaz.gov>; jmacdona@blm.gov; judith.e.movilla.civ@mail.mil; lkillman@town.wellton.az.us; info@ssd11.org; Robert.blevins@yumaaz.gov; sfajardo@mohawk17.org; tony.lomboy@charter.com; usarmy.ypg.atec.list.public-affairs-office@mail.mil; openunuri@ycwua.org; golivas@yumaunion.org; stephen.eckert@azwestern.edu; Javier Barraza <Javier.Barraza@yumacountyaz.gov>; Fernando Villegas <Fernando.Villegas@yumacountyaz.gov>; Arturo Alvarez <Arturo.Alvarez@yumacountyaz.gov>; Pat Headington <Pat.Headington@yumacountyaz.gov>; Rachel Stallworth <Rachel.Stallworth@yumacountyaz.gov>; George Amaya <George.Amaya@yumacountyaz.gov>; joseph.a.garcia46.civ@mail.mil; bobette.m.bauermann.civ@mail.mil; Mary.e.finch@usmc.mil; jperez@cityofsanluis.org; kleonard@azstateparks.gov; Ed.Alupay@CenturyLink.com; contact@ngvidd.com; egrubough@wmidd.org; LCVALDEZ@UP.COM; carmenjuarez@somertonaz.gov; antonio.martinez1@usmc.mil
Subject: RFC Variance Case No. 20-10

Please review and let me know if you have any questions, comments or concerns.

Regards,

Juan Leal Rubio, Senior Planner

Department of Development Services/Planning & Zoning Division
2351 W. 26th Street Yuma, AZ 85364
(928) 817-5176 |Fax (928) 817-5020

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YUMA COUNTY
Planning & Zoning Division
REQUEST FOR COMMENTS

May 7, 2020

CASE NUMBER: VARIANCE CASE NO. 20-10

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To allow the construction of a single family residence with a maximum building height of 35.6 feet where 32 feet is the maximum allowed.

PUBLIC HEARING: Tentatively scheduled for June 16, 2020

COMMENTS DUE: ASAP

COMMENT NO COMMENT

DATE: 13 May 2020 PRINTED NAME: Bobette Bauermann

AGENCY/DIVISION: YPG - Master Planning

RETURN TO: Juan Leal Rubio
2351 West 26th Street
Yuma, Arizona 85364
Fax: (928)817-5157
Juan.Leal-Rubio@yumacountyaz.gov

Attachments: Case Map and Site Plan

Yuma County Board of Adjustment

Item No. 5

AIR-9897

5.

BOA Agenda

Meeting Date: 06/16/2020

Submitted For: Maggie Castro

Submitted By: Maggie Castro

Department: Planning & Zoning Division - DDS

Information

1. REQUESTED ACTION:

Interpretation Case No. 20-01: Barry Olsen, agent for Terry W. Cameron, et. al., requests the following interpretations for the property owned by Tony L. & Kathleen Abbott and Bryan L. Abbott, located at 10343 East North Martinez Lake Road, Assessor's Parcel Number 459-50-078, zoned Rural Area-20 acre minimum (RA-20):

- Is the Abbott's proposed "New Garage" actually a dwelling unit, or an accessory dwelling unit, under Section 202.00 of the Zoning Ordinance?
- Is the Abbott's proposed "New Garage" an impermissible expansion of a non-conforming lot/use under Sections 1001.00, et. seq., of the Zoning Ordinance.
- If the Abbott's proposed "New Garage" is permissible under Sections 602 and 1001, et. seq., of the Zoning Ordinance, what is the required side yard setback for the "New Garage" per Section 1011 of the Zoning Ordinance?

2. INTENT:

Establish whether:

- The Abbott's proposed "New Garage" is actually a dwelling unit, or an accessory dwelling unit, under Section 202.00 of the Zoning Ordinance?
- The Abbott's proposed "New Garage" is an impermissible expansion of a non-conforming lot/use under Sections 1001.00, et. seq., of the Zoning Ordinance.
- If the Abbott's proposed "New Garage" is permissible under Sections 602 and 1001, et. seq., of the Zoning Ordinance, what is the required side yard setback for the "New Garage" per Section 1011 of the Zoning Ordinance?

3. For detailed analysis see attached staff report

4. STAFF'S RECOMMENDATION:

Staff recommends the Board of Adjustment find that:

- The structure is not intended or permitted for residential purposes.
 - The structure permitted with Building Permit No. B17-0377 is an accessory use permitted pursuant to Section 601.02(L) of the Yuma County Zoning Ordinance.
 - The parcel is approximately 50 feet in width. Therefore, the setback requirement for the side yards is five feet for any new development on the subject property.
-

Attachments

Staff Report

Vicinity map

Site plan

Permit No. B17-0377

Floor plan

Elevation plans

Roof plan

STAFF REPORT
Yuma County Planning and Zoning Division

Prepared for the Hearing of
June 16, 2020
Yuma County Board of Adjustment

CASE NUMBER: Interpretation 20-01

APPLICANT: Barry Olsen, agent for Terry W. Cameron, et. Al.

PREPARED BY: Maggie Castro, AICP, Planning Director

DATE PREPARED: May 5, 2020

DESCRIPTION OF REQUEST

Barry Olsen, agent for Terry W. Cameron, et. Al., requests the following interpretations:

1. Is the Abbott's proposed "New Garage" actually a dwelling unit, or an accessory dwelling unit, under Section 202.00 of the Zoning Ordinance?
2. Is the Abbott's proposed "New Garage" an impermissible expansion of a non-conforming lot/use under Sections 1001.00, et. seq., of the Zoning Ordinance.
3. If the Abbott's proposed "New Garage" is permissible under Sections 602 and 1001, et. seq., of the Zoning Ordinance, what is the required side yard setback for the "New Garage" per Section 1011 of the Zoning Ordinance?

The owners of the property are Tony L. & Kathleen Abbott and Bryan L. Abbott. The property is located at 10343 East North Martinez Lake Road, Assessor Parcel Number 459-50-078, zoned Rural Area-20 acre minimum (RA-20).

Building Permit No. B17-0377 was issued on November 8, 2017 for a new garage. The dimensions of the garage being 41 feet in length, 35 feet in width, and 29.56 feet in height. The approved site plan depicts a structure labeled as existing cabin, a sewage disposal system, and a proposed new garage and the following setbacks for the new garage: Front yard setback of 65.10 feet, east side yard setback of 5.0 feet, and rear yard setback of 34 feet. The approved elevation plans depict the structure is two stories. The approved main

floor plan depicts a garage with two boats, washer, drier, sink and staircase, and the first floor plan depicts a balcony facing south, an open room containing lavatory, bath, and sink, closet, niche, and staircase.

STAFF ANALYSIS

1. Section 202.00 of the Yuma County Zoning Ordinance defines accessory dwelling unit and dwelling unit as follows:
 - Accessory dwelling unit (ADU): A habitable space that contains independent sanitary and cooking facilities on the same parcel as an existing primary dwelling intended to house family or guests of the occupants of the principal dwelling without compensation.
 - Dwelling Unit: One (1) or more rooms within a building arranged, designed or used for residential purposes for one (1) family and containing independent sanitary and cooking facilities. The presence of cooking facilities conclusively establishes the intent to use for residential purposes.

While the approved floor plans for the structure depict an independent sanitary facility, they do not depict an independent cooking facility. Pursuant to Section 202.00, the structure is not intended for residential purposes.

2. Is the Abbott's proposed "New Garage" an impermissible expansion of a non-conforming lot/use under Sections 1001.00, et. seq., of the Zoning Ordinance.
 - Section 1001.00—Purposes of the Yuma County Zoning Ordinance states as follows: This Article is intended to limit nonconforming uses of land, buildings, or structures existing at the time of its effective date by prohibiting or limiting an enlargement, re-establishment after abandonment, or restoration after destruction of the use. Nothing in this ordinance shall affect existing uses of property or the right to its continued use or the reasonable repair or alteration thereof for the purpose for which said use, at the time the ordinance affecting the property, takes effect. The Yuma County Zoning Ordinance originally became effective on December 1, 1975.
 - Section 1003.00--Change from a Nonconforming Use to Another Nonconforming Use of the Yuma County Zoning Ordinance states as follows: An existing nonconforming use of property may not be changed to another nonconforming use of property.
 - Section 1005.00--Change to a Conforming Use of the Yuma County Zoning Ordinance states as follows: A nonconforming use changed to a conforming use shall not thereafter revert to a nonconforming use.

- Section 1006.00--Alterations & Repairs of the Yuma County Zoning Ordinance states as follows: Reasonable repairs and alterations of existing non-conform in uses of property are permit-ted. *(For the purpose of this section, uses in the C-1, C-2, LI, HI, and II Districts shall be considered business use).*
- Section 1008.00--Change in Intensity of the Yuma County Zoning Ordinance states as follows: An increase in the use intensity does not constitute a change in a nonconforming use. However, a change of the use, or addition of new uses, constitutes a use intensification and is not permitted.

The property is zoned RA-20. The use of the property for a single-family dwelling unit (site-built, factory-built, or manufactured) is a permitted use pursuant to Section 601.02—Permitted Uses of the Yuma County Zoning Ordinance. Additionally, accessory buildings and uses customarily incidental to the uses listed in Section 601.02 are allowed such as garages and carports, patios, storage sheds, barns, corrals and arenas, private swimming pools, pool house, tennis courts, fish ponds, walls and fences, signs and parking. The structure permitted with Building Permit No. B17-0377 is an accessory use permitted pursuant to Section 601.02(L) of the Yuma County Zoning Ordinance.

The property is a nonconforming lot. Section 202.00 of the Yuma County Zoning Ordinance defines Nonconforming Lot as follows:

- Lot, Nonconforming: A lot or parcel of land that was of record and lawfully established and maintained but which, because of the enactment of the zoning ordinance or as a result of a land division not authorized by the County, no longer conforms to the land-use standards or use regulations of the zone in which it is located.

Additionally, Black’s Law Dictionary, Eighth Edition, Bryan A. Garner Editor in Chief, defines nonconforming lot as follows: A previously lawful lot that now violates a newly adopted or an amended zoning ordinance.

The property was established in 1971 and is 7,020 square feet in size. Therefore, the property is subject to compliance with Section 1011.00 of the Yuma County Zoning Ordinance.

3. If the Abbott’s proposed “New Garage” is permissible under Sections 602 and 1001, et. seq., of the Zoning Ordinance, what is the required side yard setback for the “New Garage” per Section 1011 of the Zoning Ordinance?

The property owned by Abbott is 7,020 square feet in size and was established in 1971. In part, Section 1011.00—Nonconforming Lots of the Yuma County Zoning Ordinance states as follows: Any lot of record, which does not conform to the minimum lot area or width requirements for the zoning district in which it is located,

may be used for any use permitted in the zoning district. Each side yard setback on lots of nonconforming width may be reduced to ten percent (10%) of the width of the lot, but in no case shall the side yard setbacks be less than five (5) feet on each side...

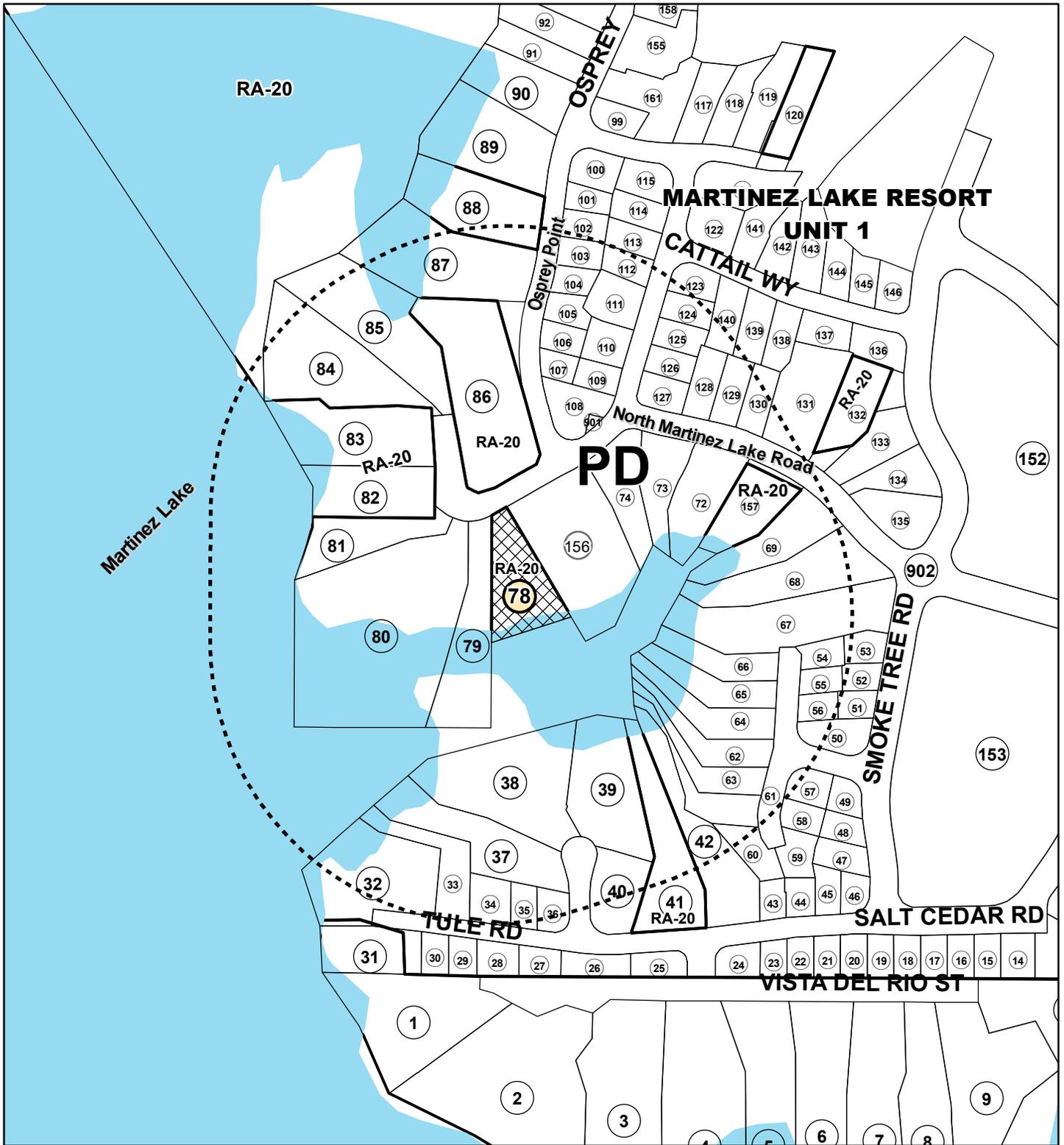
Section 202.00—Definitions of the Yuma County Zoning Ordinance defines lot width as follows:

- Lot Width: The largest horizontal distance between side lot lines, measured along the lines parallel to the lot's road frontage and located behind the minimum front yard setback applicable to the lot and in the front two-thirds of the lot's depth.

Pursuant to the definition of lot width, the parcel is approximately 50 feet in width. Therefore the setback requirement for the side yards is five feet for any new development on the subject property.

CONCLUSION

The proposed New Garage and setbacks are permissible pursuant to Sections 202.00, 1001.00, 1003.00, 1005.00, 1006.00, and 1008.00 of the Yuma County Zoning Ordinance.



DEPARTMENT OF DEVELOPMENT SERVICES
PLANNING & ZONING DIVISION
 2351 W. 26TH STREET
 YUMA, AZ 85364

FOR INFORMATION ONLY - NO LIABILITY ASSUMED

CASE NO: INT20-01
LOCATION: 10343 E. North Martinez Lake Road
APN(s): 459-50-078

CASE PLANNER: Juan Leal Rubio
DATE DRAWN: 05-21-2020
REVIEWED BY: N/A

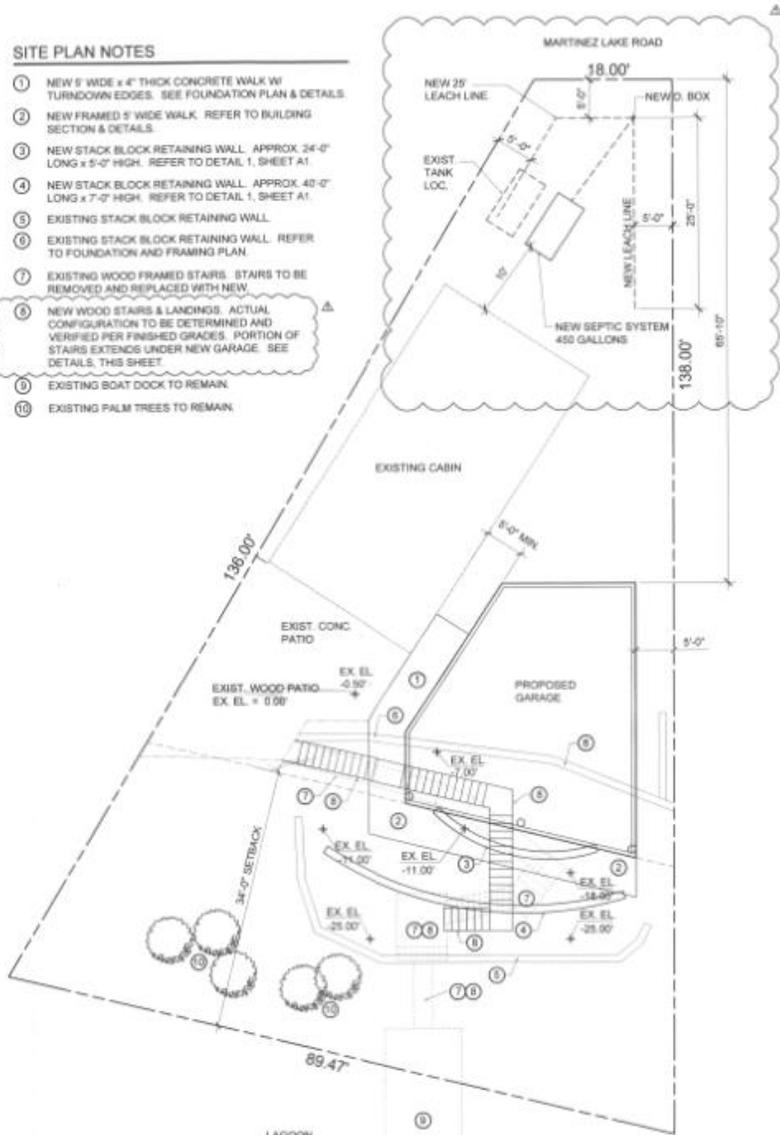
SCALE: 1" = 150'



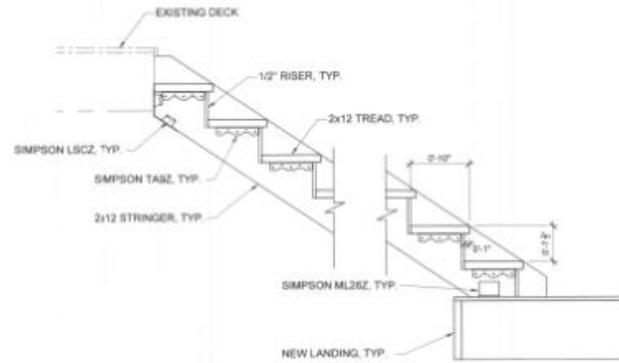
Legend
 300' BUFFER
 SUBJECT PROPERTY
 Zoning Boundary

SITE PLAN NOTES

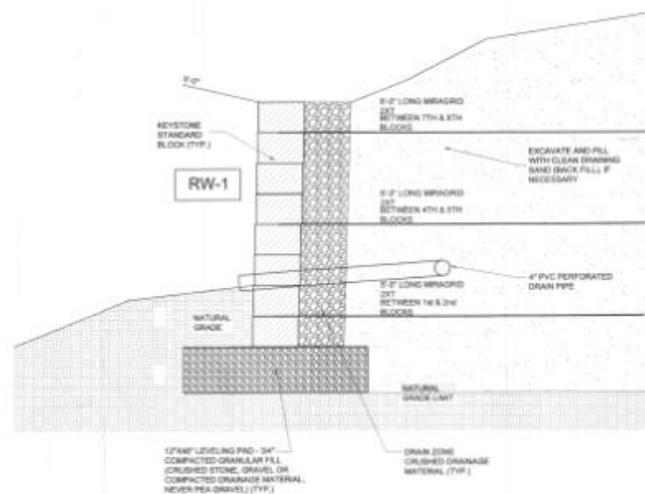
- 1 NEW 5' WIDE x 4" THICK CONCRETE WALK W/ TURNDOWN EDGES. SEE FOUNDATION PLAN & DETAILS.
- 2 NEW FRAMED 5' WIDE WALK. REFER TO BUILDING SECTION & DETAILS.
- 3 NEW STACK BLOCK RETAINING WALL. APPROX. 24'-0" LONG x 5'-0" HIGH. REFER TO DETAIL 1, SHEET A1.
- 4 NEW STACK BLOCK RETAINING WALL. APPROX. 40'-0" LONG x 7'-0" HIGH. REFER TO DETAIL 1, SHEET A1.
- 5 EXISTING STACK BLOCK RETAINING WALL.
- 6 EXISTING STACK BLOCK RETAINING WALL. REFER TO FOUNDATION AND FRAMING PLAN.
- 7 EXISTING WOOD FRAMED STAIRS. STAIRS TO BE REMOVED AND REPLACED WITH NEW.
- 8 NEW WOOD STAIRS & LANDINGS. ACTUAL CONFIGURATION TO BE DETERMINED AND VERIFIED PER FINISHED GRADES. PORTION OF STAIRS EXTENDS UNDER NEW GARAGE. SEE DETAILS, THIS SHEET.
- 9 EXISTING BOAT DOCK TO REMAIN.
- 10 EXISTING PALM TREES TO REMAIN.



SITE PLAN
1/8"=1'-0"



EXTERIOR STAIRS - TYPICAL CONST. DETAILS



STACK BLOCK RETAINING WALL DETAIL - 1
3/4"=1'-0"

REINF. CONCRETE FOUNDATION SPECIFICATION

1. CONCRETE ASTM DESIGNATION C34. NORMAL WEIGHT. MINIMUM SPECIFIED 28 DAYS COMPRESSIVE STRENGTH $f_c = 5,500$ PSI. LIND USE FLY ASH OR SLAG MEETING ASTM C989 & ASTM D316, RESPECTIVELY.
2. MAXIMUM WATER/CEMENT RATIO = 0.4
3. MAXIMUM WATER-SOLUBLE CHLORIDE ION (C) CONTENT IN CONCRETE PERCENT BY WEIGHT OF CEMENT = 0.15 PER ASTM C216
4. CONCRETE EXPOSURE CATEGORY AND CLASS P AND P1.
5. CONCRETE EXPOSURE CONDITIONS, TABLES 4.2.1 & 4.3.1 OF ACI 318.11. P1 & C2 IN CONTACT WITH WATER WHERE LOW PERMEABILITY IS REQUIRED. CONCRETE EXPOSED TO MOISTURE AND AN EXTERNAL SOURCE OF CHLORIDES FROM BRACKISH WATER.
6. REINFORCING STEEL ASTM DESIGNATION A615 GRADE 40 FOR #3 AND SMALLER REBAR AND GRADE 60 FOR #4 AND LARGER.
7. ALLOWABLE FOUNDATION PRESSURE = 2,000 PSF. SOIL CLASS 3.4 PER TABLE 1001.2 OF IBC 2012.
8. ANCHOR BOLTS AT FOUNDATION BELL PLATE: ASTM DESIGNATION A307 ANCHOR STANDARD B18.2.1-1998.
9. CONCRETE MASONRY UNIT MATERIALS PER ASTM C 55 OR ASTM C80, GROUT PER ASTM C 476 & MORTAR MATERIALS PER ASTM C 270.

BUILDING CODES

GENERAL BUILDING CODE	IBC 2012
CONCRETE CODE	ACI 318-11
MINIMUM DESIGN LOADS FOR BUILDING	ASCE 7-10
TIMBER CONSTRUCTION	NDS-2012 WITH SUPPLEMENTS INCLUDING 2010 SPECIAL DESIGN PROVISIONS FOR WIND & SEISMIC (SDPWS)

EARTHQUAKE DESIGN DATA*

RISK CATEGORY	II
SITE CLASSIFICATION FOR SEISMIC DESIGN	D
SEISMIC IMPORTANCE FACTOR, I_w	1
MAPPED SPECTRAL SHORT ACCELERATION, S_s	0.400 g
MAPPED SPECTRAL LONG ACCELERATION, S_L	0.215 g
5 PERCENT DAMPED SPECTRAL RESPONSE ACCELERATION PARAMETER AT SHORT PERIODS, S_{ds}	0.400 g
5 PERCENT DAMPED SPECTRAL RESPONSE ACCELERATION PARAMETER AT 1 SECOND PERIOD, S_{d1}	0.202 g
SEISMIC DESIGN CATEGORY	D

* DATA FROM USGS DESIGN MAPS SUMMARY REPORT, ATTACHED WITH CALCULATIONS

WIND DESIGN DATA

ULTIMATE DESIGN WIND SPEED, V_{ult} (3 SEC GUST)	115 MPH
NORMAL WIND SPEED, V_{ind}	39 MPH
RISK CATEGORY	B
WIND EXPOSURE CATEGORY	C

Manual J/S/D's shall be submitted & approved prior to requesting rough framing inspection

Yuma County Flood Control District
Floor # _____
Date: 11/7/2017
Approved: _____
11-17-2017
17-1175

OSMAN ENGINEERING P.L.L.C.
183 E 24TH STREET, SUITE 6
YUMA, AZ 85364
Tel: (928) 314-1737 FAX: (928) 314-1738

SITE PLAN
OCT 6 2017

NEW GARAGE FOR
TONY & BRYAN ABBOTT
10343 E NIGHTINGALE LANE ROAD
YUMA, ARIZONA

DATE	NO.
10/10/2017	009/2017
DATE	NO.
10/2/2017	002/17
DATE	NO.
TITLE	AS SHOWN
SHEET	OF
A1	12

YUMA COUNTY, ARIZONA

PERMIT

DEPARTMENT OF DEVELOPMENT SERVICES BUILDING SAFETY DIVISION

2351 W. 26th Street

Phone (928) 817-5000

For Inspections Call (928) 817-5196 or Fax (928) 817-5055

Parcel ID #: 45950078

Permit #: B17-0377

Date Issued: 11/08/2017

REVISED COPY
05/13/2019

Owner's Name:	Address:	City & State:	Zip Code:	Phone #:
ABBOTT TONY L & KATHLEEN TRUST 1/2 &	P O BOX 710447	SANTEE CA	92072	(619) 606-0548

Project Address:	Proposed Use:	Type of Construction:
10343 E NORTH MARTINEZ LAKE RD	NEW GARAGE	WOOD

Subdivision:	Lot/Sp #:	Blk #:	Noise Abat Req:	Fld Plain Req:	Zoning:
NO SUBDIVISION			N		RA-20

Contractor's Name:	Address:	City & State:	Zip Code:	Phone #:

BUILDING:	L:	41' W:	35' Ht:	29.56'	Occup Type: R-3	Const Type: VB
Valuation:	\$60,119.12	Total Square Feet:	2839			

FEES:	Electrical	\$75.10	Reinspection	\$0.00	
Plan Check	\$773.99	Mechanical	\$44.00	Fire Code Fees	\$0.00
Building	\$720.75	Other Fees	\$0.00	Additional Fees	\$0.00
Plumbing	\$79.00	Penalties	\$0.00	Total Fees	\$1,692.84

COMMENTS:

1: REVIEWED AND APPROVED REVISIONS

PROVISIONS: The issuance of this permit shall not be construed to release the owner or the owner's agents from the obligation to comply with the provisions of all laws and ordinances, including federal, state and local jurisdictions, which regulate construction and performance of construction. Building permits become null and void if the construction work authorized is not begun within 180 days from date of issue or if at any time prior to final inspection and approval the work is suspended for a period of 180 days. Manufactured home and rehabilitation permits are only valid for 180 days.

I hereby certify that I am the owner or duly authorized owner's agent, that I have read this application and all information is correct. I further certify that I have read, understand and will comply with all of the provisions outlined above. As a condition of this building permit, I agree that the actual construction may be inspected at any time during normal working hours.

Owner or Agent

Signature

Date

Telephone

YUMA COUNTY, ARIZONA

PERMIT

DEPARTMENT OF DEVELOPMENT SERVICES
BUILDING SAFETY DIVISION
 2351 W. 26th Street
 Phone (928) 817-5000
 For Inspections Call (928) 817-5196 or Fax (928) 817-5055

Parcel ID #: 45950078

Permit #: B17-0377

Date Issued: 11/08/2017

Owner's Name: ABBOTT TONY L & KATHLEEN TRUST 1/2 &	Address: P O BOX 710447	City & State: SANTEE CA	Zip Code: 92072	Phone #: (619) 606-0548
--	-----------------------------------	---------------------------------------	---------------------------	-----------------------------------

Project Address: 10343 E NORTH MARTINEZ LAKE RD	Proposed Use: NEW GARAGE	Type of Construction: WOOD
---	------------------------------------	--------------------------------------

Subdivision: NO SUBDIVISION	Lot/Sp #:	Blk #:	Noise Abat Req: N	Fld Plain Req:	Zoning: RA-20
---------------------------------------	------------------	---------------	-----------------------------	-----------------------	-------------------------

Contractor's Name:	Address:	City & State:	Zip Code:	Phone #:
---------------------------	-----------------	--------------------------	------------------	-----------------

BUILDING: L:	41' W:	35' Ht:	29.56'	Occup Type: R-3	Const Type: VB
Valuation:	\$60,119.12	Total Square Feet:	2839		

FEES:		Electrical	\$75.10	Reinspection	\$0.00
Plan Check	\$703.49	Mechanical	\$44.00	Fire Code Fees	\$0.00
Building	\$720.75	Other Fees	\$0.00	Additional Fees	\$0.00
Plumbing	\$79.00	Penalties	\$0.00	Total Fees	\$1,622.34

COMMENTS:

PROVISIONS: The issuance of this permit shall not be construed to release the owner or the owner's agents from the obligation to comply with the provisions of all laws and ordinances, including federal, state and local jurisdictions, which regulate construction and performance of construction. Building permits become null and void if the construction work authorized is not begun within 180 days from date of issue or if at any time prior to final inspection and approval the work is suspended for a period of 180 days. Manufactured home and rehabilitation permits are only valid for 180 days.

I hereby certify that I am the owner or duly authorized owner's agent, that I have read this application and all information is correct. I further certify that I have read, understand and will comply with all of the provisions outlined above. As a condition of this building permit, I agree that the actual construction may be inspected at any time during normal working hours.

Owner or Agent	Signature	Date	Telephone
----------------	-----------	------	-----------

no finalized date
" void "

exp. 1-30-19

appr. 11-03-17

ISSUE 11-08-17

(PRINT ON LETTERHEAD)

459-50-078

B17-0377

Final

06/20/18

Date

✓
* Architectural Comments

Name

Company

Street Address

City, State, Zip

Re: Subject of Letter

Project No: PRMXX-XXX Permit No: BXX-XXXX

Parcel No: XXX-XX-XXX

Dear Applicant(s):

Thank you for your submittal of plans for review. The plan review for the above referenced residential project has been completed. This letter reflects the deficiencies that need to be addressed. As the applicant it is your responsibility to be the only person submitting response to the comments below. We ask that a single cover letter reflecting the corresponding item(s) be attached and resubmit (2) sets of corrected plans with the corrective action taken. Please identify the corrections addressed using clouds, deltas, and revision numbers. This will help with the resubmittal review. We are in day ____ of the 1st, 2nd, 3rd, 4th review. After the 4th review the project will be denied. The review time will resume when all items have been returned. Please return your corrections to the point of contact listed below.

BUILDING SAFETY COMMENTS

(required forms attached)

1. Complete the attached Special Inspection Form for the specified 5000psi concrete. Section 1704 IBC (International Building Code) 2012
2. Please sign and return the attached third party agreement form, for a third party structural review of your project, i.e., a garage with living space above. Section 107 IBC (International Building Code) 2012
3. What is identified as new wood framed stairs (Site Plan Note 8) on site plan seem to be passing through proposed building. Revise or explain. Section R106 IRC 2012
4. Provide a door between garage and living space above that meets the requirements of Section R302.5.1 IRC (International Residential Code) 2012
5. Provide a note or place in a detail the requirement for under-stair protection. Section R302.7 IRC 2012
6. Provide a note or place in a detail the requirement for 5/8" Type X gypsum on ceilings between a garage and habitable space above. Section and Table R302.6 IRC 2012

7. Trusses shall be designed to carry all loads (A/C unit and spiral stair) imposed on them. Section R802.10 IRC 2012
8. Provide detail for connection of spiral stair, to building, at base and upper level landing. Section R106 IRC 2012
9. Provide Manual J/S/D for this project. Section R106 IRC 2012
10. Provide notes and/or details showing compliance with the minimum energy efficiency requirements of Chapter 11 (as amended) IRC 2012
11. Glazing in hazardous locations (2'x2' in bathroom) shall be labeled, tempered or safety glass, on plans. Section R308.4.5
12. Elevation measurements on Sheet A3 do not match the scale given. Revise or explain. Section R106 IRC 2012
13. Provide a Detail/Section showing exterior wall, roof and deck finishes. Section R106 IRC 2012
14. Provide detail for guardrail to show compliance with Table R301.5 and Section R312 IRC 2012
15. Provide a carbon monoxide alarm at top of stairs. Section R315 IRC 2012
16. In one line diagram on Sheet E1, show in diagram or note the requirement for a grounding electrode at the sub-panel. Section E3607.3 IRC 2012
17. Show location of water heater. Section R306.4 IRC 2012

ENVIRONMENTAL PROGRAMS COMMENTS

(required forms attached)

1. Insert any corrections required.
2. Correction (a)
3. Correction (b)

ENGINEERING COMMENTS

1. Insert any corrections required.
2. Correction (a)
3. Correction (b)

FLOOD CONTROL COMMENTS

1. Insert any corrections required.
2. Correction (a)
3. Correction (b)

PLANNING & ZONING COMMENTS

1. Insert any corrections required.
2. Correction (a)
3. Correction (b)

RURAL/METRO COMMENTS

1. Insert any corrections required.
2. Correction (a)
3. Correction (b)

It is our goal to assist you with approval of your plans in as timely a manner as possible. Please be aware if no response, addressing all comments is made within 180 days from the date of this notice the application will expire. Upon expiration, the plan review fees will be owed and all other fees paid are non-refundable unless otherwise requested per the current refund policy. If you wish to continue with the project a new project submittal will be required including all applicable fees. Please feel free to contact the below named staff member if you have any questions or require further assistance and thank you in advance for your cooperation with this matter.

Sincerely,

Yuma County Plan Review Team
Name, Title (928) 817-XXXX

Enclosure:

cc: Name and Street Address of others that require a copy.

August 4, 2017

Tony Abbott
10343 E North Martinez Lake Rd
Yuma AZ 85367

Re: Garage
Project No: PRM17-1175 Permit No: B17-0377
Parcel No: 459-50-078

Dear Applicant(s):

Thank you for your submittal of plans for review. The plan review for the above referenced residential project has been completed. This letter reflects the deficiencies that need to be addressed. As the applicant it is your responsibility to be the only person submitting response to the comments below. We ask that a single cover letter reflecting the corresponding item(s) be attached and resubmit (2) sets of corrected plans with the corrective action taken. Please identify the corrections addressed using clouds, deltas, and revision numbers. This will help with the resubmittal review. We are in the 1st, 2nd, 3rd, 4th review. After the 4th review the project will be denied. The review time will resume when all items have been returned. Please return your corrections to the point of contact listed below.

BUILDING SAFETY COMMENTS

(required forms attached)

1. THE POTENTIAL FOR A THIRD PARTY STRUCTURAL REVIEW OF THIS PROJECT HAS BEEN DISCUSSED WITH THE EOR (ENGINEER OF RECORD) HE ASKED FOR US TO DO A REVIEW, GIVE HIM OUR COMMENTS AND LET HIM ADDRESS THEM PRIOR TO THIRD PARTY REVIEW, THEREBY ELIMINATING SOME OF THE CONCERNS. SO, THE EOR WAS GIVEN A COMBINATION OF COMMENTS AND REDLINES ON PLANS 8/2/17 IN ORDER TO ADDRESS THOSE ITEMS. THE BULK OF THE COMMENTS BELOW ARE ARCHITECTRURAL, IN NATURE, NOT STRUCTURAL. THE STRUCTURAL COMMENTS ARE WITH THE APPLICATION.
2. Complete the attached Special Inspection Form for the specified 5000psi concrete. Section 1704 IBC (International Building Code) 2012
3. Please sign and return the attached third party agreement form, for a third party structural review of your project, i.e., a garage with living space above. Section 107 IBC (International Building Code) 2012
4. What is identified as new wood framed stairs (Site Plan Note 8) on site plan seem to be passing through proposed building. Revise or explain. Section R106 IRC 2012
5. Provide a door between garage and living space above that meets the requirements of

- Section R302.5.1 IRC (International Residential Code) 2012
6. Provide a note or place in a detail the requirement for under-stair protection. Section R302.7 IRC 2012
 7. Provide a note or place in a detail the requirement for 5/8" Type X gypsum on ceilings between a garage and habitable space above. Section and Table R302.6 IRC 2012
 8. Trusses shall be designed to carry all loads (A/C unit and spiral stair) imposed on them. Section R802.10 IRC 2012
 9. Provide detail for connection of spiral stair, to building, at base and upper level landing. Section R106 IRC 2012
 10. Provide Manual J/S/D for this project. Section R106 IRC 2012
 11. Provide notes and/or details showing compliance with the minimum energy efficiency requirements of Chapter 11 (as amended) IRC 2012
 12. Glazing in hazardous locations (2'x2' in bathroom) shall be labeled, tempered or safety glass, on plans. Section R308.4.5
 13. Elevation measurements on Sheet A3 do not match the scale given. Revise or explain. Section R106 IRC 2012
 14. Provide a Detail/Section showing exterior wall, roof and deck finishes. Section R106 IRC 2012
 15. Provide detail for guardrail to show compliance with Table R301.5 and Section R312 IRC 2012
 16. Provide a carbon monoxide alarm at top of stairs. Section R315 IRC 2012
 17. In one line diagram on Sheet E1, show in diagram or note the requirement for a grounding electrode at the sub-panel. Section E3607.3 IRC 2012
 18. Show location of water heater. Section R306.4 IRC 2012

it is our goal to assist you with approval of your plans in as timely a manner as possible. Please be aware if no response, addressing all comments is made within 180 days from the date of this notice the application will expire. Upon expiration, the plan review fees will be owed and all other fees paid are non-refundable unless otherwise requested per the current refund policy. If you wish to continue with the project a new project submittal will be required including all applicable fees. Please feel free to contact the below named staff member if you have any questions or require further assistance and thank you in advance for your cooperation with this matter.

Sincerely,

Yuma County Plan Review Team
Esmeralda Alvarez, Senior Permit Tech (928) 817-5117

- Section R302.5.1 IRC (International Residential Code) 2012
6. Provide a note or place in a detail the requirement for under-stair protection. Section R302.7 IRC 2012
 7. Provide a note or place in a detail the requirement for 5/8" Type X gypsum on ceilings between a garage and habitable space above. Section and Table R302.6 IRC 2012
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Sincerely,

Yuma County Plan Review Team
Esmeralda Alvarez, Senior Permit Tech (928) 817-5117

(PRINT ON LETTERHEAD)

~~A~~ Structural Comments

Date

PRM17-1175

Name
Company
Street Address
City, State, Zip

Re: Subject of Letter
Project No: PRMXX-XXX
Parcel No: XXX-XX-XXX

Permit No: BXX-XXXX

These Building Safety comments are not exhaustive. We stopped at the point we determined a third party review would be required.

Dear Applicant(s):

Thank you for your submittal of plans for review. The plan review for the above referenced residential project has been completed. This letter reflects the deficiencies that need to be addressed. As the applicant it is your responsibility to be the only person submitting response to the comments below. We ask that a single cover letter reflecting the corresponding item(s) be attached and resubmit (2) sets of corrected plans with the corrective action taken. Please identify the corrections addressed using clouds, deltas, and revision numbers. This will help with the resubmittal review. We are in day ____ of the 1st, 2nd, 3rd, 4th review. After the 4th review the project will be denied. The review time will resume when all items have been returned. Please return your corrections to the point of contact listed below.

BUILDING SAFETY COMMENTS

(required forms attached)

1. Complete the attached Special Inspection Form for all required items pertaining to this project. Section 1704 IBC (International Building Code) 2012
2. The Site Plan seems to show new wood framed stairs running through building. Revise or explain. Section R106 IRC (International Residential Code) 2012
3. Provide documentation to show 'Keystone' stack wall can support the load of proposed garage. Section 1807.2 IBC 2012
4. Footing F1 is assigned a 16" depth on Footing Schedule, while the 16" number describes it's thickness, not depth. It's depth is not shown. Revise or explain. Section 107 IBC 2012
5. Sheet S1 cites Detail 2/S2, which shows a footing running parallel to and bearing on a retaining wall. Unable to identify where that situation occurs. Revise or explain. Section 107 IBC 2012
6. The design of the footing in Detail 2/S2 does not have a corresponding location in the Footing Schedule and does not show any vertical reinforcement, which would lend itself

- to failure in a lateral direction. Revise or explain.
7. Cross section X-X on Sheet S2 has #3 ties at 12" o.c., while standard engineering principles typically show #3 ties at 8" o.c. with 3 ties within the top 5" of column w/ seismic hooks on all ties. Revise or explain. Section 107 IBC 2012
 8. The lack of a moment frame at the main garage door opening will create a 'soft story'. Please provide specific documentation that an SSW18X9 can support, on its own, the combined loads of both levels of the structure. Section 1604 IBC 2012
 9. For clarity sake, describe building levels as 1 and 2, as there is not 3, usable, levels. Or describe rooftop deck as 3rd level. Section R106 IRC 2012
 10. Based on the required 16" end length of the CS 14 strap, designated as holdown A, on shearwall plan, how is the upper level shear load transferred to lower level. Revise or explain. Section 1604.4 IBC 2012
 11. Holdown E in schedule is labeled as MSTC66B3, while the Simpson catalog labels it w/out the B3 and does not list 14-10d nails as a fastening option. Also, provide detail to show how that strap will connect to both levels. Section 107 IBC 2012
 12. Explain or revise why LBW abbreviation is used only at SW 1-1 near garage front. Section 107 IBC 2012
 13. The Simpson catalog calls out 7/8" diameter anchors for both the HD7B and the HD9B, while the holdown schedule has specified a PAB8 for the HD9B, which has a 1" diameter. Revise or explain. Section 107 IBC 2012

ENVIRONMENTAL PROGRAMS COMMENTS

(required forms attached)

1. Insert any corrections required.
2. Correction (a)
3. Correction (b)

ENGINEERING COMMENTS

1. Insert any corrections required.
2. Correction (a)
3. Correction (b)

FLOOD CONTROL COMMENTS

1. Insert any corrections required.
2. Correction (a)
3. Correction (b)

PLANNING & ZONING COMMENTS

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2. Correction (a)
3. Correction (b)

RURAL/METRO COMMENTS

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2. Correction (a)
3. Correction (b)

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Sincerely,

Yuma County Plan Review Team
Name, Title (928) 817-XXXX

Enclosure:

cc: Name and Street Address of others that require a copy.

REQUEST FOR EXTENSION

Date: 1/17/2019

Permit Number: B17-0377

Owner's Name: Tony, Kathleen & Bryan Abbott

Property Address: ~~10433~~ 10343 E. North Martinez Lake

Phone Number: 619-770-9549

Date of Last Inspection: 8/3/18 Colt Bacon

Desired Extension Date: 12/31/2019

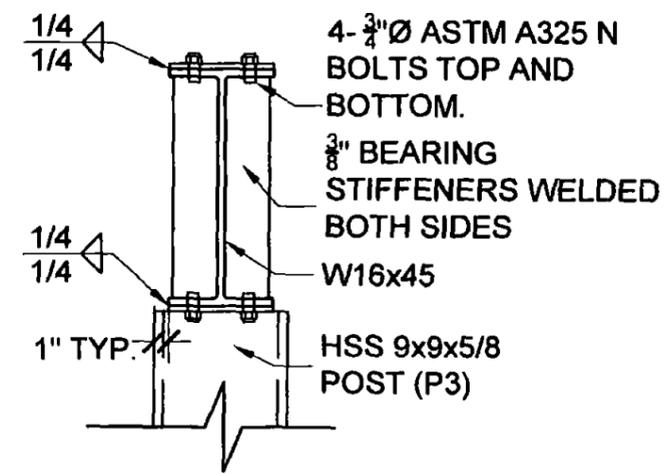
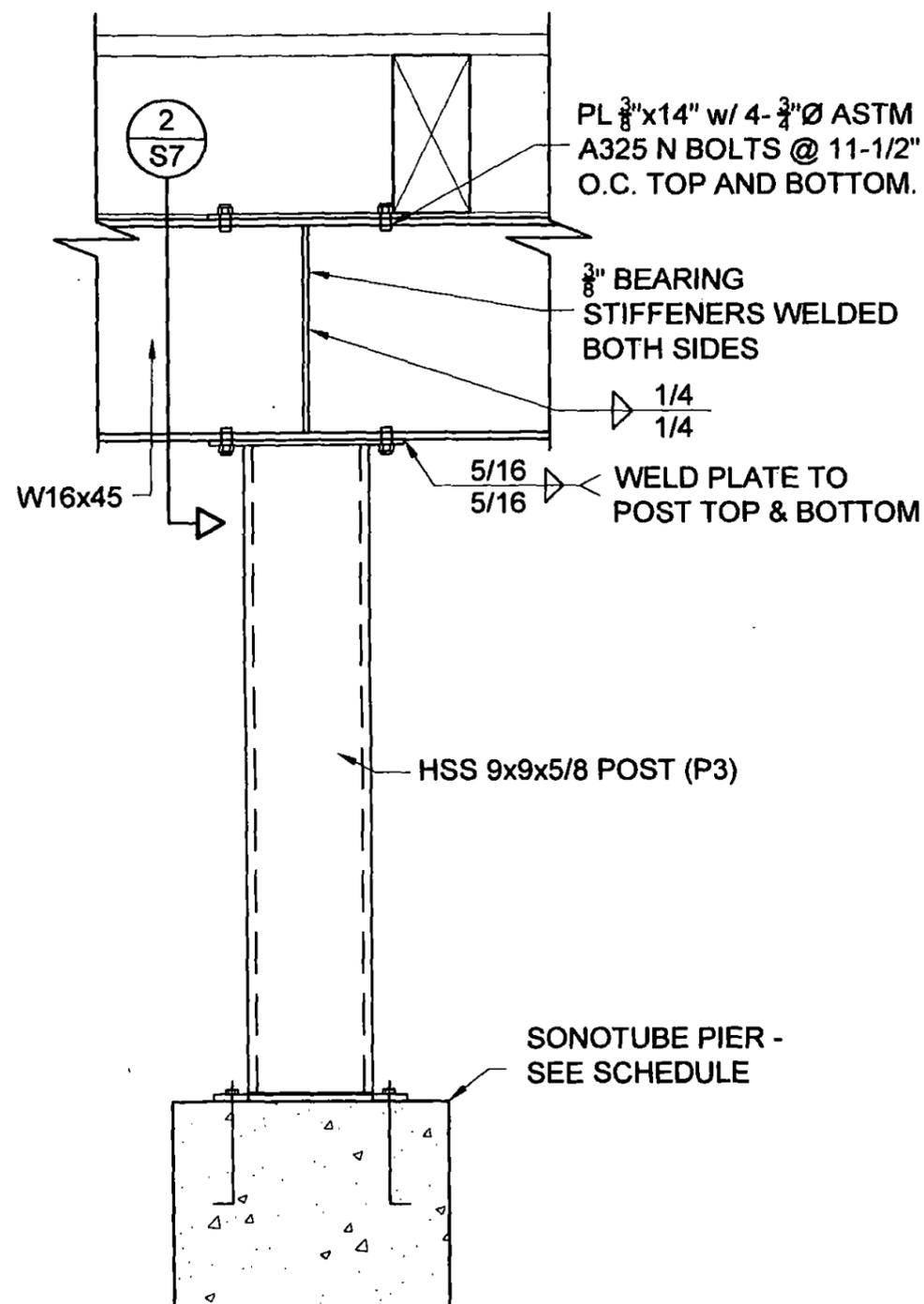
Reason For Extension: Have been in a lawsuit

with neighbor which has delayed

our progress.

Approved: Denied:

Entered By: [Signature] Date: 1/17/19



DETAIL - 2
SCALE: 3/4" = 1'-0"

DETAIL - 1
SCALE: 3/4" = 1'-0"

DESCRIPTION	REVISION PER OWNER'S REQUEST
DATE	05/23/2019
REV.	A

OSMAN ENGINEERING PLLC
183 E 24TH STREET, SUITE 6 YUMA, AZ 85364
Tel: (928) 314-1737 FAX: (928) 314-1738

COLUMN CONNECTION DETAILS

NEW BUILDING ADDITION
TONY ABBOTT RESIDENCE
10343 E NORTH MARTINEZ LAKE ROAD
YUMA, ARIZONA

DSGN. I.O.	DATE 5/3/2019
DRWN. E.R.	PROJ. SD 2-17
CRD. I.O.	SCALE: AS SHOWN

SHEET OF
S7 13

OSMAN ENGINEERING PLLC

STRUCTURAL ENGINEERING

July 14, 2017

RECEIVED
JUL 14 2017
BY: 

Permit Technician
Department of Development Services
Building Safety Division, Yuma County
2351 W. 26th Street
Yuma, AZ 85364

Re: New Garage
Project Number: PRM17-1175
Permit Number:
Parcel No. 459-50-078

In reference to your comments on the letter dated 6/23/2017, please find below our responses enclosed with two sets of the revised site plan with revisions clouded.

BUILDING SAFETY COMMENTS:

1. Manual J, S and D calculations are required at time of submittal. 2012IRC R106.1
: Refer to revised Plan M1.
2. Provide pre-engineered stairway specifications and design documents. 2012IRC R106.1 : Refer to stairway detail plan S1.

ENVIRONMENTAL PROGRAMS COMMENTS:

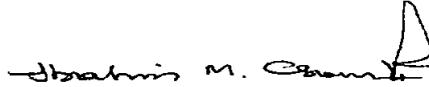
1. Provide existing septic system location and/or proposed septic system location on site
Plan.: Refer to revised Site Plan A1.

FLOOD CONTROL COMMENTS:

1. A flood plain use permit and elevation certificate are required for this project:
 - Attached: Original & copy of permit application.
 - Attached: Original of Elevation Certificate.

Please contact me at the address below should you have any questions.

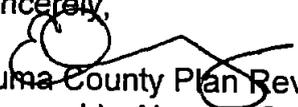
Thank you,



Ibrahim M. Osman, P.E.
Principal/Member

questions or require further assistance and thank you in advance for your cooperation with this matter.

Sincerely,


Yuma County Plan Review Team
Esmeralda Alvarez, Senior Permit Tech (928) 817-5117

OSMAN ENGINEERING PLLC

STRUCTURAL ENGINEERING

November 2, 2017

Esmeralda Alvarez
Senior Permit Technician
Yuma County Plan Review Team
Department of Development Services
Building Safety Division
2351 W. 26th Street
Yuma, AZ 85364

RECEIVED
NOV 02 2017
BY:.....

Re: New Garage-Tony Abbott
Project Number: PRM17-1175
Permit Number: B17-0377
Parcel No. 459-50-078

Dear Esmeralda:

In reference to your comments on the letter dated 10/16/2017, please find enclosed herewith our response with two sets of the revised plans with revisions clouded.

BUILDING SAFETY COMMENTS:

1. Special inspection form for 5,000 psi concrete is included.
2. 3rd floor truss layout with trusses that support the roof-mounted AC equipment: T2, T3, T4 and T5 are attached.
3. The circular tie detail is shown on details X-X and 9 in S2.
4. Mechanical connection schedule has been revised for consistency with loads and dimensions.
5. Ventilation calculations added to A3.

Please contact me at the address below should you have any questions.

Thank you,


Ibrahim M. Osman, P.E.
Principal/Member



183 E 24th Street, Suite 6
Phone (928) 314-1737

Yuma, AZ 85364
Fax: (928) 314-1738

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STRUCTURAL ENGINEERING

October 3, 2017

Permit Technician
Department of Development Services
Building Safety Division, Yuma County
2351 W. 26th Street
Yuma, AZ 85364

Re: New Garage
Project Number: PRM17-1175
Permit Number:
Parcel No. 459-50-078

In reference to your comments on the letter dated 8/23/2017, please find below our responses enclosed with two sets of the revised site plan with revisions clouded.

Structural Building Safety Comments:

1. Attached is the signed and sealed special inspection form.
2. For the stairs refer to architectural response letter.
3. For Keystone vertical load capacity refer to attached documents (two pages of previously approved stack wall and keystone design charts) and page 3 of hand calculations of CF4 that shows the axial capacity.
4. Footing F1: refer to S1
5. Refer to S1 for location of CF4 and details 2/S2 and 5/S2.
6. For comment "6" refer to response "5" above.
7. Refer to revised section X-X on S2
8. Refer to S3, S6 and attached spreadsheet for the design and detail of the special steel moment frame (SSMF) and its connections.
9. Building levels have been identified to facilitate plan review.
10. CS14 strap
11. Hold-down E name and fastening have been corrected.
12. LBW refers to load-bearing walls. Emphasis to indicate LBW at the 1st floor indicates the 2-2X6 stud of the LBW in shear walls 1 and 2.
13. Simpson anchors have been revised, refer to hold-down schedules in S1 and S3.
14. Note that the size of CF1 has not been changed since the lateral load value remains the same.

15. Calculations comment on page 1: 5/8" type X gyp board has been considered. Refer to architectural plans.
16. Calculations comment on page 5: all floor panels are 1-1/8" T&G OSB except at roofs non-balcony areas which use 1/2" OSB. Refer to Notes on S4.
17. Calculations comment on page 14 is not valid since the strong wall has been replaced by the SSMF.
18. Calculations comment on page 15: refer to S1 and S2.
19. Calculations comment on page 16: refer to detail on S4.
20. Calculations comment on page 23 of hand calculations: gravity loads have been taken to account for boat front axles as provided by the owner.
21. Calculations comment on page 2 of foundation hand calculations: note that the strong wall has been replaced by a SSMF.
22. Hand calculations comment on page 3 of foundation: the 5,000-psi concrete applies only for F1 and F2. Refer to revised S1

Architectural Building Safety Comments:

1. Refer to Engineering comments and responses.
2. Refer to attached completed Special Inspection Form for 5,000psi concrete.
3. Refer to Engineering comments and responses.
4. The proposed exterior stairs indicated on site plan are designed to pass under the new garage. Refer to Site Plan Note 8.
5. Door and additional wall has been added to separate garage from upper living area. Refer to revised sheet A2.
6. Refer to note added to sheet A2.
7. Refer to note added to sheet A2.
8. Refer to Engineering comments and responses.
9. Refer to details, sheet S1.
10. Refer to Mechanical, Plumbing and Electrical Notes on sheets M1 and E1.
11. Refer to Energy Inspection Checklist, Sheet M1.
12. Bathroom window has been labeled with safety glass. Refer to sheet A2
13. Refer to revised elevations, sheet A3.
14. Refer to typical wall section, sheet A2.
15. Refer to detail 2, sheet A2.
16. Refer to carbon monoxide alarm added to sheet E1.
17. Refer to note added to sheet E1.
18. Refer to sheet M1 for water heater location & note.

Please contact me at the address below should you have any questions.

Thank you,

Ibrahim M. Osman
 Ibrahim M. Osman, P.E.
 Principal/Member



183 E 24th Street, Suite 6
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Yuma, AZ 85364
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Enclosed:

One page of previously-approved stack wall

One page of Keystone design chart

Six pages of spread sheet of design of special steel moment frame

Three pages of base plate connection

Three pages of design of CF4 (grade beam)

Eight pages of design of lateral forces on structural members of bottom floor

Load diagram of B2

Simpson Strong-Tie PAB Anchor Bolt Data

Revised Elevation Certificate

OSMAN ENGINEERING PLLC

STRUCTURAL ENGINEERING

February 6, 2018

Yuma County
Department of Development Services
Building Safety Division
2351 W. 26th Street
Yuma, AZ 85364

Re: New Garage-Tony Abbott
Project Number: PRM17-1175
Permit Number: B17-0377
Parcel No. 459-50-078

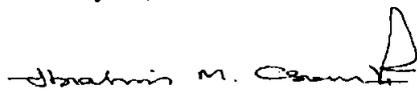
Subject: Clarification of Stack-Block Backfill Construction

In reference to construction inspection of the stack-block retaining wall, please find below my recommendation.

1. Average 1" diameter gravel, with no organic content such as tree roots, wood or other evidence of living organisms, can be used in lieu of the clean sand as such material will provide better drainage at the back of the wall eliminating hydrostatic pressure from building behind the wall.
2. Slurry can be used at the lower part of the wall, adjacent to waterline to a level of at least 12" above the high-water mark elevation. Gravel can be used above the slurry per No. 1.
3. Plans sealed and stamped by me on 9/29/2017 do not call for testing of soils.
4. Pinning of undisturbed soil within the geogrid layers can be done using #4 rebar seismic hoop (with 135°) with XP or SET Simpson Epoxy at 18" OC embedded into the hard soil or rock to 6".

Please contact me at the address below should you have any questions.

Thank you,


Ibrahim M. Osman, P.E.
Principal/Member

SALTER SPIRAL STAIRS TECHNICAL SPECIFICATIONS

Exterior Galvanized Spiral Stair Technical Specifications



Exterior Galvanized Special High Turn Stair with narrow profile 16.36° Treads

Code Stair Package

Benefits of a Code Package

Designed for applications regulated by the national building codes. See our Codes Specifications section for further information.

Please check with your local building inspector for the dimensional requirements in your area before placing your order. The smallest stair available to still maintain the building code is 5' diameter. Salter Spiral Stair manufactures Code Stair Packages in 5', 5'6" and 6' diameter. Code Stair Packages also include extra center balusters for a maximum spacing of 4" (*safety feature for stairs that will be used by small children to prevent pass through*).

*Verify if your local code enforcement requires Code Riser treads, which are sold separately.

Code Packages Include:

(Metal Stairs) 30° stair treads; 3 to 4 balusters per tread (*maximum spacing of 4"*); 60° platform (*This will provide a minimum of 6'6" headroom*); 1 1/2" aluminum handrail with end caps; Platform railing, 1" x 1" frame with 1" round balusters; Center column, base plate and top cap; 40" tall column extension

Diameter	Recommended Finished Well Opening*	Platform Size	Clear Walking Path
5'	62" x 62"	31" x 31"	26"
5'6"	68" x 68"	34" x 34"	29"
6'	74" x 74"	37" x 37"	32"

* Minimum opening is 2" smaller than the recommended opening size. Talk to your salesperson for further information.

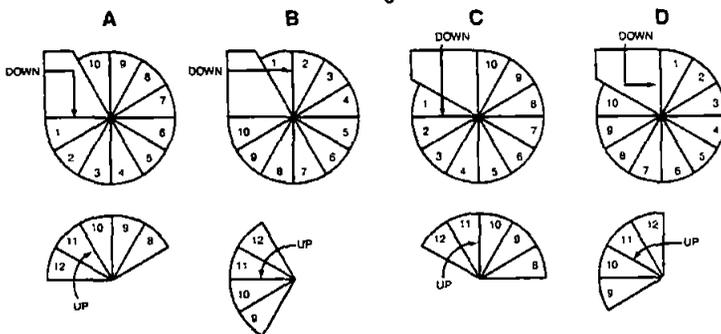
30° Tread for Stair Sizes

5' | 5'6" | 6'

Floor to Floor Height Tread Adjustment		Treads & Risers Required		Degree of Rotation
Min.	Max.	Treads	Risers	
85"	95"	9	10	270°
93 1/2"	104 1/2"	10	11	300°
102"	114"	11	12	330°
110 1/2"	123 1/2"	12	13	360°
119"	133"	13	14	390°

NOTE: A special 60° platform is supplied with this package. The platform is not as versatile as a square one. For ordering purposes use these diagrams to choose the appropriate layout. Available sizes are 5', 5'2", 5'6" and 6" diameters with custom and larger sizes available upon request.

Code Package - 30°



EXTERIOR GALVANIZED STAIRS TECHNICAL SPECIFICATIONS

Standard Stair Package

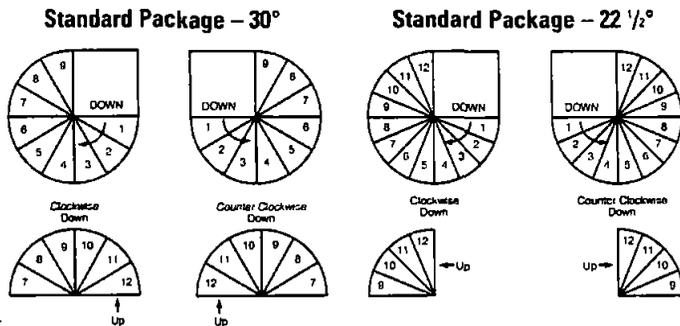
(Does Not Meet Code Compliancy)

Standard Packages Include:

(Metal Stairs) Center column, base plate and top cap; 1" outside diameter steel balusters; adjustable treads; 1 1/2" outside diameter handrail; Platform railing, 1" x 1" frame with 1" round balusters, platform (square); 40" tall column extension

Diameter	Recommended Finished Well Opening*	Platform Size	Clear Walking Path
3'6"	44" x 44"	22" x 22"	17"
4'	50" x 50"	25" x 25"	20"
4'6"	56" x 56"	28" x 28"	23"
5'	62" x 62"	31" x 31"	26"
5'6"	68" x 68"	34" x 34"	29"
6'	74" x 74"	37" x 37"	32"
6'6"	80" x 80"	40" x 40"	35"

* Minimum opening is 2" smaller than the recommended opening size.
Talk to your salesperson for further information.



30° Tread for Stair Sizes

3'6" | 4' | 4'6" | 5' | 5'6" | 6'

Floor to Floor Height Tread Adjustment		Treads & Risers Required		Degree of Rotation
Min.	Max.	Treads	Risers	
85"	95"	9	10	270°
93 1/2"	104 1/2"	10	11	300°
102"	114"	11	12	330°
110 1/2"	123 1/2"	12	13	360°
119"	133"	13	14	390°

22 1/2° Tread for Stair Sizes

5' | 5'6" | 6' | 6'6"

Floor to Floor Height Tread Adjustment		Treads & Risers Required		Degree of Rotation
Min.	Max.	Treads	Risers	
82 1/2"	95"	10	11	225°
90"	102"	11	12	247 1/2°
97 1/2"	110 1/2"	12	13	270°
105"	119"	13	14	292 1/2°
112 1/2"	127 1/2"	14	15	315°

NOTE: To determine the number of treads and risers needed for your project, measure your finished floor to floor height and refer to the above charts. Since the standard stair packages use either 30° or 22 1/2° treads, use the appropriate chart. Custom sizes available upon request.

Special Half-Turn Stair Package

(Does Not Meet Code Compliancy)

Diameter	Recommended Finished Well Opening	Platform Size	Clear Walking Path
5'6"	36" x 67"	N/A	28.5"
6'6"	42" x 79"	N/A	31.5"

Special Half-Turn Packages Include:

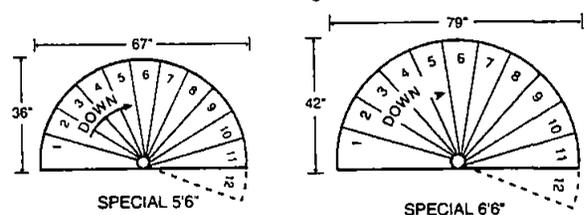
(Metal Stairs) Center Column, base plate and top cap; 1" outside diameter balusters; Adjustable treads; 1 1/2" outside diameter handrail; Platform railing, 1" x 1" frame with 1" round balusters; 38" tall column extension; 180° wood balcony bracket

16.36° Tread for Stair Sizes

5'6" | 6'6"

Floor to Floor Height Tread Adjustment		Treads & Risers Required		Degree of Rotation
Min.	Max.	Treads	Risers	
90"	102"	11	12	180°
97 1/2"	110 1/2"	12	13	196 1/3°

Half-Turn Package - 16.36°



NOTE: The half-turn does not require a platform

REVISIONS AT BOTTOM FLOOR
STRUCTURAL ENGINEERING DESIGN CALCULATIONS FOR BEAM B2, POST P3,
COLUMN P2 & FOOTING F1
TONNY ABBOT'S GARAGE
10343 E N MARTINEZ LAKE ROAD, YUMA, AZ 85365



Prepared by:

OSMAN ENGINEERING PLLC

183 E 24th Street, Suite 6
Yuma, Arizona 85364

May 3, 2019

*Tony Abbott Garage Revisions
Structural Design Calculations
10343 E N M. Lake Road, Yuma
May 3, 2019
Project No. SD 1-19*

Revisions at Bottom Floor
Beam B2, Post P3, Column P2 & Footing F1
Tony Abbott's Garage
10343 E N Martinez Lake Road, Yuma, AZ 85365

STRUCTURAL DESIGN CALCULATIONS

*Tony Abbott Garage Revisions
Structural Design Calculations
10343 E N M. Lake Road, Yuma
May 3, 2019
Project No. SD 1-19*

Content	Number of Pages
Cover page	1
Design Criteria & Content	3
Structural Design of Beam, Post, Column and Footing	5
Total number of pages	9

Tony Abbott Garage Revisions
 Structural Design Calculations
 10343 E N M. Lake Road, Yuma
 May 3, 2019
 Project No. SD 1-19

DESIGN CRITERIA

Feature	Design Data
Building Codes	2012 IBC, Minimum Design Loads for Building and Other Structures ASCE 7-10, and ACI 318-11
Design Method	Allowable Stress Design
Structural Concrete	ASTM C94 and $f'_c = 2,500$ psi and 5,000 psi for Footing F1 Special Inspection Required
Reinforcing Steel	ASTM A615 Grade 40 for # 4 and smaller ASTM A615, Grade 60 for #5 and larger
Soil, All values from Table 1806.2 IBC 2012	Foundation Vertical Pressure = 2,000 psf Lateral Bearing Pressure = 200 psf/ft Coefficient of Friction = 0.40 Soil Class 4 Type of footing: Spread
Design Load All References per ASCE 7-10 UNO	Basic Wind speed, V_{3S} : 115 mph, 3-sec, Equivalent Wind Speed, V_{fm} (Figure 26.5-1A): 89 mph, Gust Exposure D Wind Design: Simplified, Chapters 28 and 30
Seismic Design Category	D

1- Situation

Re-design of B2 at the bottom level after cancelling the middle post per owner request. 2 HSS posts will rest on 2 concrete columns instead of 3. 2 Footings will be resized for the new load situation.

2- Vertical Loads:

Floor D.L., psf	30.00	Effective Wind Area, ft ²	135
Floor L.L., psf	40.00	ASD Out of plane wind, plf	101.934
Weight of wall, psf	15.00	Wind Exposure	D

3- Design of Beam B2:

Simple Span, ft	30.00	
Cantilevered Span, ft	5.58	
Tributary width, ft	10.00	
Wall height, ft	12.50	
U.D. Dead Load, klf	0.345	
Wall supported by B2, klf	0.188	
U.D. Live Load, klf	0.588	
Total D+L udl, klf	0.933	
Max Shear Force, kips	19.675	
Smaller reaction, kips	13.504	
Location of Max BM, ft	14.48	from left end
Max positive DL B.M. k-ft	36.17	
Max positive LL B.M. k-ft	61.60	
Total D+L BM, klf	97.77	
Out-of-plane BM, k-ft	11.47	
λ Adjustment Factor, wind	1.40	

Table 3-1: Beam Section Properties & Material Specs:
Ref Table 1-1 AISC Manual 2015

Section	W16X45
ASTM	A992
weight, plf	45.00
d, in	16.10
A, in ²	13.30
t _w , in	0.345
b _f , in	7.04
t _f , in	0.565
Z _x , in ³	82.30
I _x , in ⁴	586.00
I _y , in ⁴	32.80
F _y , ksi	50.00
E, ksi	29,000.00
Z _y , in ³	14.5
k _{design} , Table 1-1, page 22 AISC Manual	0.967
Ω safety factor for bending	1.67
Allowable Major flexural capacity, k-ft	205.34
Allowable Minor flexural capacity, k-ft	36.18
Interaction of biaxial loading	0.793

Use W16x45 for B2

4- Design of P3:

HSS Section	HSS9x9x5/8
Total axial D.L. load, from previous calculations, kips	13.971 adjusting for 2 columns
Total axial L.L. load, from previous calculations, kips	36.456 adjusting for 2 columns
Total ASD axial compressive load, D+L, kips	50.427 < 505.00 OK
Effective length factor, K	1
Maximum height of column, h, ft	4
Axial capacity, kips	505 AISC Manual Table 4-4, page 4-55
Bending moment on post-from previous calcs for 2 posts	103.55 < 133.00 OK
Flexural bending capacity, k-ft	133 AISC Manual Table 3-13, page 3-147
Interaction of biaxial bending	0.828 < 1 OK

Use HSS9x9x5/8 for P3

5- Design of Moment Connection:

Moment connection: 1/21/2"X8" Plate welded to top and bottom flanges of beam with 6-3/4"φ H.S. bolts
Bolt hole type: STD

Table 5-1: Required Forces

Parameter	Value
Tension force at beam flange, Kips	79.987
Shear force, kips	19.675

Table 5-2: Flange Top and Bottom Plate:

Fillet Weld Strength, F_{EXX} , ksi	70		
Plate specified minimum yield stress, F_y , ksi	36		
Plate specified minimum tensile stress, F_y , ksi	58		
Ω tensile rupture in net section safety factor	2		
Ω tensile yielding in gross section safety factor	1.67		
Minimum area based on yielding of gross section, in^2	3.711		
Selected plate length, in	10.00		
Selected plate thickness, in	0.38		
Selected plate area, in^2	3.75	>	3.711 OK
Bearing capacity at bolt holes based on spacing, kips	19.575		
Bearing capacity at bolt holes based on edge distance, kips	19.575		
Net area of plate, A_n , in^2	3.141		
U, shear lag factor for connection	1		case 1 of Table D3.1 AISC 360
Effective area of plate, A_e , in^2	3.141		Eq D3-1 of AISC 360
Tensile rupture capacity in net section, kips	91.078	>	79.987 OK
Required weld size, sixteenth of one inch	4.310		
Selected weld size, in	5/16		
Weld capacity, kips	92.80	>	79.987 OK

Table 5-3: High Strength Bolts

Number of bolts	6		
Number of rows	2		
Diameter of bolt, in	3/4		
Edge Distance, in	2		
Bolt spacing, in	3		
Bolt tensile capacity, kips	19.9		Table 7-2 AISC Manual
Total tensile capacity of bolts, kips	119.4	>	79.987 OK
Bolt shear capacity, kips	11.9		Table 7-1 AISC Manual
Total shear capacity of bolts, kips	71.4	>	19.675 OK

Table 5-4 Check Need for Beam Stiffeners:

Web local yielding capacity, Tale 9-4 page 9-48 AISC, kips	38.92	>	19.675 OK
Web local crippling capacity, Tale 9-4 page 9-48 AISC, kips	39.85	>	19.675 OK
Local flange bending capacity, Eq J10-1, kips	59.74	>	19.675 OK
No Web Stiffeners are Required			

Table 5-5 Shear Plate ASTM A36 Connection to HSS Post:

Selected width, in	4.00			
Selected thickness, in	3/8			
Selected length, in	8.5			
Number of ϕ 3/4" A325 N bolts	3			
Bolted simple shear pl connection capacity, kips, T 10-10a, page 10-110	28.8	>	19.675	OK

6- Design of Concrete Column:

Column is not braced (sway column).

Parameter	Value			
Specified compressive strength, f'_c , psi	2,500			
Unit weight of concrete, pcf	150			
Diameter, ft	2.00			
Gross area of column, A_g , in ²	452.39			
Maximum Height, l_c , ft	6.00			
Effective length factor, K_{bottom}	1.00			
Gross moment of inertia, I_g , in ⁴	5,184			
Moment of inertia of compression member, I_c , in ⁴	3,629			
Moment of inertia of the girder (beam), I_g , in ⁴	586			
Length of girder, l_g , ft	30.00			
Modulus of elasticity of concrete, E_c , ksi	2,850			
Modulus of elasticity of steel, E_s , ksi	29,000.00			
ψ_A value per Fig R10.10.1.1 ACI 318-11	3.04			
Effective length factor, K_{top}	1.55			
r , radius of gyration, in, Section 10.10.1.2 ACI 318-11	6.00			
$K_{top}l_u/r$ ratio, Eq10-6 ACI 318-11	18.60	<	22	No slenderness
M_u , ultimate wind moment at the base, k-ft (previous calcs)	369.28			Distributed to 2 columns instead of 3
Size of vertical rebar, in	1			
area of vertical rebar, in ²	0.785			
number of vertical rebar	12			
Area of steel reinforcement, A_s , in ²	9.425			
A_s/A_g , Section 10.9.1 ACI 318-11	2.083%	≥	1.00%	OK
side cover of reinforcement, in	1.5			
size of lateral reinforcement (link), in	0.375			
γ ration of distance between centroids of longit rebar to overall diameter	0.844			
P_u , total axial factored force, kips	78.488			
P_u/A_g , ksi	0.173			
$M_u/(A_g h)$, ksi- from App D of Structural Eng Ref Manual	0.408			
$M_u/(A_g h)$, ksi- limit from App D of Structural Eng Ref Manual	0.420	>	0.408	OK
Required development length, $0.02d_b f_y / (\lambda \sqrt{f'_c})$ in	24.00			controls
Required development length, $0.003 f_y d_b$, in	18.00			
Available development length, in	42.00	>	24.00	OK

Use ϕ 24" Diameter Concrete Column with 12#8 vertical rebar and #3 ties @ 8" OC

7- Design of Base Plate:

Section 14 of AISC Manual

Method ASD

N, in	21.00	
B, in	21.00	
d, in	9.00	
b, in	9.00	
m, in	6.23	
N, in	6.90	controls
n', in	2.25	
λ	1.00	
$\lambda n'$	2.25	
l, in	6.90	
Total axial load, kips	22.50	
t_{min} , minimum thickness of base plate, in	0.474	
Selected thickness, in	0.625	

8- Design of Spread Footing, F1:

Description	Value	Material Specifications:	Value
Length, ft	6.333	Description	
Width, ft	6.333	γ concrete density, normal weight, pcf	150.00
Depth, ft	4.00	f'_c Concrete compressive strength, psi	5,000.00
Area, ft ²	40.111	f_y , reinforcement yield stress, psi	60,000.00
Reinforcement size, in	1.000	ϕ , strength reduction factor, bending	0.90
area of one rebar, in ²	0.785	ϕ , strength reduction factor, shear	0.75
Bottom cover of reinforcement, in	3	λ , modification factor for concrete	1.00
Side cover of reinforcement, in	2		
Weight of footing, kips	24.067		
Effective depth of reinforcement, ft	3.667		
Max D.L. without footing weight, kips	13.971		
Maximum D.L. with footing, kips	38.038		
Maximum L.L., kips	36.456		
Max axial compression with footing wt, kips	74.494	L.C. D+L	
Max b moment on top of footing, kips-ft	79.80		
Min restraining moment, kips-ft	120.45		
Net OTM	-40.65		
Eccentricity, ft	-1.07		
L/6	1.056		
FOS against OT	1.509		
μ coefficient of friction	0.40		
Lateral force at top of footing, kip:	9.675		
FOS against Sliding	1.573		

Pressure at Soil/Foundation Interface:

L.C. D+L _r				L.C. 0.6D+0.6W	
q _s , foundation pressure, psf	1,857.18			OTM, k-ft	40.65
				Axial compression, kips	22.8226
L.C. D+0.6W				Eccentricity, ft	1.78
OTM, k-ft	40.65			Adjusted eccentricity, f	1.39
Axial compression, kips	38.04			q _s , max foundation press, psf	1,734.04
Eccentricity, ft	1.069				
Adjusted eccentricity, ft	2.10				
q _s , max foundation press, psf	1,908.54	<	2,000.00	OK	

Punching Shear:

Maximum axial factored force, kips	75.09				
Punching shear perimeter, b _o , ft	11.3				
Punching shear area, ft ²	32.111				
Factored pressure under footing, ksf	1.872				
Required Punching shear, kips	14.977				
Available punching shear, kips	1,269.40	>>	14.977	OK	

Flexural Bending:

K _u , ksi	0.0104				
ρ required reinf ratio	1.92586E-07				
ρ _{min}	0.0018	Controls			
β factor	0.80				
ρ _t min reinf ratio- tension-controlled	0.0213	section is tension-controlled			
A _s required area of steel, in ²	6.566				
Number of #8 rebar	9				
A _s provided area of steel, in ²	7.069	>	6.566	OK	

F1: 76"X76"X48" with 9#8 bottom and top reinforcement equally-spaced



MiTek USA, Inc.

7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610
Telephone 916/676-1900
Fax 916/676-1909

Re: 00416-17Y
Tony Abbott

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Foxworth Galbraith-Yuma, AZ.

Pages or sheets covered by this seal: R50761176 thru R50761236

My license renewal date for the state of Arizona is March 31, 2019.

Arizon COA: 11906-0

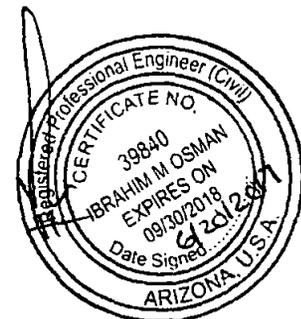
Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

*Truss calculations and layouts
were reviewed and found to
be consistent with my
design & calculations.*



Ong, Choo Soon

June 7, 2017



IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



Client: **LOCAL BUILDER**

Job Name: **Tony Abbott - First Floor**

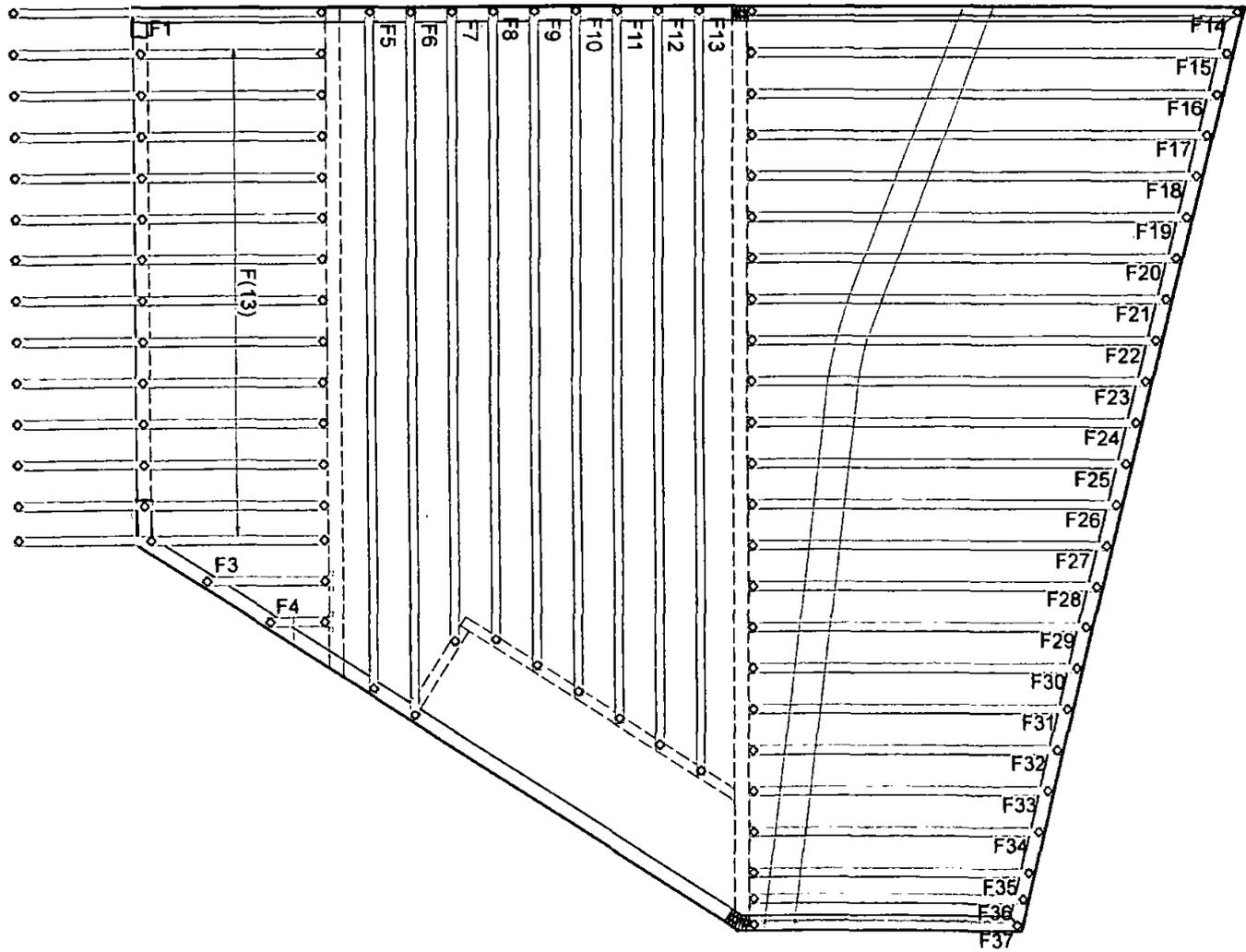
Job No: **00416-17Y**

Bid No: **00416-17Y**

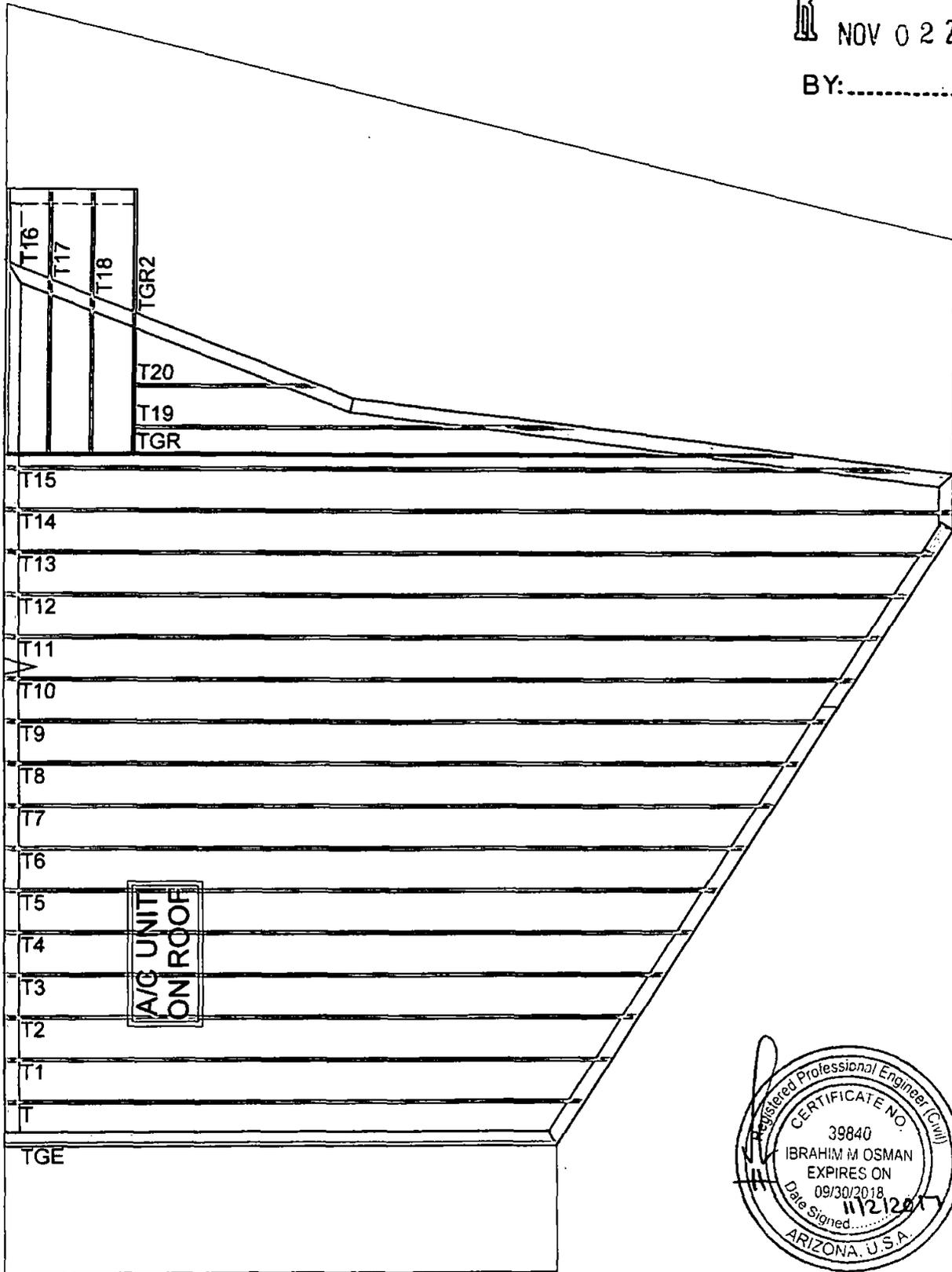
Address: **10343 E North Martinez Lake Road**

City: **Yuma**

State: **AZ**



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 BY:

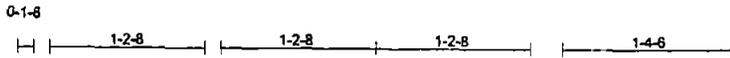


FOXWORTH GALBRAITH TRUSS DIVISION	Client:	LOCAL BUILDER		Job No.	00416-17Y	
	Job Name:	Tony Abbott				
	Address:	10343 E North Martinez Lake Road	City:	Yuma	State:	AZ

Job 00416-17Y	Truss F	Truss Type Floor	Qty 13	Ply 1	Tony Abbott	R50761176
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Foxworth Galbreath Lumber Co, Yuma, Az 85385

7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:00 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8i-7T3jeNv5EjLX63KsT874gQYLv7VAQp9ToSS5jFz9?KJ



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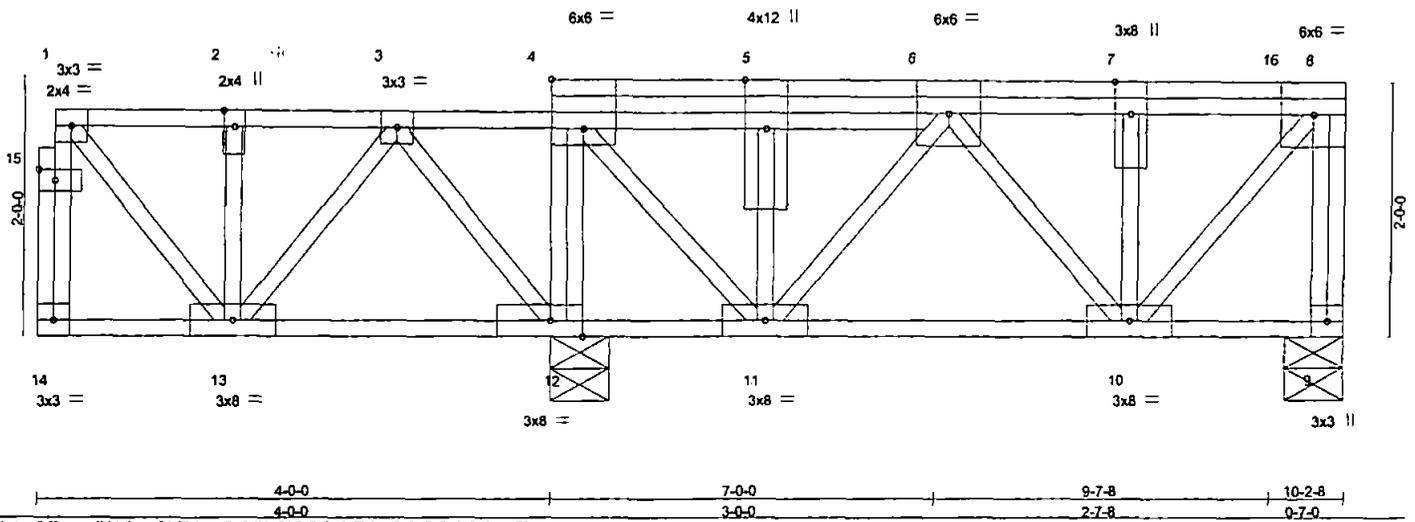


Plate Offsets (X, Y) - [4:Edge,0-4-8], [12:0-3-0,Edge], [15:0-1-8,0-1-0]

LOADING (psf)	SPACING-	CSL	DEFL	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.16	in (loc) l/defl L/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.08	Vert(LL) -0.00 11 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.28	Vert(TL) -0.01 10-11 >999 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) -0.00 9 n/a n/a		
	Code IRC2012/TPI2007			Weight: 66 lb	FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF No.2(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purtins, except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 10-0-0 oc bracing: 9-10.
WEBS 2x4 HF/SPF Stud/Std(flat)	

REACTIONS. (lb/size) 9=121/0-5-8, 12=1271/0-5-8
Max Uplift 9=-38(LC 3)
Max Grav 9=173(LC 4), 12=1271(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=0/346
BOT CHORD 11-12=-411/0
WEBS 4-12=-1033/0, 3-12=-274/0, 4-11=0/399

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 9.
 - 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 5) CAUTION, Do not erect truss backwards.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 4-0-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 9-14=13, 1-8=67
Concentrated Loads (lb)
Vert: 4=600(F)



Expires: 3-31-2019
June 7, 2017

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.</p>	 7777 Greenback Lane Suite 109 Citrus Heights, CA 95610
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Job	Truss	Truss Type	Qty	Ply	Tony Abbott	R50761177
00416-17Y	F1	Floor	1	1	Job Reference (optional)	

Foxworth Galbraith Lumber Co, Yuma, Az 85365

7,640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:01 2017 Page 1
 ID:Z_0Fgkz7ob1GlpCh_14UWJzDd8i-bfd5rjvk?1T0kDv21sWJCe5WpXsz9IPc06BVGiz9?KI



Scale = 1:17.3

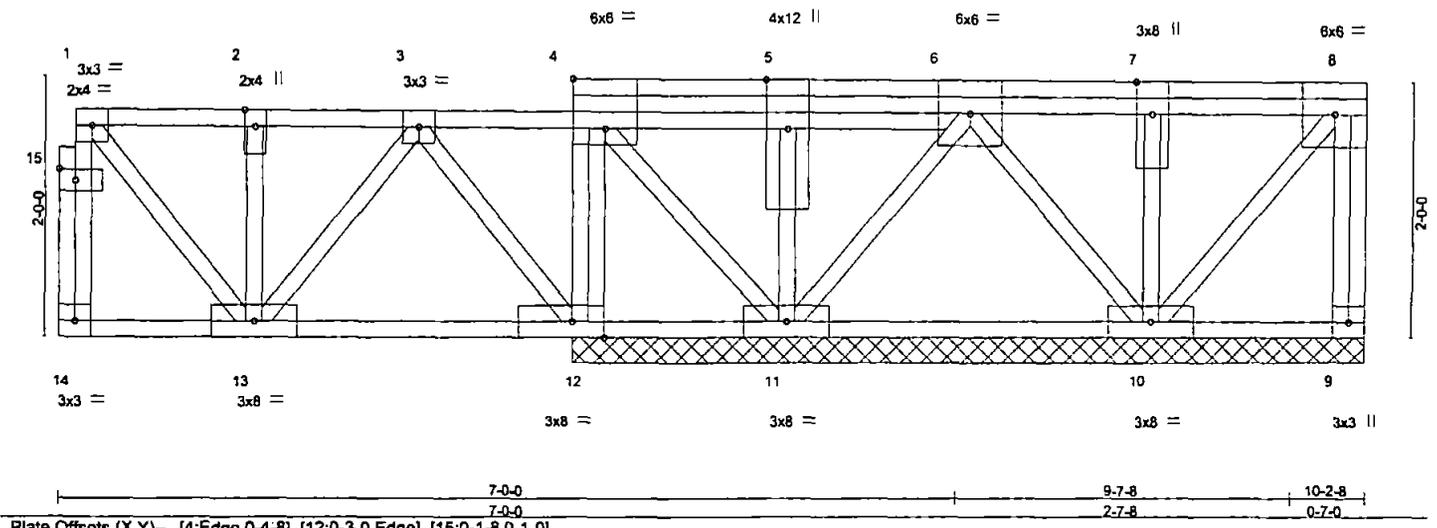


Plate Offsets (X,Y) - [4:Edge,0-4-8], [12:0-3-0,Edge], [15:0-1-8,0-1-0]

LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.15	in (loc) l/defl L/d	MT20	185/144
TCDL 10.0	Lumber DOL 1.00	BC 0.04	Vert(LL) n/a - n/a 999		
BCLL 0.0	Rep Stress Incr NO	WB 0.15	Vert(TL) n/a - n/a 999		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)	Horz(TL) -0.00 9 n/a n/a		
				Weight: 66 lb	FT = 0%F, 0%E

LUMBER-
 TOP CHORD 2x4 SPF No.2(flat)
 BOT CHORD 2x4 SPF No.2(flat)
 WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 10-0-0 oc bracing: 9-10.

REACTIONS. All bearings 6-2-8.
 (lb) - Max Uplift All uplift 100 lb or less at joint(s) 9, 10
 Max Grav All reactions 250 lb or less at joint(s) 9, 10 except 12=1045(LC 1), 11=282(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 3-4=0/347, 4-5=0/325, 5-6=0/325
 BOT CHORD 11-12=-397/0
 WEBS 4-12=-806/0, 3-12=-276/0, 6-11=-256/0

- NOTES-**
- Unbalanced floor live loads have been considered for this design.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 10.
 - Non Standard bearing condition. Review required.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - CAUTION, Do not erect truss backwards.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 4-0-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 9-14=-13, 1-8=-67
 Concentrated Loads (lb)
 Vert: 4=-600(F)



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 June 7, 2017

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 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job	Truss	Truss Type	Qty	Ply	Tony Abbott	R50761178
00416-17Y	F3	Floor	1	1	Job Reference (optional)	

Foxworth Galbraith Lumber Co, Yuma, Az 85385

7.640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:12 2017 Page 1
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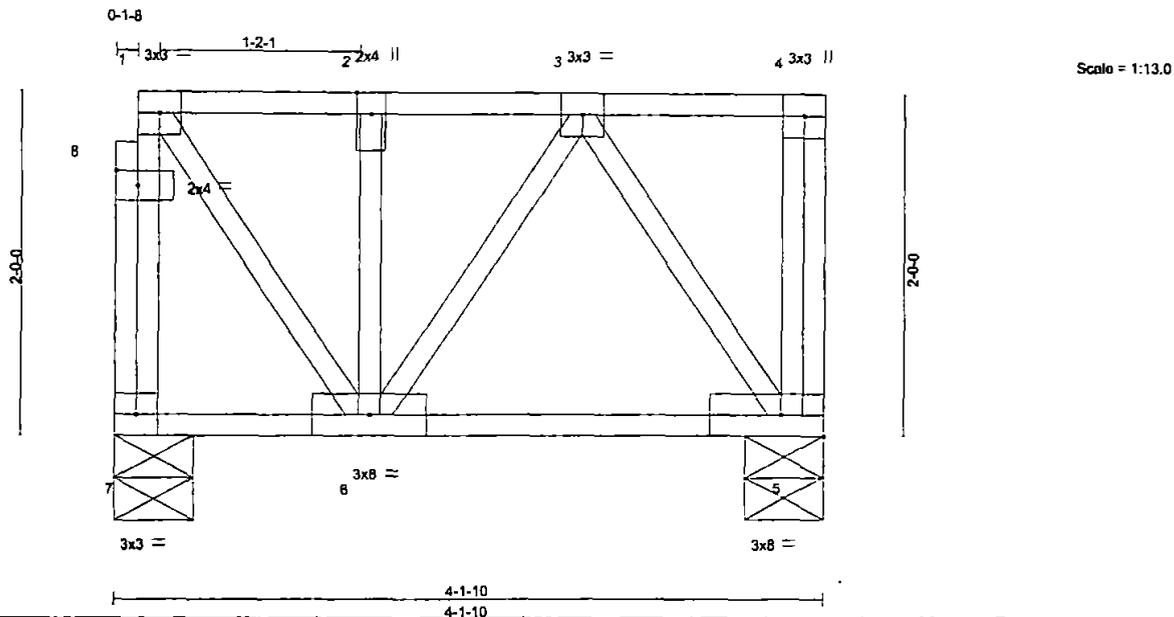


Plate Offsets (X,Y) - [8:0-1-8,0-1-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.05	in (oc) l/defl L/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.06	Vert(LL) -0.00 6 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.09	Vert(TL) -0.01 5-6 >999 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 5 n/a n/a		
	Code IRC2012/TPI2007			Weight: 27 lb	FT = 0%F, 0%E

LUMBER-

TOP CHORD 2x4 SPF No.2(flat)
 BOT CHORD 2x4 SPF No.2(flat)
 WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-1-10 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 7=151/0-5-8, 5=155/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 2) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 3) CAUTION, Do not erect truss backwards.



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 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIH-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Leo Street, Suite 312, Alexandria, VA 22314.

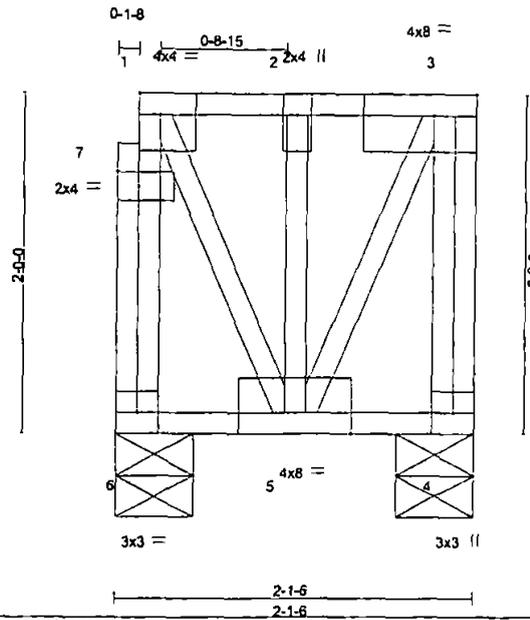


7777 Greenback Lane
 Suite 108
 Citrus Heights, CA 95610

Job	Truss	Truss Type	Qty	Ply	Tony Abbott	R50761179
00416-17Y	F4	Floor	1	1		

Foxworth Galbraith Lumber Co. Yuma, Az 85365

7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:16 2017 Page 1
 ID:Z_0Fgkz7ob1GlpCh_4UWJzDd8l-FY1m7rS8ToMF1WZxPVHrJoD6ea_2A6vqTyKolKz9?KT



Scale = 1:13.2

Plate Offsets (X,Y)- [1:Edge,0-1-8], [3:0-3-0,Edge], [7:0-1-8,0-1-0]

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	V/defl	L/d	PLATES	GRIP	
TCLL 40.0	Plate Grip DOL	1.00	TC 0.02	Vert(LL)	-0.00	6	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.01	Vert(TL)	-0.00	5	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.03	Horz(TL)	0.00	4	n/a	n/a		
BCDL 10.0	Code IRC2012/TPI2007		(Matrix)							
									Weight: 19 lb	FT = 0%F, 0%E

LUMBER-

TOP CHORD 2x4 SPF No.2(flat)
 BOT CHORD 2x4 SPF No.2(flat)
 WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 2-1-6 oc purtins. except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 6=71/0-5-8, 4=75/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 2) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 3) CAUTION, Do not erect truss backwards.



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSIT/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

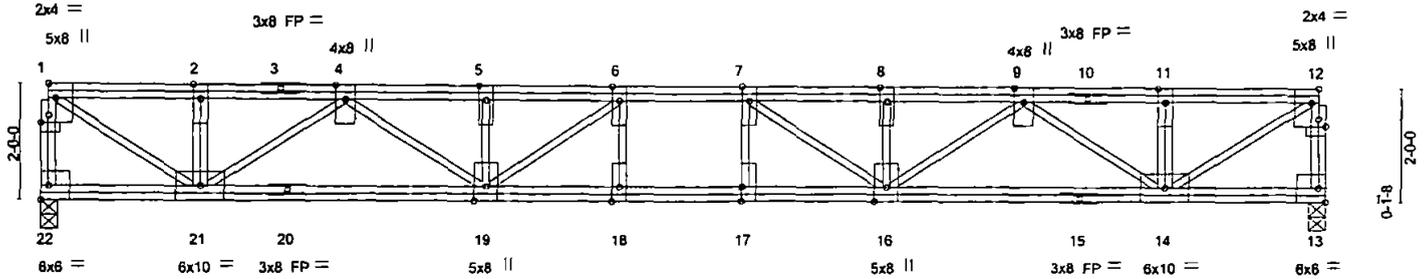
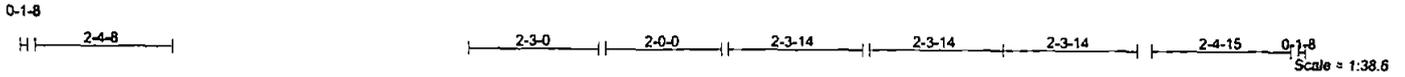


7777 Groenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss F5	Truss Type FLOOR	Qty 1	Ply 1	Tony Abbott	R50761180
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

7.840 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:17 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_4UWJzDd8i-7kb8CB6mEyU6eg77zDo4s?IE0zDivK6zic3Mqmqz9?KS



10-1-8	11-1-8, 12-1-8	22-3-1
10-1-8	1-0-0, 1-0-0	10-1-9

Plate Offsets (X,Y) - [1:Edge,0-1-8], [1:0-1-8,0-1-8], [12:0-3-0,Edge], [12:0-1-8,0-1-8], [17:0-3-0,0-0-0]

LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.24	in (loc) l/defl L/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.45	Vert(LL) -0.12 16-17 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 1.00	Vert(TL) -0.20 16-17 >999 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.04 13 n/a n/a		
	Code IRC2003/TPI2002			Weight: 148 lb	FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF No.2(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purins, except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 HF/SPF Stud/Std(flat)	

REACTIONS. (lb/size) 22=880/0-3-8, 13=880/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-22=-870/0, 12-13=-869/0, 1-2=-1199/0, 2-3=-1199/0, 3-4=-1199/0, 4-5=-2523/0, 5-6=-2523/0, 6-7=-2753/0, 7-8=-2512/0, 8-9=-2512/0, 9-10=-1211/0, 10-11=-1211/0, 11-12=-1211/0
BOT CHORD 20-21=0/1981, 19-20=0/1981, 18-19=0/2753, 17-18=0/2753, 16-17=0/2753, 15-16=0/1975, 14-15=0/1975
WEBS 1-21=0/1435, 4-21=-954/0, 4-19=0/667, 6-19=-508/30, 7-16=-515/21, 9-16=0/666, 9-14=-939/0, 12-14=0/1445

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x8 MT20 unless otherwise indicated.
 - 3) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 4) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Expires: 3-31-2019
June 7.2017

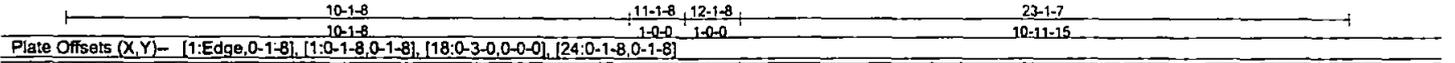
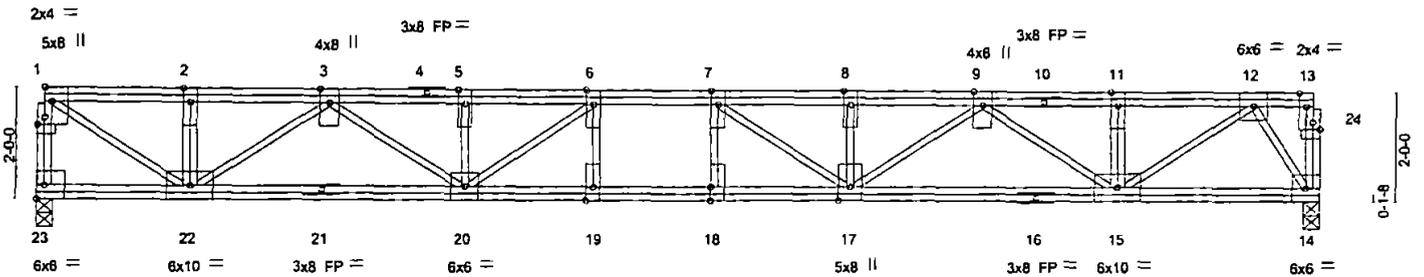
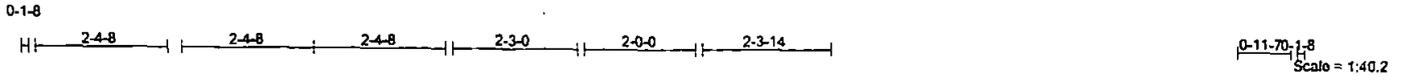
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, OSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F6	Truss Type FLOOR	Qty 1	Ply 1	Tony Abbott	R50761181
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Foxworth Galbraith Lumber Co. Yuma, Az 85385

Job Reference (optional)
7.640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:17 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8f-7kb8CB6mEyU6og77zDo4s7iDKzCjvMAzic3Mqz9?KS



LOADING (psf)		SPACING- 1-4-0		CSI.		DEFL		PLATES		GRIP	
TCLL	40.0	Plate Grip DOL	1.00	TC	0.28	in (loc)	l/defl	L/d	MT20	185/144	
TCDL	10.0	Lumber DOL	1.00	BC	0.52	Vert(LL)	-0.14 17-18	>999	360		
BCLL	0.0	Rep Stress Incr	YES	WB	0.86	Vert(TL)	-0.24 17-18	>999	240		
BCDL	10.0	Code IRC2003/TP12002		(Matrix)		Horz(TL)	0.04 14	n/a	n/a		
									Weight: 153 lb FT = 0%F, 0%E		

LUMBER-
TOP CHORD 2x4 SPF No.2(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat) *Except*
1-22,3-22,3-20: 2x4 SPF No.2(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 23=912/0-3-8, 14=912/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-23=-903/0, 1-2=-1248/0, 2-3=-1247/0, 3-4=-2657/0, 4-5=-2657/0, 5-6=-2657/0,
6-7=-2954/0, 7-8=-2781/0, 8-9=-2781/0, 9-10=-1561/0, 10-11=-1561/0, 11-12=-1561/0
BOT CHORD 21-22=0/2075, 20-21=0/2075, 19-20=0/2954, 18-19=0/2954, 17-18=0/2954, 16-17=0/2287,
15-16=0/2287, 14-15=0/543
WEBS 1-22=0/1494, 3-22=-1010/0, 3-20=0/717, 6-20=-581/0, 7-17=-472/93, 9-17=0/612,
9-15=-893/0, 12-15=0/1252, 12-14=-1024/0

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x8 MT20 unless otherwise indicated.
 - 3) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP1 1.
 - 4) *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Expires: 3-31-2019
June 7, 2017

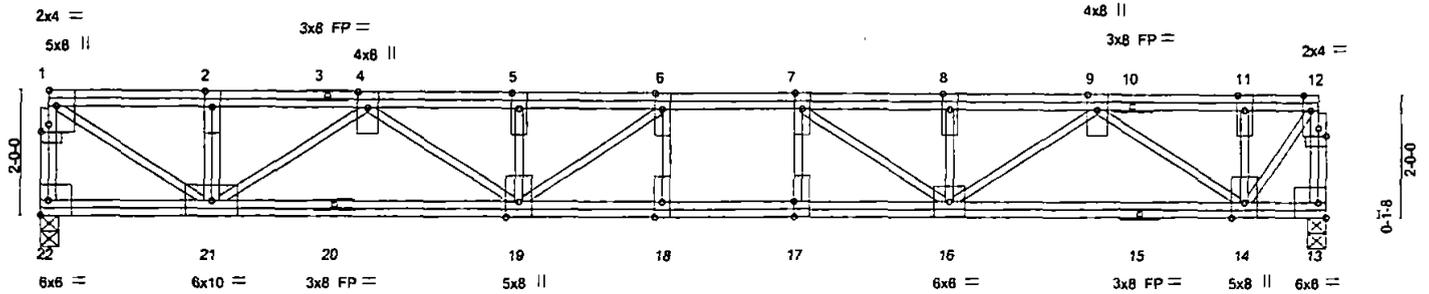
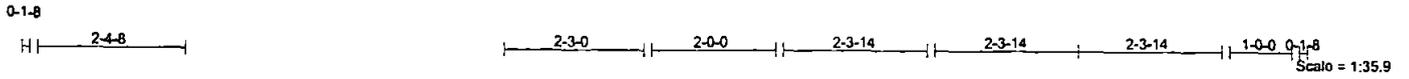
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

7777 Groenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F7	Truss Type FLOOR	Qty 1	Ply 1	Tony Abbott	R50761182
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

Job Reference (optional)
7.840 s Apr 19 2018 MITek Industries, Inc. Tue Jun 06 10:48:18 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_14UWJzDd8i-cw9WQX7O?FczGqJWwJJDIO4NZBeof7xGpvMDz9?KR



10-1-8	11-1-8	12-1-8	20-8-10
10-1-8	1-0-0	1-0-0	8-7-2

Plate Offsets (X,Y) - [1:Edge,0-1-8], [1:0-1-8,0-1-8], [12:0-1-8,0-1-8], [17:0-3-0,0-0-0]

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.28	Vert(LL)	-0.12 18-19	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.48	Vert(TL)	-0.19 18-19	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.91	Horz(TL)	0.03 13	n/a	n/a		
BCDL 10.0	Code IRC2003/TPI2002		(Matrix)						
								Weight: 138 lb	FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF No.2(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid coiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 HF/SPF Stud/Std(flat)	

REACTIONS. (lb/size) 22=819/0-3-8, 13=819/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-22=-808/0, 12-13=-831/0, 1-2=-1106/0, 2-3=-1106/0, 3-4=-1106/0, 4-5=-2265/0, 5-6=-2265/0, 6-7=-2367/0, 7-8=-1994/0, 8-9=-1994/0, 9-10=-549/0, 10-11=-549/0, 11-12=-550/0
 BOT CHORD 20-21=0/1801, 19-20=0/1801, 18-19=0/2367, 17-18=0/2367, 16-17=0/2367, 15-16=0/1386, 14-15=0/1386
 WEBS 1-21=0/1324, 4-21=-848/0, 4-19=0/572, 6-19=-372/118, 7-16=-605/0, 9-16=0/753, 9-14=-1038/0, 12-14=0/971

- NOTES-
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x8 MT20 unless otherwise indicated.
 - 3) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 4) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

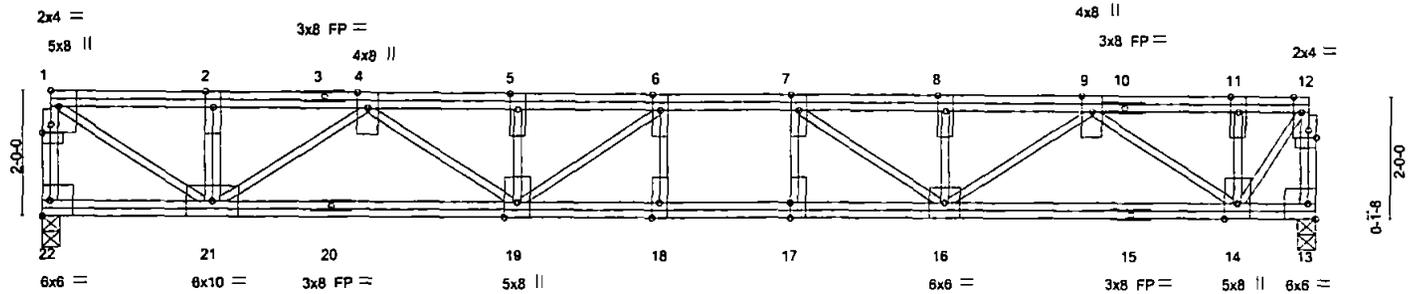
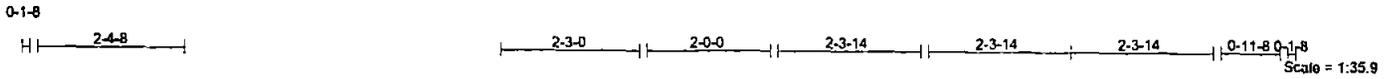


7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job	Truss	Truss Type	Qty	Ply	Tony Abbott	R50761183
00416-17Y	F8	FLOOR	1	1		

Foxworth Galbraith Lumber Co, Yuma, Az 85385

7.840 s Apr 19 2016 Mitek Industries, Inc. Tue Jun 06 10:48:19 2017 Page 1
 ID:Z_0Fgkz7ob1GlpCh_4UWJzDd8i-47ivd170mZlqu_HW4eqYxQrZpnuPNFzG9wZSufz9?KQ



10-1-8	11-1-8	12-1-8	20-8-2
10-1-8	1-0-0	1-0-0	8-6-10

Plate Offsets (X,Y) - [1:Edge,0-1-8], [1:0-1-8,0-1-8], [12:0-1-8,0-1-8], [17:0-3-0,0-0-0]

LOADING (psf)	SPACING-	CSL	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.28	Vort(LL)	-0.12	18-19	>999	MT20	185/144
TCDL 10.0	Lumber DOL 1.00	BC 0.48	Vort(TL)	-0.19	18-19	>999		
BCLL 0.0	Rep Stress Incr YES	WB 0.91	Horz(TL)	0.03	13	n/a		
BCDL 10.0	Code IRC2003/TPI2002	(Matrix)						

Weight: 138 lb FT = 0%F, 0%E

LUMBER-
 TOP CHORD 2x4 SPF No.2(flat)
 BOT CHORD 2x4 SPF No.2(flat)
 WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 22=817/0-3-8, 13=817/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-22=-806/0, 12-13=-830/0, 1-2=-1104/0, 2-3=-1103/0, 3-4=-1103/0, 4-5=-2258/0, 5-6=-2258/0, 6-7=-2357/0, 7-8=-1980/0, 8-9=-1980/0, 9-10=-531/0, 10-11=-531/0, 11-12=-531/0
 BOT CHORD 20-21=0/1796, 19-20=0/1796, 18-19=0/2357, 17-18=0/2357, 16-17=0/2357, 15-16=0/1370, 14-15=0/1370
 WEBS 1-21=0/1321, 4-21=-845/0, 4-19=0/569, 6-19=-368/120, 7-16=-608/0, 9-16=0/756, 9-14=-1041/0, 12-14=0/962

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x8 MT20 unless otherwise indicated.
 - 3) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 4) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-1473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with Mitek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

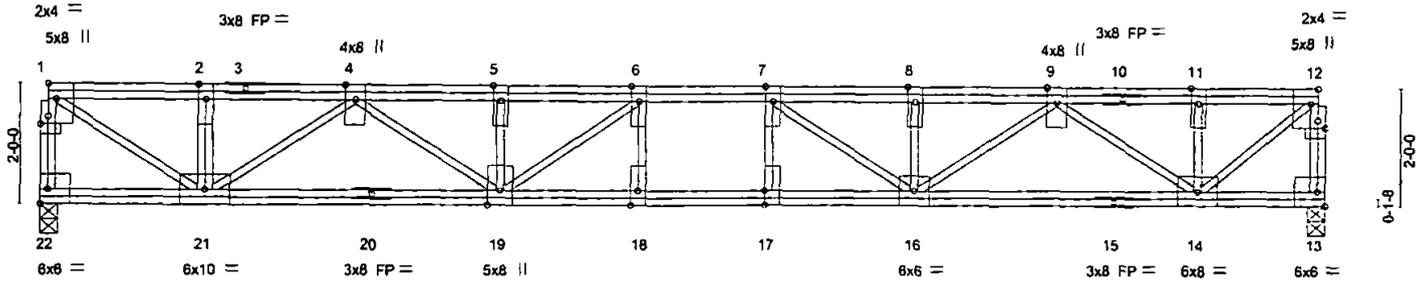
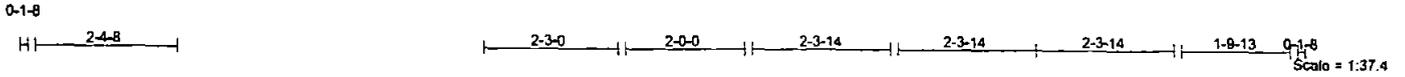


7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss F9	Truss Type FLOOR	Qty 1	Ply 1	Tony Abbott	R50761184
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

7.640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:19 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8i-47ivdt70mZlqu_HW4oqYxQrZEnucNFGG9wZsufz9?KQ



10-1-8	11-1-8	12-1-8	21-6-7
10-1-8	1-0-0	1-0-0	9-4-15

Plate Offsets (X,Y)~ [1:Edge,0-1-8], [1:0-1-8,0-1-8], [12:0-3-0,Edge], [12:0-1-8,0-1-8], [17:0-3-0,0-0-0]									
LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.26	Vert(LL)	-0.12 18-19	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.46	Vert(TL)	-0.20 18-19	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.95	Horz(TL)	0.03 13	n/a	n/a		
BCDL 10.0	Code IRC2003/TPI2002		(Matrix)						
								Weight: 142 lb	FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF No.2(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 HF/SPF Stud/Std(flat)	

REACTIONS. (lb/size) 22=851/0-3-8, 13=851/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-22=-841/0, 12-13=-848/0, 1-2=-1156/0, 2-3=-1155/0, 3-4=-1155/0, 4-5=-2402/0, 5-6=-2402/0, 6-7=-2573/0, 7-8=-2269/0, 8-9=-2269/0, 9-10=-911/0, 10-11=-911/0, 11-12=-912/0
 BOT CHORD 20-21=0/1897, 19-20=0/1897, 18-19=0/2573, 17-18=0/2573, 16-17=0/2573, 15-16=0/1700, 14-15=0/1700
 WEBS 1-21=0/1383, 4-21=-905/0, 4-19=0/623, 6-19=-444/70, 7-16=-557/0, 9-16=0/706, 9-14=-978/0, 12-14=0/1204

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x8 MT20 unless otherwise indicated.
 - 3) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 4) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

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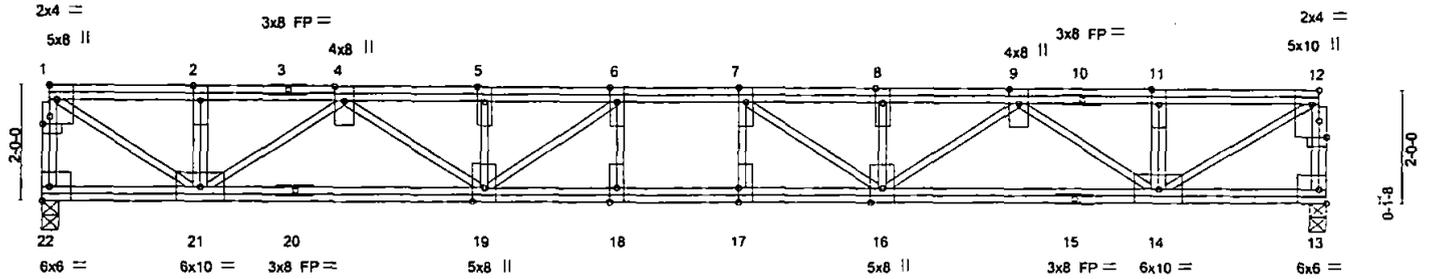
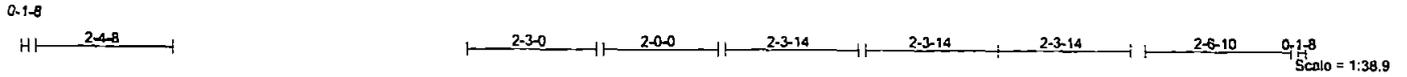


7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F10	Truss Type FLOOR	Qty 1	Ply 1	Tony Abbott	R50761185
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Foxworth Galbraith Lumber Co. Yuma, Az 85385

7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:02 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_14UWJzDd8l-4rBT33wMmLbELMUFaZ1YIrdggw5euYNmFmx3o8z9?Kk



10-1-8	11-1-8, 12-1-8	22-4-12
10-1-8	1-0-0, 1-0-0	10-3-4

Plate Offsets (X,Y) - [1:Edge,0-1-8], [1:0-1-8,0-1-8], [12:0-3-0,Edge], [12:0-1-8,0-3-8], [17:0-3-0,0-0-0]					
LOADING (psf)	SPACING- 1-4-0	CSI.	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.25	Vert(LL) -0.12 16-17 >999 360	MT20	185/144
TCDL 10.0	Lumber DOL 1.00	BC 0.46	Vert(TL) -0.21 16-17 >999 240		
BCLL 0.0	Rep Stress Incr YES	WB 1.00	Horz(TL) 0.04 13 n/a n/a		
BCDL 10.0	Code IRC2003/TPI2002	(Matrix)			
				Weight: 148 lb	FT = 0%F, 0%E

LUMBER-
TOP CHORD 2x4 SPF No.2(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat) *Except*
12-14: 2x4 SPF No.2(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 22=886/0-3-8, 13=886/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-22=-876/0, 12-13=-874/0, 1-2=-1208/0, 2-3=-1207/0, 3-4=-1207/0, 4-5=-2546/0, 5-6=-2546/0, 6-7=-2788/0, 7-8=-2559/0, 8-9=-2559/0, 9-10=-1275/0, 10-11=-1275/0, 11-12=-1275/0
BOT CHORD 20-21=0/1997, 19-20=0/1997, 18-19=0/2788, 17-18=0/2788, 16-17=0/2788, 15-16=0/2029, 14-15=0/2029
WEBS 1-21=0/1446, 4-21=-964/0, 4-19=0/676, 6-19=-521/22, 7-16=-507/34, 9-16=0/658, 9-14=-926/0, 12-14=0/1499

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x8 MT20 unless otherwise indicated.
 - 3) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 4) *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 216 N. Lee Street, Suite 312, Alexandria, VA 22314.

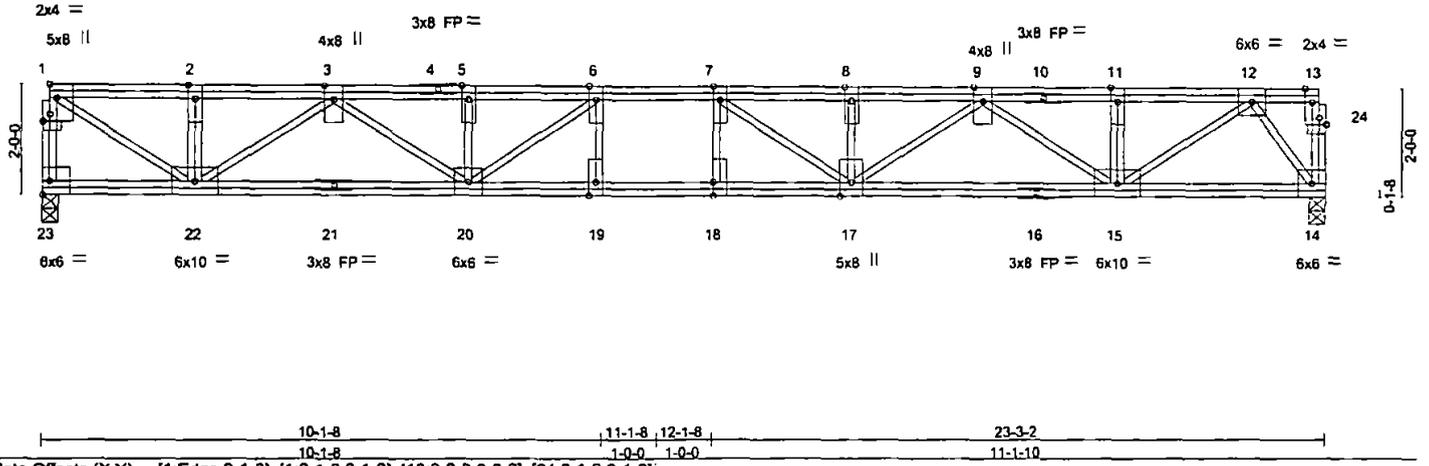
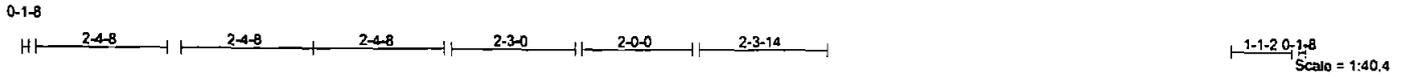
7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F11	Truss Type FLOOR	Qty 1	Ply 1	Tony Abbott	R50761186
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Foxworth Galbraith Lumber Co., Yuma, Az 85385

7.640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:03 2017 Page 1

ID:Z_0Fgkz7ob1GlpCh_4UWJzDd8l-Y2lsGOx_Xek5zW3R8HYoH3AqkKQtd1qvUQgcKaz9?Kg



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.29	in (loc) l/defl U/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.53	Vert(LL) -0.14 17-18 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.86	Vert(TL) -0.24 17-18 >999 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.04 14 n/a n/a		
	Code IRC2003/TPI2002			Weight: 154 lb	FT = 0%F, 0%E

LUMBER-
 TOP CHORD 2x4 SPF No.2(flat)
 BOT CHORD 2x4 SPF No.2(flat)
 WEBS 2x4 HF/SPF Stud/Std(flat) *Except*
 1-22,3-22,3-20: 2x4 SPF No.2(flat)

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purtins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 23=918/0-3-8, 14=918/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-23=908/0, 1-2=1256/0, 2-3=1256/0, 3-4=2681/0, 4-5=2681/0, 5-6=2681/0,
 6-7=2990/0, 7-8=2828/0, 8-9=2828/0, 9-10=1623/0, 10-11=1623/0, 11-12=1623/0
 BOT CHORD 21-22=0/2091, 20-21=0/2091, 19-20=0/2990, 18-19=0/2990, 17-18=0/2990, 16-17=0/2341,
 15-16=0/2341, 14-15=0/614
 WEBS 1-22=0/1504, 3-22=1020/0, 3-20=0/726, 6-20=-593/0, 7-17=-464/106, 9-17=0/603,
 9-15=883/0, 12-15=0/1241, 12-14=-1059/0

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x8 MT20 unless otherwise indicated.
 - 3) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 4) *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MITEK
 7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss F12	Truss Type FLOOR	Qty 1	Ply 1	Tony Abbott	R50761187
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Foxworth Galbraith Lumber Co. Yuma, Az 85385

7.640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:04 2017 Page 1
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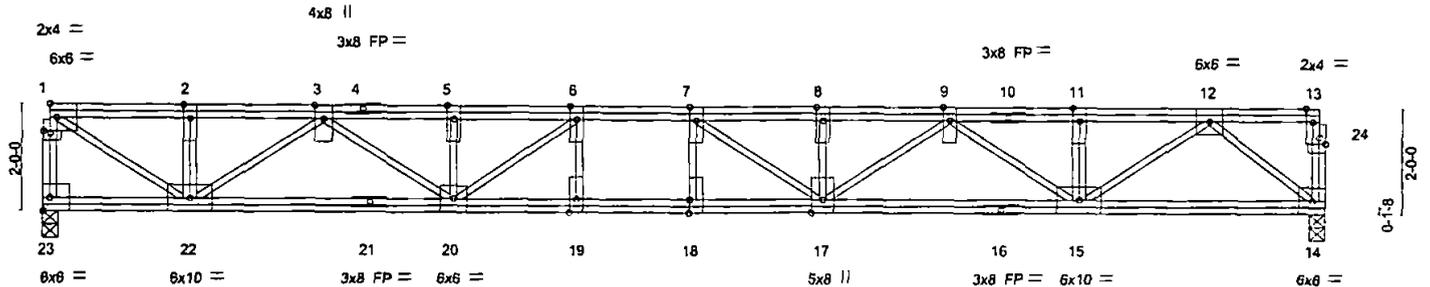
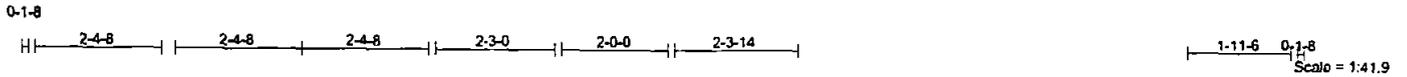


Plate Offsets (X,Y)- [1:0-1-8,0-0-8], [18:0-3-0,0-0-0], [24:0-1-8,0-1-8]
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LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.34	Vert(LL)	-0.17 17-18	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.60	Vert(TL)	-0.29 17-18	>974	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.81	Horz(TL)	0.04 14	n/a	n/a		
BCDL 10.0	Code IRC2003/TPI2002		(Matrix)						
								Weight: 159 lb	FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF No.2(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 HF/SPF Stud/Std(flat) "Except" 1-22,3-22,3-20: 2x4 SPF No.2(flat)	

REACTIONS. (lb/size) 23=952/0-3-8, 14=952/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-23=-943/0, 1-2=-1308/0, 2-3=-1307/0, 3-4=-2824/0, 4-5=-2824/0, 5-6=-2824/0,
 6-7=-3204/0, 7-8=-3114/0, 8-9=-3114/0, 9-10=-2002/0, 10-11=-2002/0, 11-12=-2002/0
 BOT CHORD 21-22=0/2191, 20-21=0/2191, 19-20=0/3204, 18-19=0/3204, 17-18=0/3204, 16-17=0/2672,
 15-16=0/2672, 14-15=0/1049
 WEBS 1-22=0/1566, 3-22=-1079/0, 3-20=0/779, 6-20=-672/0, 7-17=-420/183, 9-17=0/548,
 9-15=-823/0, 12-15=0/1173, 12-14=-1348/0

- NOTES-
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x8 MT20 unless otherwise indicated.
 - 3) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 4) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

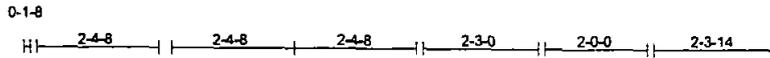


Job	Truss	Truss Type	Qty	Ply	Tony Abbot	R50761188
00416-17Y	F13	FLOOR	1	1		

Foxworth Galbreath Lumber Co., Yuma, Az 85385

Job Reference (optional)

7.640 s Apr 19 2016 MITEK Industries, Inc. Tue Jun 06 10:48:05 2017 Page 1
ID:Z_0Fgk7ob1GlpCh_t4UWJzDd8I-UQsch4zE3G_pCqDqGlaGNUFB286s5y9Cxx9)PTz9?K6



0-0-8
Scale = 1:43.4

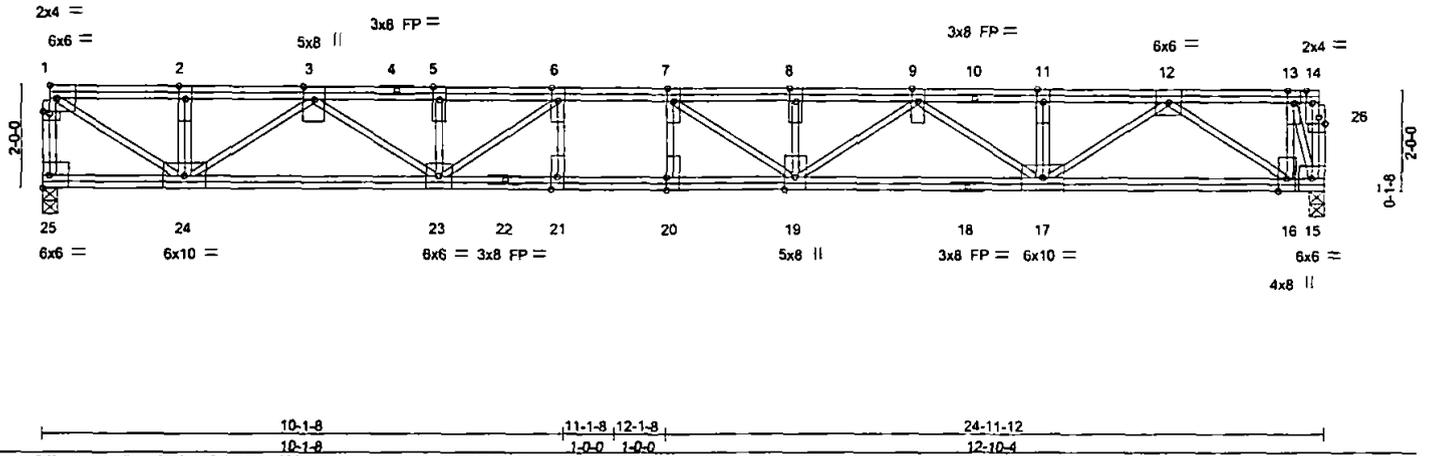


Plate Offsets (X,Y)- [1:0-1-8,0-0-8], [20:0-3-0,0-0-0], [26:0-1-8,0-1-8]					
LOADING (psf)	SPACING- 1-4-0	CSI.	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.24	Vert(LL) -0.18 19-20 >999 360	MT20	18S/144
TCDL 10.0	Lumber DOL 1.00	BC 0.56	Vert(TL) -0.32 19-20 >937 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.80	Horz(TL) 0.05 15 n/a n/a		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)			
				Weight: 167 lb	FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF No.2(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 HF/SPF Stud/Std(flat) *Except* 1-24,3-24,3-23: 2x4 SPF No.2(flat)	

REACTIONS. (lb/size) 25=987/0-3-8, 15=987/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-25=976/0, 1-2=1354/0, 2-3=1353/0, 3-4=2961/0, 4-5=2961/0, 5-6=2961/0,
6-7=3412/0, 7-8=3394/0, 8-9=3394/0, 9-10=2376/0, 10-11=2376/0, 11-12=2376/0,
12-13=291/0
BOT CHORD 23-24=0/2286, 22-23=0/3412, 21-22=0/3412, 20-21=0/3412, 19-20=0/3412, 18-19=0/2997,
17-18=0/2997, 16-17=0/1430, 15-16=0/297
WEBS 1-24=0/1620, 3-24=1139/0, 3-23=0/830, 6-23=746/0, 7-19=372/259, 9-19=0/492,
9-17=763/0, 12-17=0/1164, 12-16=1411/0, 13-16=0/812, 13-15=1016/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x8 MT20 unless otherwise indicated.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIF-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



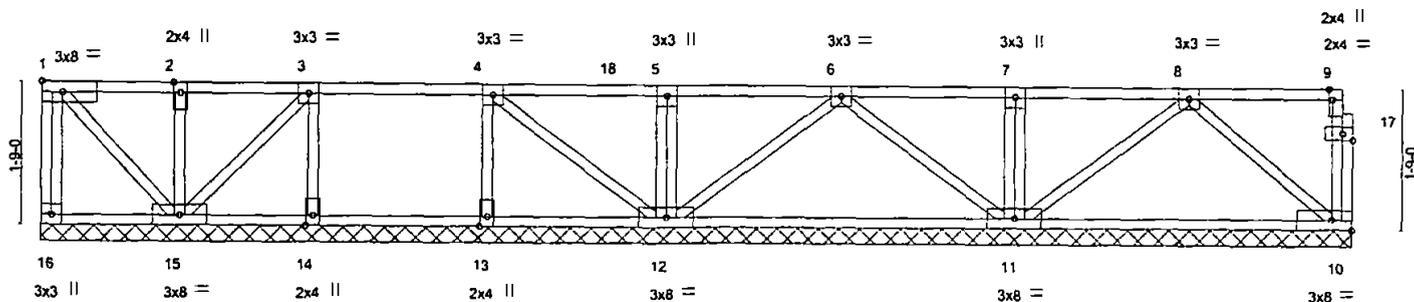
Job	Truss	Truss Type	Qty	Ply	Tony Abbott	R50761189
00416-17Y	F14	FLOOR SUPPORTED GABL	1	1		

Foxworth Galbraith Lumber Co, Yuma, Az 85365

7.840 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 08 10:48:05 2017 Page 1
ID:Z_OFgkz7ob1GlpCh_14UWJzDd8l-UQsch4zE3G_pCqDqGlaGNUF4q8CD57Kcx9PTz97Ke

0-ft-8

Scale = 1:27.3



	3-4-8	4-4-8	5-4-8	15-9-10	16-0-10
	3-4-8	1-0-0	1-0-0	10-5-2	0-3-0
Plate Offsets (X,Y)-	[9:0-1-8,Edge], [17:0-1-8,0-1-0]				

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 40.0	Plate Grip DOL	1.00	TC 0.70	Vert(LL)	n/a	-	n/a	999	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.15	Vert(TL)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	NO	WB 0.09	Horz(TL)	0.00	10	n/a	n/a		
BCDL 10.0	Code IRC2012/TPI2007		(Matrix)							
									Weight: 75 lb	FT = 0%F, 0%E

LUMBER-
TOP CHORD 2x4 SPF 2100F 1.8E(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purtins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 16-0-10.
(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 10, 14, 15 except 13=325(LC 1), 12=775(LC 1), 11=332(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 4-13=-302/0, 5-12=-654/0

- NOTES-**
- 1) Gable requires continuous bottom chord bearing.
 - 2) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 3) Gable studs spaced at 1-4-0 oc.
 - 4) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 6) CAUTION. Do not erect truss backwards.
 - 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 7-0-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 10-16=-13, 1-9=-67
Concentrated Loads (lb)
Vert: 18=-600(F)



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

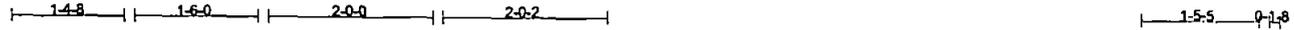


7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job	Truss	Truss Type	Qty	Ply	Tony Abbott	R50761190
00416-17Y	F15	FLOOR SUPPORTED GABL	1	1		

Foxworth Galbraith Lumber Co. Yuma, Az 85365

7,640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:06 2017 Page 1
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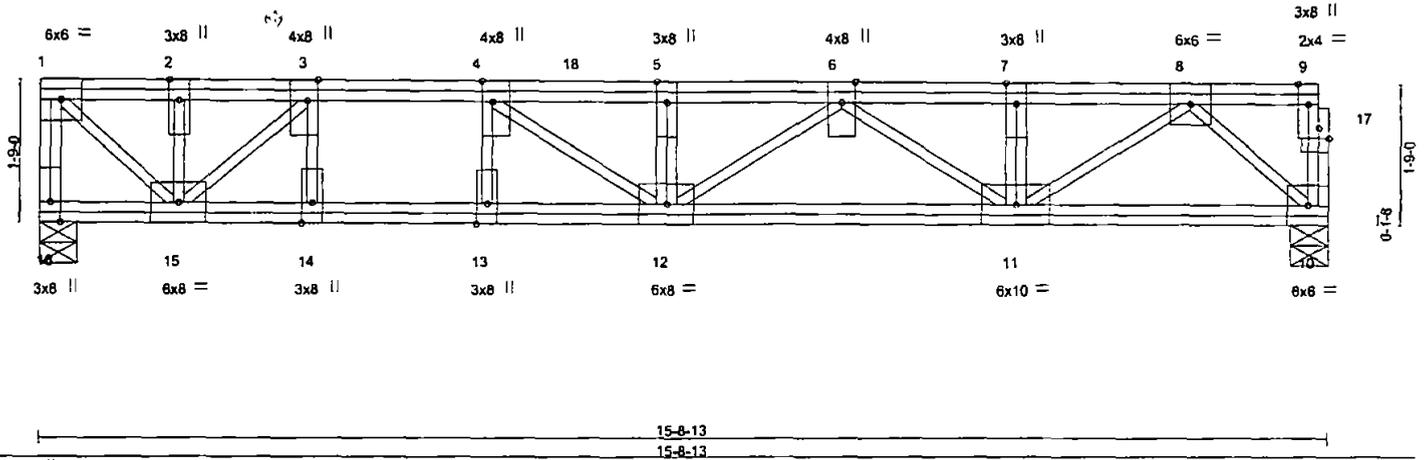


Plate Offsets (X, Y) - [3:0-3-0,Edge], [4:0-3-0,Edge], [17:0-1-8,0-1-8]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.82	in (loc) l/defl L/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.81	Vert(LL) -0.16 12-13 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.81	Vert(TL) -0.28 12-13 >673 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.02 10 n/a n/a		
	Code IRC2012/TPI2007			Weight: 105 lb	FT = 0%F, 0%E

LUMBER-

TOP CHORD 2x4 SPF No.2(flat)
 BOT CHORD 2x4 SPF 1650F 1.5E(flat)
 WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 16=967/0-5-8, 10=867/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-16=-901/0, 1-2=-843/0, 2-3=-843/0, 3-4=-2281/0, 4-18=-3061/0, 5-18=-3061/0, 5-6=-3061/0, 6-7=-1766/0, 7-8=-1766/0
 BOT CHORD 14-15=0/2281, 13-14=0/2281, 12-13=0/2281, 11-12=0/2425, 10-11=0/850
 WEBS 3-14=0/872, 4-13=-515/0, 1-15=0/1169, 3-15=-1957/0, 4-12=0/944, 5-12=-734/0, 6-12=0/777, 6-11=-806/0, 8-11=0/1121, 8-10=-1150/0

NOTES-

- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 2) Gable studs spaced at 1-4-0 oc.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION. Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 6-6-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)
 Vert 10-16=-13, 1-9=-67
 Concentrated Loads (lb)
 Vert: 18=-600(F)



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, OSB-89 and BCSI Building Component Safety information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

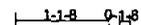


7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss F16	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761191
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

Job Reference (optional)
7.640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:06 2017 Page 1
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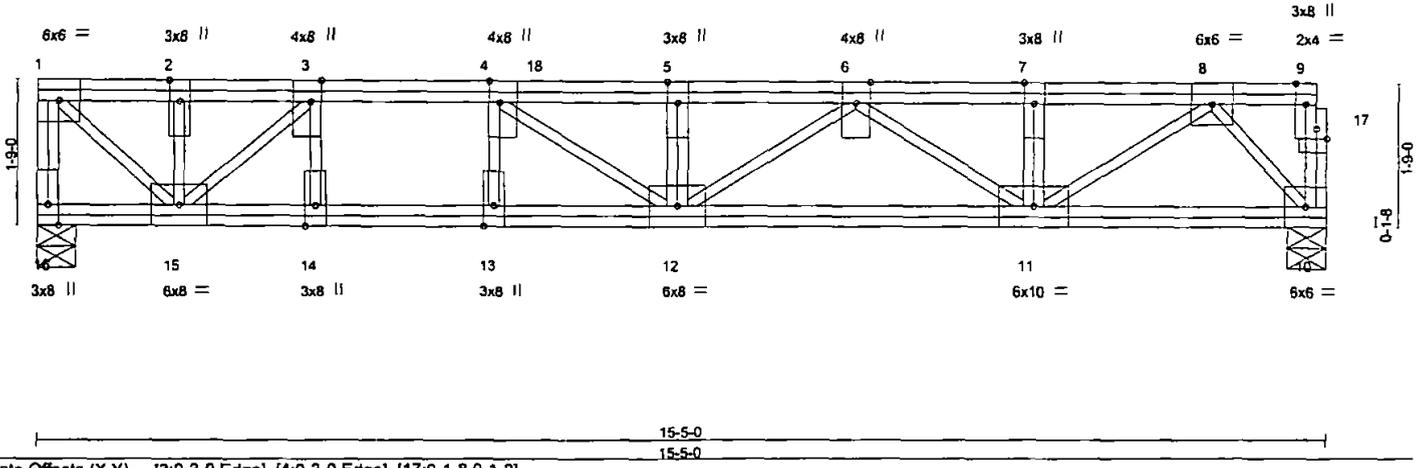


Plate Offsets (X,Y) - [3:0-3-0,Edge], [4:0-3-0,Edge], [17:0-1-8,0-1-8]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.87	in (oc) l/defl L/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.81	Vert(LL) -0.15 12-13 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.81	Vert(TL) -0.27 12-13 >683 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.02 10 n/a n/a		
	Code IRC2012/TPI2007			Weight: 103 lb	FT = 0%F, 0%E

LUMBER-
TOP CHORD 2x4 SPF No.2(flat)
BOT CHORD 2x4 SPF 1650F 1.5E(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 16=971/0-5-8, 10=838/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-16=905/0, 1-2=847/0, 2-3=847/0, 3-4=2288/0, 4-18=2838/0, 5-18=2838/0, 5-6=2838/0, 6-7=1574/0, 7-8=1574/0
BOT CHORD 14-15=0/2288, 13-14=0/2288, 12-13=0/2288, 11-12=0/2239, 10-11=0/663
WEBS 3-14=0/672, 4-13=520/0, 1-15=0/1174, 3-15=1962/0, 4-12=0/665, 5-12=545/0, 6-12=0/731, 6-11=813/0, 8-11=0/1114, 8-10=1014/0

- NOTES-**
- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 2) Gable studs spaced at 1-4-0 oc.
 - 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 5) CAUTION, Do not erect truss backwards.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 6-0-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 10-16=-13, 1-9=-67
Concentrated Loads (lb)
Vert: 18=-600(F)



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

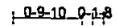
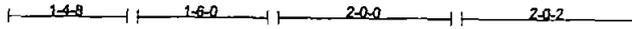
7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F17	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761192
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

Job Reference (optional)

7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:07 2017 Page 1
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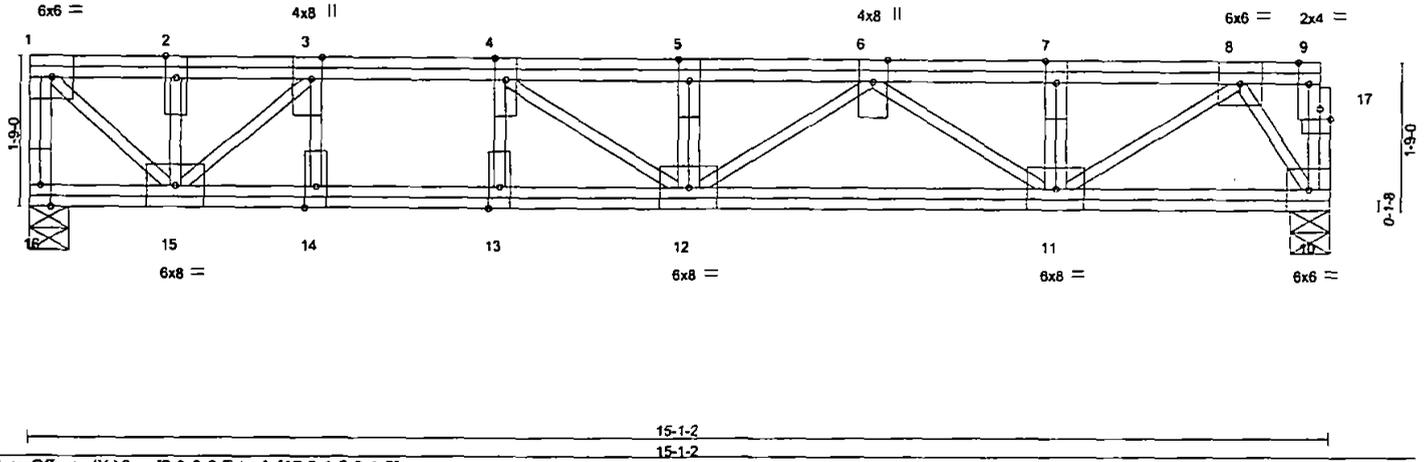


Plate Offsets (X,Y)- [3:0-3-0,Edge], [17:0-1-8,0-1-8]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	1-4-0 Plate Grip DOL 1.00	TC 0.73	Vert(LL)	-0.14 12-13	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL 1.00	BC 0.98	Vert(TL)	-0.25 12-13	>711	240		
BCLL 0.0	Rep Stress Incr NO	WB 0.82	Horz(TL)	0.02 10	n/a	n/a		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)						
							Weight: 102 lb	FT = 0%F, 0%E

LUMBER-

TOP CHORD 2x4 SPF No.2(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid coiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 16=976/0-5-8, 10=807/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-16=-916/0, 1-2=-857/0, 2-3=-857/0, 3-4=-2273/0, 4-5=-2599/0, 5-6=-2599/0, 6-7=-1385/0, 7-8=-1385/0
BOT CHORD 14-15=0/2273, 13-14=0/2273, 12-13=0/2273, 11-12=0/2054, 10-11=0/480
WEBS 3-14=0/596, 4-13=-460/0, 1-15=0/1188, 3-15=-1927/0, 4-12=0/395, 5-12=-375/0, 6-12=0/666, 6-11=-817/0, 8-11=0/1107, 8-10=-901/0

NOTES-

- All plates are 3x8 MT20 unless otherwise indicated.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 1-4-0 oc.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- CAUTION, Do not erect truss backwards.**
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 5-6-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 10-16=-13, 1-9=-67
Concentrated Loads (lb)
Vert: 4=-600(F)



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F18	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761183
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Foxworth Galbreith Lumber Co. Yuma, Az 85365

Job Reference (optional)
7,840 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:07 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8l-Qp_M6m_UatEXS8MCN6dkSvLP_xiozrBVp2eqTLz97Kc



0-1-8
0-5-13
Scale = 1:25.3

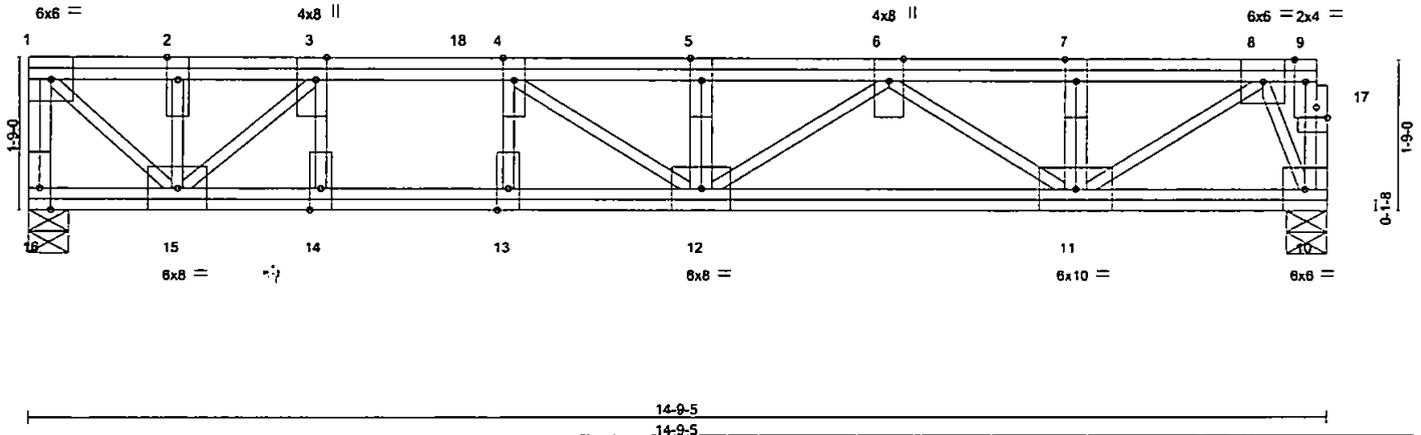


Plate Offsets (X,Y)-- [3:0-3-0,Edge], [17:0-1-8,0-1-8]

LOADING (psf)	SPACING-	CSL	DEFL	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.66	in (loc) l/defl L/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.85	Vert(LL) -0.12 12-13 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.83	Vert(TL) -0.21 12-13 >811 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.02 10 n/a n/a		
	Code IRC2012/TP12007			Weight: 100 lb	FT = 0%F, 0%E

LUMBER-
TOP CHORD 2x4 SPF No.2(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 16=978/0-5-8, 10=779/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-16=-928/0, 1-2=-870/0, 2-3=-870/0, 3-18=-2227/0, 4-18=-2227/0, 4-5=-2391/0, 5-6=-2391/0, 6-7=-1212/0, 7-8=-1212/0
BOT CHORD 14-15=0/2227, 13-14=0/2227, 12-13=0/2227, 11-12=0/1882, 10-11=0/306
WEBS 3-14=0/490, 4-13=-374/0, 1-15=0/1206, 3-15=-1847/0, 5-12=-269/0, 6-12=0/622, 6-11=-818/0, 8-11=0/1109, 8-10=-827/0

- NOTES-**
- All plates are 3x8 MT20 unless otherwise indicated.
 - Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - Gable studs spaced at 1-4-0 oc.
 - "Semi-rigid pitchbreaks including heets" Member end fixity model was used in the analysis and design of this truss.
 - Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - CAUTION, Do not erect truss backwards.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 4-11-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 10-16=-13, 1-9=-67
Concentrated Loads (lb)
Vert: 18=-600(F)



Expires: 3-31-2019
June 7, 2017

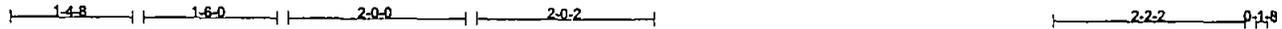
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with Mitek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general information regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, OSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job 00416-17Y	Truss F19	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761194
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Foxworth Galbraith Lumber Co., Yuma, Az 85385

Job Reference (optional)
7,840 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:08 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_14UWJzDd8i-u?YIJ6?7LBMO3HxOxq8z_6tbqLSllvodiON?oz97Kb



Scale = 1:25.0

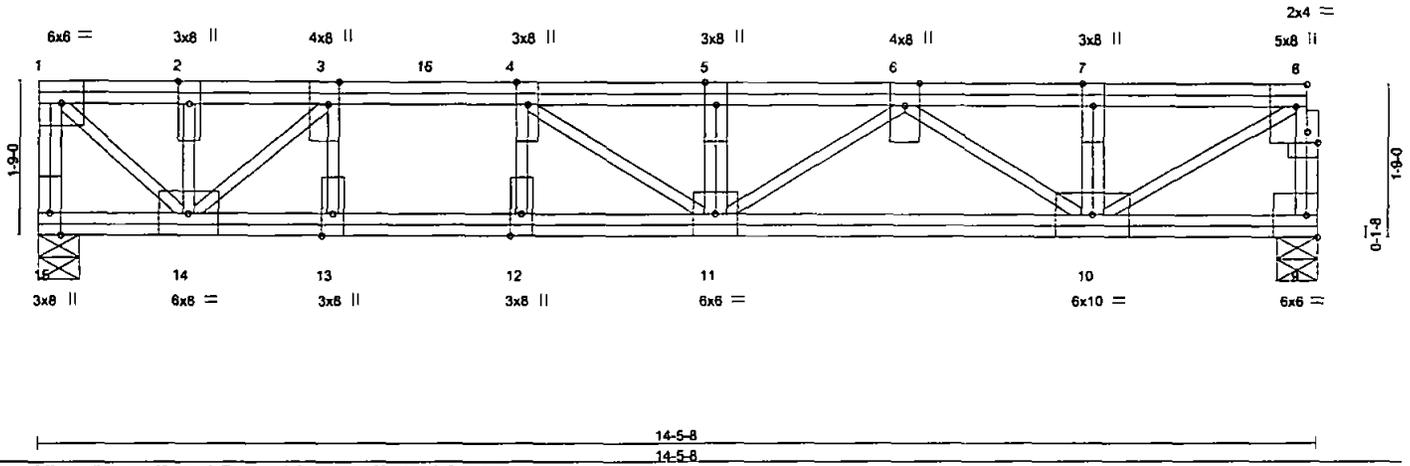


Plate Offsets (X,Y) - [3:0-3-0,Edge], [8:0-1-8,0-1-8], [8:0-3-0,Edge]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.59	in (loc) l/defl L/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.67	Vert(LL) -0.10 11-12 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.87	Vert(TL) -0.17 11-12 >999 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.02 9 n/a n/a		
	Code IRC2012/TPI2007			Weight: 97 lb	FT = 0%F, 0%E

LUMBER-

TOP CHORD 2x4 SPF No.2(flat)
 BOT CHORD 2x4 SPF No.2(flat)
 WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 15=987/0-5-8, 9=750/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-15=-949/0, 8-9=-733/0, 1-2=-897/0, 2-3=-897/0, 3-16=-2160/0, 4-16=-2160/0, 4-5=-2198/0, 5-6=-2198/0, 6-7=-1072/0, 7-8=-1073/0
 BOT CHORD 13-14=0/2160, 12-13=0/2160, 11-12=0/2160, 10-11=0/1723
 WEBS 3-13=0/340, 1-14=0/1244, 3-14=-1718/0, 6-11=0/581, 6-10=-794/0, 8-10=0/1256

NOTES-

- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 2) Gable studs spaced at 1-4-0 oc.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 4-5-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 9-15=-13, 1-8=-67
 Concentrated Loads (lb)
 Vert: 16=-600(F)



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Loe Street, Suite 312, Alexandria, VA 22314.

MiTek
 7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

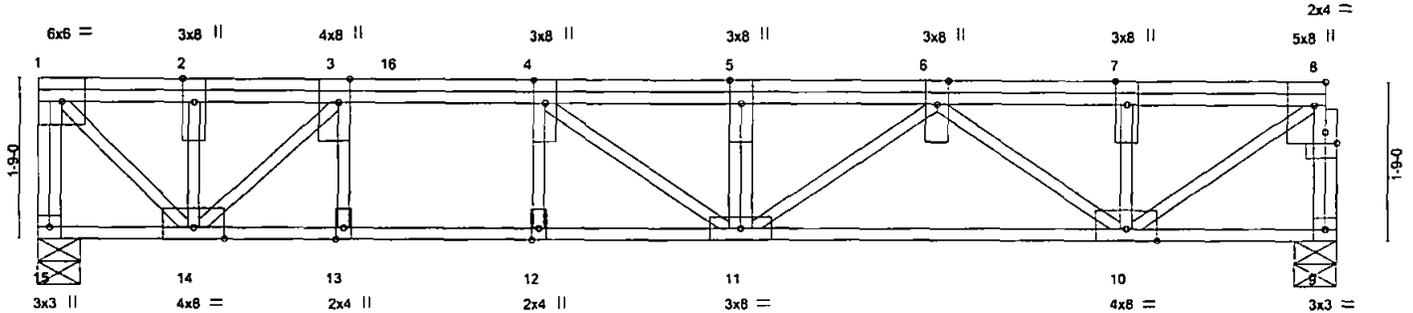
Job 00416-17Y	Truss F20	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761195
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:08 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_4UWJzDd8i-u7YlJ677LBMO3HxOxq8z_6lcmL39lIpodiON7oz97Kb



Scale: 1/2"=1'



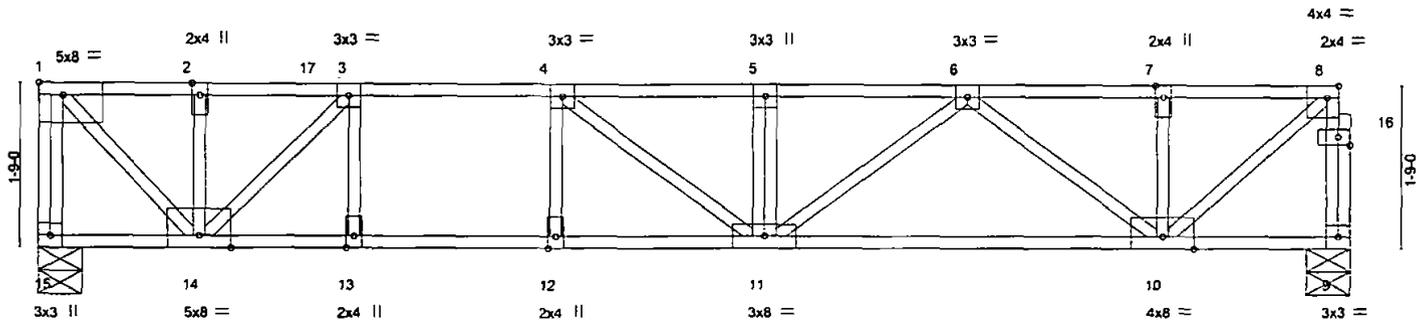
Job 00416-17Y	Truss F21	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761196
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Foxworth Galbreath Lumber Co, Yuma, Az 85365

Job Reference (optional)
7.840 s Apr 18 2016 MiTek Industries, Inc. Tue Jun 06 10:48:09 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_i4UWJzDd8l-MB67XS0i6UUFhRWbVxICXKQqplSy1k_osM7xYEz9?Ka



Scale = 1:23.5



3-4-8	4-4-8	5-4-8	13-9-14
3-4-8	1-0-0	1-0-0	8-5-8

Plate Offsets (X, Y) - [1:Edge,0-1-8], [8:0-1-8,Edge], [16:0-1-8,0-1-0]

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.38	Vert(LL)	-0.08 11-12	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.61	Vert(TL)	-0.13 11-12	>999	240		
BCLL 0.0	Rep Stress Incr	NO	WB 0.94	Horz(TL)	0.03 9	n/a	n/a		
BCDL 10.0	Code IRC2012/TPI2007		(Matrix)						
								Weight: 65 lb	FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF 2100F 1.8E(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 HF/SPF Stud/Std(flat)	

REACTIONS. (lb/size) 15=1019/0-5-8, 9=662/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-15=-1006/0, 9-16=-658/0, 8-16=-657/0, 1-2=-946/0, 2-17=-946/0, 3-17=-946/0, 3-4=-1684/0, 4-5=-1613/0, 5-6=-1613/0, 6-7=-698/0, 7-8=-698/0
 BOT CHORD 13-14=0/1684, 12-13=0/1684, 11-12=0/1684, 10-11=0/1233
 WEBS 1-14=0/1384, 3-14=-1043/0, 6-11=0/477, 6-10=-679/0, 8-10=0/909

- NOTES-
- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 2) Gable studs spaced at 1-4-0 oc.
 - 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 5) CAUTION, Do not erect truss backwards.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 2-11-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Dead + Floor Live (balanced); Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 9-15=-13, 1-8=-67
 Concentrated Loads (lb)
 Vert: 17=-600(F)



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.
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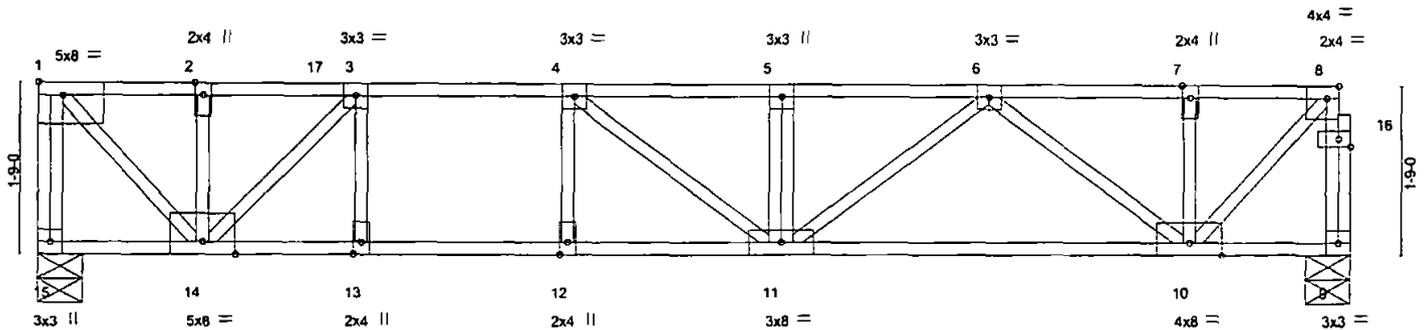
Job 00416-17Y	Truss F22	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761197
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Foxworth Galbraith Lumber Co, Yuma, Az 85385

7.840 s Apr 19 2016 Mitek Industries, Inc. Tue Jun 06 10:48:09 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8l-MB67XS0i6UUFhRWbVXfCXKQpDITw1kAosM7xYEz97Ka



Scale = 1:22.9



3-4-8	4-4-8	5-4-8	13-6-0
3-4-8	1-0-0	1-0-0	8-1-8

Plate Offsets (X,Y) - [1:Edge,0-1-8], [8:0-1-8,Edge], [16:0-1-8,0-1-0]

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.42	Vert(LL)	-0.06 11-12	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.55	Vert(TL)	-0.11 11-12	>999	240		
BCLL 0.0	Rep Stress Incr	NO	WB 0.93	Horz(TL)	0.03 9	n/a	n/a		
BCDL 10.0	Code IRC2012/TP12007		(Matrix)						

Weight: 64 lb FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF 2100F 1.8E(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purfins, except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 HF/SPF Stud/Std(flat)	

REACTIONS. (lb/size) 15=1004/0-5-8, 9=652/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-15=991/0, 9-16=652/0, 8-16=652/0, 1-2=933/0, 2-17=933/0, 3-17=933/0, 3-4=1641/0, 4-5=1537/0, 5-6=1537/0, 6-7=582/0, 7-8=582/0
 BOT CHORD 13-14=0/1641, 12-13=0/1641, 11-12=0/1641, 10-11=0/1139
 WEBS 1-14=0/1346, 2-14=-254/0, 3-14=-1002/0, 6-11=0/500, 6-10=708/0, 8-10=0/831

NOTES-

- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 2) Gable studs spaced at 1-4-0 oc.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 2-11-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)
 Vert: 9-15=13, 1-8=67
 Concentrated Loads (lb)
 Vert: 17=600(F)



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with Mitek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

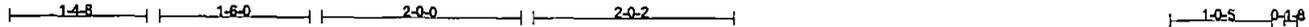


7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00418-17Y	Truss F23	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761198
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Foxworth Galbraith Lumber Co., Yuma, Az 85385

Job Reference (optional)
7,840 s Apr 19 2016 MITek Industries, Inc. Tue Jun 08 10:48:10 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_14UWJzDd8rOgVko0Ntoc6Jb5n2FAR4Xzu09qgmACx50tU4gz97KZ



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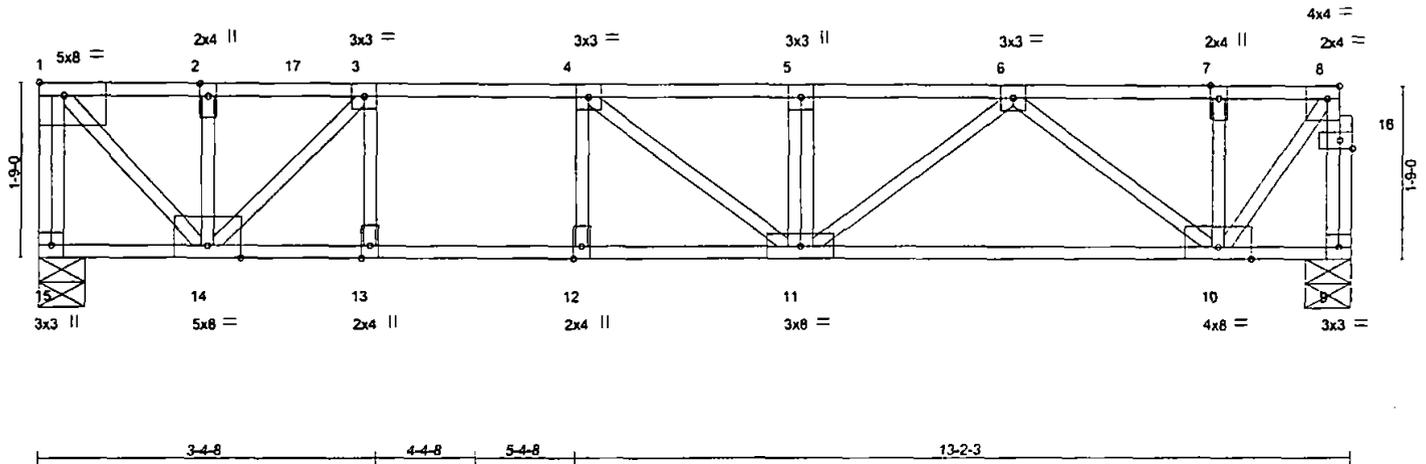


Plate Offsets (X,Y) - [1:Edge,0-1-8], [8:0-1-8,Edge], [16:0-1-8,0-1-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.81	in (loc) l/defl L/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.51	Vert(LL) -0.06 11-12 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.94	Vert(TL) -0.10 11-12 >999 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.03 9 n/a n/a		
	Code IRC2012/TPI2007			Weight: 63 lb	FT = 0%F, 0%E

LUMBER-

TOP CHORD 2x4 SPF 1650F 1.5E(flat)
 BOT CHORD 2x4 SPF No.2(flat)
 WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 15=1001/0-5-8, 9=629/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-15=-989/0, 9-16=-634/0, 8-16=-634/0, 1-2=-948/0, 2-17=-948/0, 3-17=-948/0, 3-4=-1530/0, 4-5=-1418/0, 5-6=-1418/0, 6-7=-457/0, 7-8=-457/0
 BOT CHORD 13-14=0/1530, 12-13=0/1530, 11-12=0/1530, 10-11=0/1019
 WEBS 1-14=0/1367, 2-14=-394/0, 3-14=-824/0, 6-11=0/501, 6-10=-714/0, 8-10=0/749

NOTES-

- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 2) Gable studs spaced at 1-4-0 oc.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 2-7-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 9-15=-13, 1-8=-67
 Concentrated Loads (lb)
 Vert: 17=600(F)



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

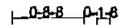


7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss F24	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbot	R50761199
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Foxworth Galbreath Lumber Co, Yuma, Az 85385

7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:10 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8i-rOgVko0Ntoc6Jb5n2FAR4Xzll9qEmADx50tU4gz9?KZ



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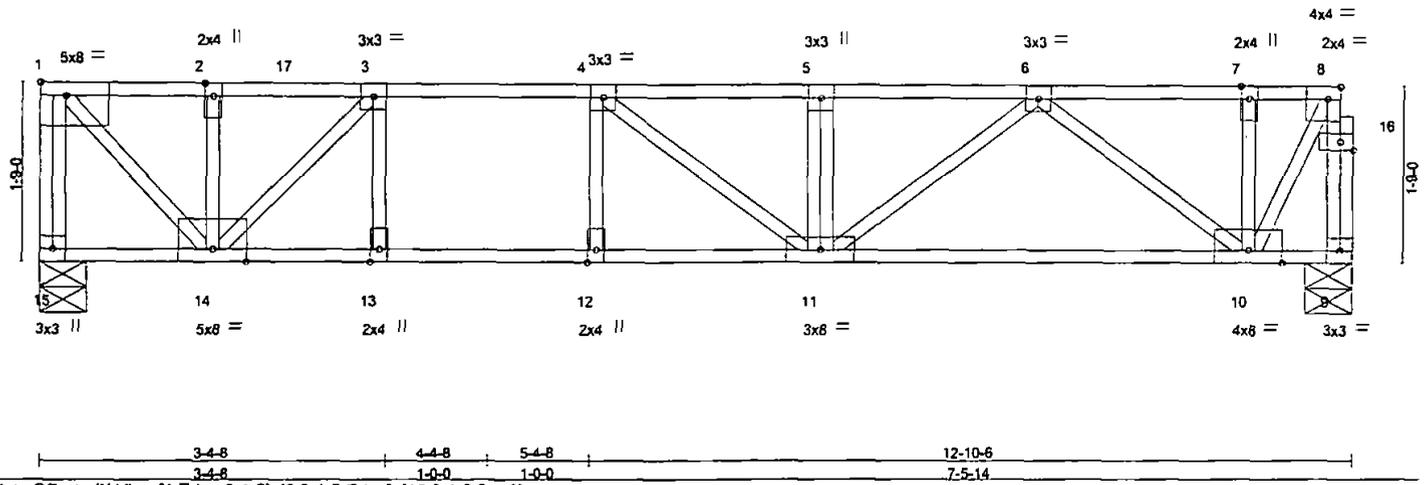


Plate Offsets (X,Y) - [1:Edge,0-1-8], [8:0-1-8,Edge], [16:0-1-8,0-1-0]

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.85	Vert(LL)	-0.05	11-12	>999	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.48	Vert(TL)	-0.09	10-11	>999		
BCLL 0.0	Rep Stress Incr	NO	WB 0.94	Horz(TL)	0.02	9	n/a		
BCDL 10.0	Code IRC2012/TPI2007		(Matrix)						
								Weight: 62 lb	FT = 0%F, 0%E

LUMBER-

TOP CHORD 2x4 SPF 1650F 1.5E(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purfins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 15=994/0-5-8, 9=611/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-15=-981/0, 9-16=-624/0, 8-16=-624/0, 1-2=-946/0, 2-17=-946/0, 3-17=-946/0, 3-4=-1447/0, 4-5=-1320/0, 5-6=-1320/0, 6-7=-340/0, 7-8=-340/0
BOT CHORD 13-14=0/1447, 12-13=0/1447, 11-12=0/1447, 10-11=0/913
WEBS 1-14=0/1365, 2-14=-473/0, 3-14=-708/0, 6-11=0/512, 6-10=-728/0, 8-10=0/693

NOTES-

- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 2) Gable studs spaced at 1-4-0 oc.
- 3) *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 2-5-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 9-15=-13, 1-8=-67
Concentrated Loads (lb)
Vert: 17=-600(F)



Expires: 3-31-2019
June 7.2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and ECSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F25	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761200
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Foxworth Galbraith Lumber Co, Yuma, Az 85385

Job Reference (optional)
7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:11 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_4UWJzDd8l-JaEty71?e6kzWgzcyhgkV3pZAqVdZ5Jgc1c7z9?KY



0-1-8
0-4-11
Scale = 1:21.3

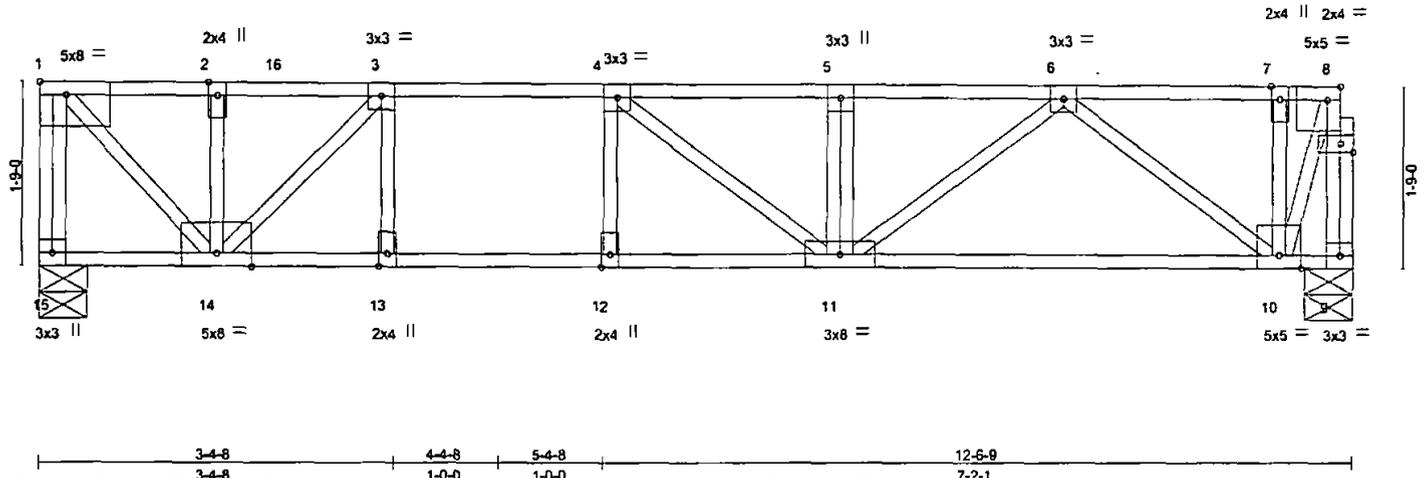


Plate Offsets (X,Y)-- [1:Edge,0-1-8], [8:0-1-8,0-1-0], [8:0-1-8,Edge]

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 40.0	Plate Grip DOL	1.00	TC 0.80	Vert(LL)	-0.05	12	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.46	Vert(TL)	-0.08	10-11	>999	240		
BCLL 0.0	Rep Stress Incr	NO	WB 0.94	Horz(TL)	0.02	9	n/a	n/a		
BCDL 10.0	Code IRC2012/TPI2007		(Matrix)							
									Weight: 61 lb	FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF 1650F 1.5E(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 HF/SPF Stud/Std(flat)	

REACTIONS. (lb/size) 15=985/0-5-8, 9=599/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-15=-973/0, 8-9=-624/0, 1-2=-940/0, 2-16=-940/0, 3-16=-940/0, 3-4=-1369/0, 4-5=-1226/0, 5-6=-1226/0
 BOT CHORD 13-14=0/1369, 12-13=0/1369, 11-12=0/1369, 10-11=0/809
 WEBS 1-14=0/1356, 2-14=-537/0, 3-14=-607/0, 6-11=0/524, 6-10=-742/0, 8-10=0/673

- NOTES-
- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 2) Gable studs spaced at 1-4-0 oc.
 - 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 5) CAUTION, Do not erect truss backwards.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 2-3-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 9-15=-13, 1-8=-67
 Concentrated Loads (lb)
 Vert: 16=-600(F)



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.
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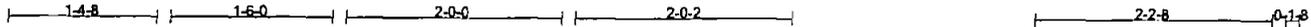
7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F26	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761201
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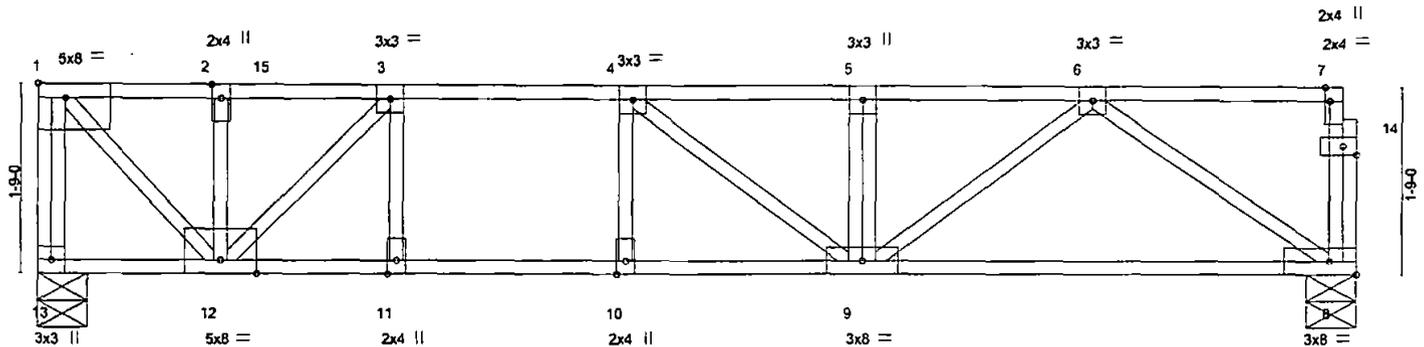
Foxworth Galbreith Lumber Co, Yuma, Az 85385

Job Reference (optional)

7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:11 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_14UWJzDd8i-JaEty717e6kzwwgzcynqckV7vZAVvdk5Jgc1c7z9?KY



Scale = 1:20.7



3-4-8	4-4-8	5-4-8	11-11-12	12-2-12
3-4-8	1-0-0	1-0-0	6-7-4	0-3-0

Plate Offsets (X,Y)- [1:Edge,0-1-8], [7:0-1-8,Edge], [14:0-1-8,0-1-0]

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.53	Vert(LL)	-0.04	10	>999	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.48	Vert(TL)	-0.10	8-9	>999		
BCLL 0.0	Rep Stress Incr	NO	WB 0.93	Horz(TL)	0.02	8	n/a		
BCDL 10.0	Code IRC2012/TPI2007		(Matrix)						
								Weight: 57 lb	FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF 2100F 1.8E(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 HF/SPF Stud/Std(flat)	

REACTIONS. (lb/size) 13=977/0-5-8, 8=577/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-13=-965/0, 1-2=-929/0, 2-15=-929/0, 3-15=-929/0, 3-4=-1291/0, 4-5=-1138/0, 5-6=-1138/0
 BOT CHORD 11-12=0/1291, 10-11=0/1291, 9-10=0/1291, 8-9=0/707
 WEBS 1-12=0/1341, 2-12=-593/0, 3-12=-511/0, 6-9=0/541, 6-8=-859/0

- NOTES-
- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 2) Gable studs spaced at 1-4-0 oc.
 - 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 5) CAUTION. Do not erect truss backwards.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 2-2-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 8-13=-13, 1-7=-67
 Concentrated Loads (lb)
 Vert: 15=-600(F)



Expires: 3-31-2019
June 7, 2017

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7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

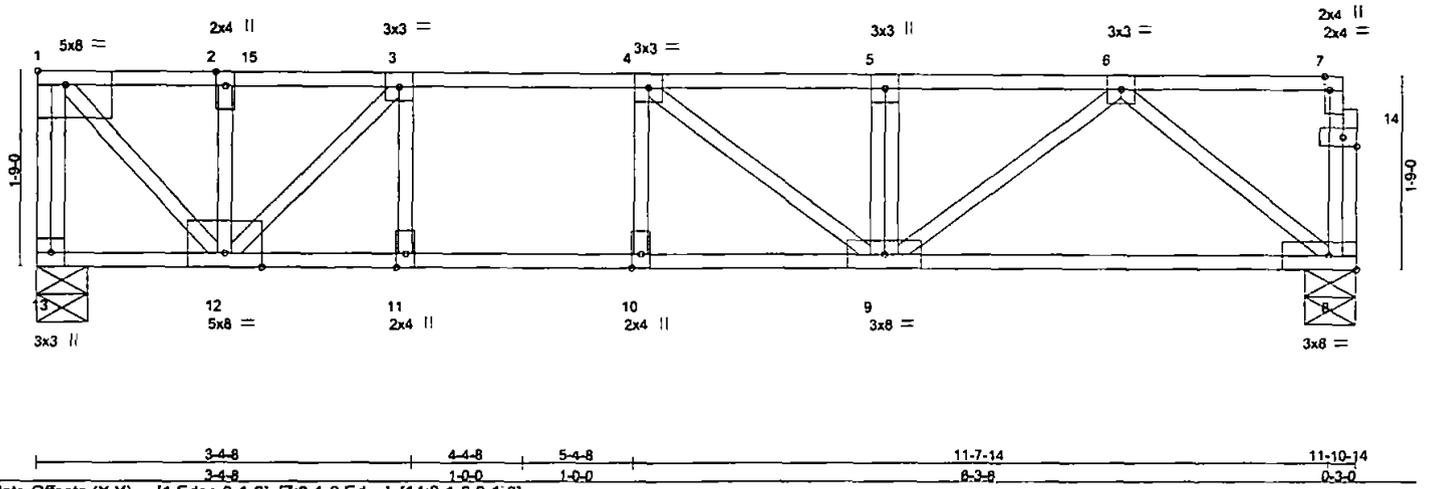
Job 00416-17Y	Truss F27	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761202
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Foxworth Galbraith Lumber Co. Yuma, Az 85365

Job Reference (optional)
7.640 e Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:11 2017 Page 1
ID:Z_OFgkz7ob1GlpCh_t4UWJzDd8l-JaEty71?e6kzwpzcyhgckVAOZB6Vd75Jgc1c7z9?KY



Scale = 1:20.1



LOADING (psf)	SPACING-	CSI.	DEFL	in (loc)	U/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.38	Vert(LL) -0.04	10	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL 1.00	BC 0.44	Vert(TL) -0.08	8-9	>999	240		
BCLL 0.0	Rep Stress Incr NO	WB 0.91	Horz(TL) 0.02	8	n/a	n/a		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)						
							Weight: 56 lb	FT = 0%F, 0%E

LUMBER-
TOP CHORD 2x4 SPF 2100F 1.8E(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 13=970/0-5-8, 8=559/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-13=-957/0, 1-2=-913/0, 2-15=-913/0, 3-15=-913/0, 3-4=-1207/0, 4-5=-1042/0, 5-6=-1042/0
BOT CHORD 11-12=0/1207, 10-11=0/1207, 9-10=0/1207, 8-9=0/601
WEBS 1-12=0/1317, 2-12=-644/0, 3-12=-417/0, 6-9=0/554, 6-8=-770/0

- NOTES-**
- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 2) Gable studs spaced at 1-4-0 oc.
 - 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 5) CAUTION, Do not erect truss backwards.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 2-0-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)
Vert: 8-13=-13, 1-7=-67

Concentrated Loads (lb)
Vert: 15=-600(F)



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-1473 rev. 10/03/2015 BEFORE USE.
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7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F28	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761203
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Foxworth Galbraith Lumber Co, Yuma, Az 85385

7.840 s Apr 19 2016 MITek Industries, Inc. Tue Jun 08 10:48:12 2017 Page 1
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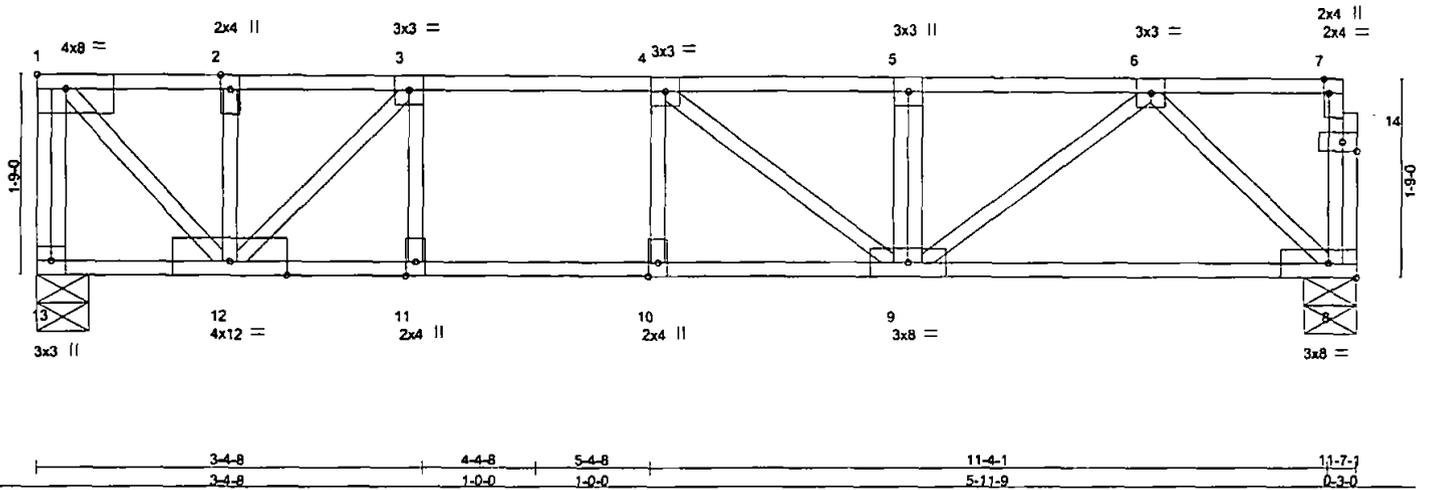


Plate Offsets (X,Y)- [1:Edge,0-1-8], [7:0-1-8,Edge], [14:0-1-8,0-1-0]					
LOADING (psf)	SPACING- 1-4-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.21	Vert(LL) -0.05 9-10 >999 360	MT20	185/144
TCDL 10.0	Lumber DOL 1.00	BC 0.44	Vert(TL) -0.08 9-10 >999 240		
BCLL 0.0	Rep Stress Incr NO	WB 0.87	Horz(TL) 0.02 8 n/a n/a		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)		Weight: 55 lb	FT = 0%F, 0%E

LUMBER-	BRACING-
TOP CHORD 2x4 SPF No.2(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. except end verticals.
BOT CHORD 2x4 SPF No.2(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 HF/SPF Stud/Std(flat)	

REACTIONS. (lb/size) 13=971/0-5-8, 8=532/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-13=956/0, 1-2=872/0, 2-3=872/0, 3-4=1090/0, 4-5=931/0, 5-6=931/0
 BOT CHORD 11-12=0/1090, 10-11=0/1090, 9-10=0/1090, 8-9=0/491
 WEBS 1-12=0/1259, 2-12=689/0, 3-12=-308/0, 6-9=0/553, 6-8=-679/0

NOTES-

- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 2) Gable studs spaced at 1-4-0 oc.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 1-10-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 8-13=13, 1-7=67
 Concentrated Loads (lb)
 Vert: 2=600(F)



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-1473 rev. 10/03/2015 BEFORE USE.
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Job 00416-17Y	Truss F29	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761204
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Foxworth Galbreith Lumber Co, Yuma, Az 85385

7,640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:12 2017 Page 1
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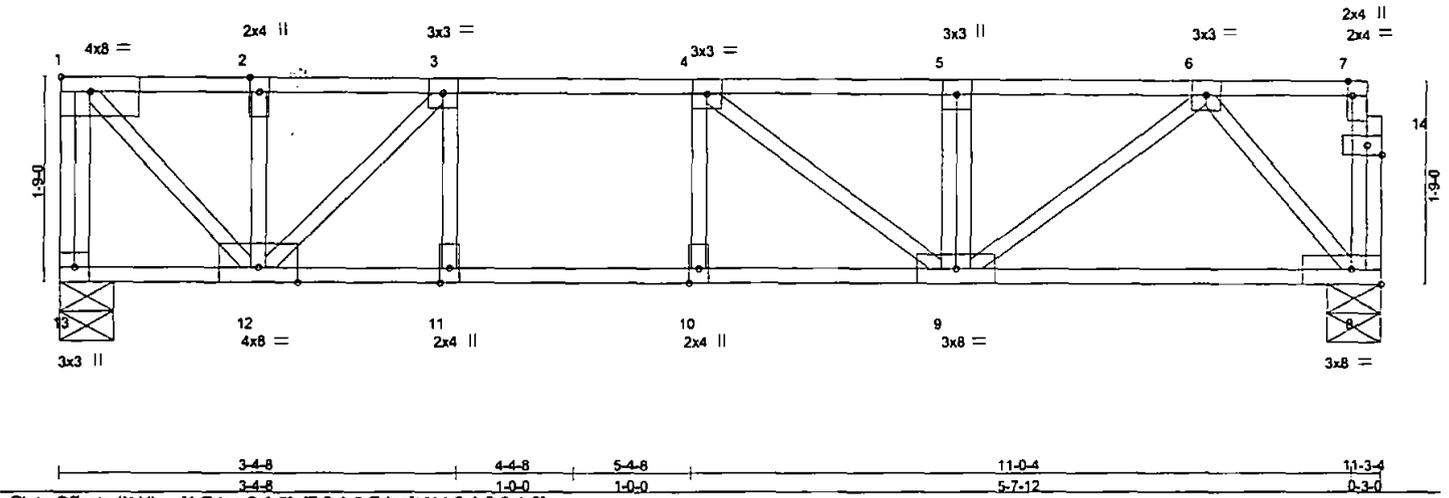


Plate Offsets (X,Y)- [1:Edge,0-1-8], [7:0-1-8,Edge], [14:0-1-8,0-1-0]

LOADING (psf)	SPACING-	CSI.	DEFL	in (loc)	l/defl	U/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.16	Vert(LL)	-0.03	10	>999	MT20	185/144
TCDL 10.0	Lumber DOL 1.00	BC 0.37	Vert(TL)	-0.06	10	>999		
BCLL 0.0	Rep Stress Incr NO	WB 0.86	Horz(TL)	0.01	8	n/a		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)						
							Weight: 54 lb	FT = 0%F, 0%E

LUMBER-
TOP CHORD 2x4 SPF No.2(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 13=956/0-5-8, 8=522/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-13=-942/0, 1-2=-860/0, 2-3=-860/0, 3-4=-1050/0, 4-5=-859/0, 5-6=-859/0
BOT CHORD 11-12=0/1050, 10-11=0/1050, 9-10=0/1050, 8-9=0/402
WEBS 1-12=0/1241, 2-12=-697/0, 3-12=-269/0, 6-9=0/574, 6-8=-620/0

- NOTES-**
- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 2) Gable studs spaced at 1-4-0 oc.
 - 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 5) CAUTION, Do not erect truss backwards.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 1-8-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Floor Live (balanced): Lumber Increase=1.00. Plate Increase=1.00
Uniform Loads (plf)
Vert: 8-13=-13, 1-7=-67
Concentrated Loads (lb)
Vert: 2=-600(F)



Expires: 3-31-2019
June 7, 2017

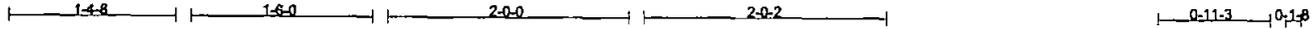
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

7777 Groatback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F30	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761205
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Foxworth Galbreith Lumber Co, Yuma, Az 85385

7.640 g Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:13 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_14UWJzDd8i-FzLeNp3FAJ_hA3qMkNj8h9bZbM8zYUNn_58h7z9?KW



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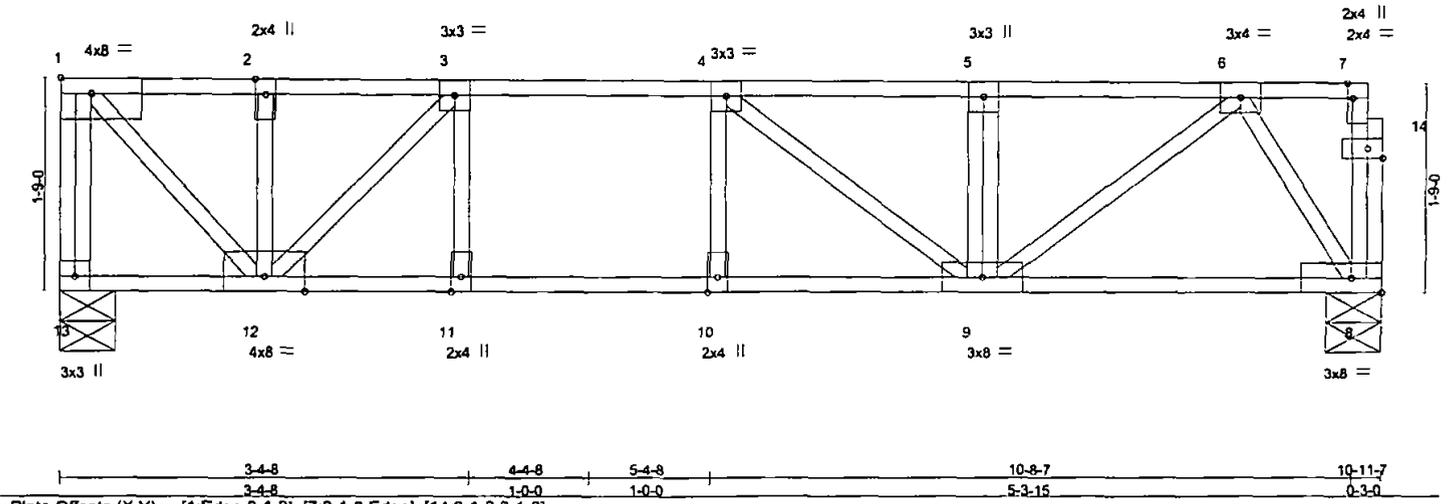


Plate Offsets (X,Y) - [1:Edge,0-1-8], [7:0-1-8,Edge], [14:0-1-8,0-1-0]

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 40.0	Plate Grip DOL	1.00	TC 0.14	Vert(LL)	-0.03	10	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.34	Vert(TL)	-0.04	10-11	>999	240		
BCLL 0.0	Rep Stress Incr	NO	WB 0.84	Horz(TL)	0.01	8	n/a	n/a		
BCDL 10.0	Code IRC2012/TP12007		(Matrix)							
									Weight: 53 lb	FT = 0%F, 0%E

LUMBER-
TOP CHORD 2x4 SPF No.2(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 13=941/0-5-8, 8=512/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-13=-929/0, 1-2=-848/0, 2-3=-848/0, 3-4=-1010/0, 4-5=-785/0, 5-6=-785/0
BOT CHORD 11-12=0/1010, 10-11=0/1010, 9-10=0/1010, 8-9=0/312
WEBS 1-12=0/1224, 2-12=-705/0, 4-9=-280/0, 6-9=0/594, 6-8=-572/0

- NOTES-**
- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 2) Gable studs spaced at 1-4-0 oc.
 - 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 5) CAUTION, Do not erect truss backwards.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 1-6-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)
Vert: 8-13=-13, 1-7=-67

Concentrated Loads (lb)
Vert: 2=-600(F)



Expires: 3-31-2019
June 7, 2017

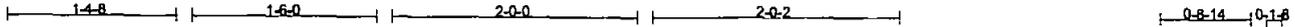
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

7777 Groenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F31	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761206
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

7.840 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 08 10:48:13 2017 Page 1
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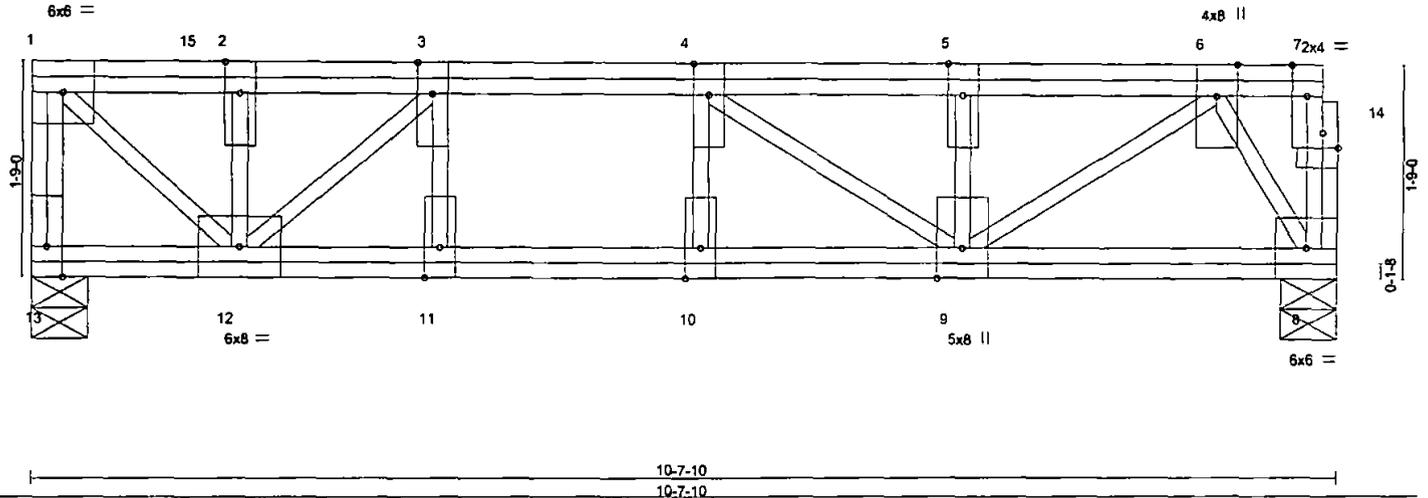


Plate Offsets (X,Y) - [14:0-1-8,0-1-8]

LOADING (psf)	SPACING-	CSL	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.25	Vert(LL) -0.01	11	>999	360	MT20	185/144
TCOL 10.0	Lumber DOL 1.00	BC 0.18	Vert(TL) -0.03	10-11	>999	240		
BCLL 0.0	Rep Stress Incr NO	WB 0.85	Horz(TL) 0.01	8	n/a	n/a		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)						
							Weight: 71 lb	FT = 0%F, 0%E

LUMBER-

TOP CHORD 2x4 SPF No.2(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 13=1028/0-5-8, 8=498/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-13=-1006/0, 1-15=-893/0, 2-15=-893/0, 2-3=-893/0, 3-4=-1010/0, 4-5=-748/0, 5-6=-748/0
BOT CHORD 11-12=0/1010, 10-11=0/1010, 9-10=0/1010, 8-9=0/274
WEBS 1-12=0/1237, 2-12=-751/0, 4-9=-321/0, 6-9=0/586, 6-8=-542/0

NOTES-

- All plates are 3x8 MT20 unless otherwise indicated.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 1-4-0 oc.
- "Semi-rigid pitchbreaks including heels" Member and fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- CAUTION, Do not erect truss backwards.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 700 lb down at 1-4-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 8-13=-13, 1-7=-67
Concentrated Loads (lb)
Vert: 15=-700(F)



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

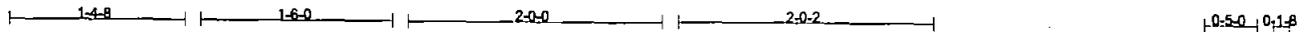


7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F32	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761207
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Foxworth Galbraith Lumber Co, Yuma, Az 85385

7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:14 2017 Page 1
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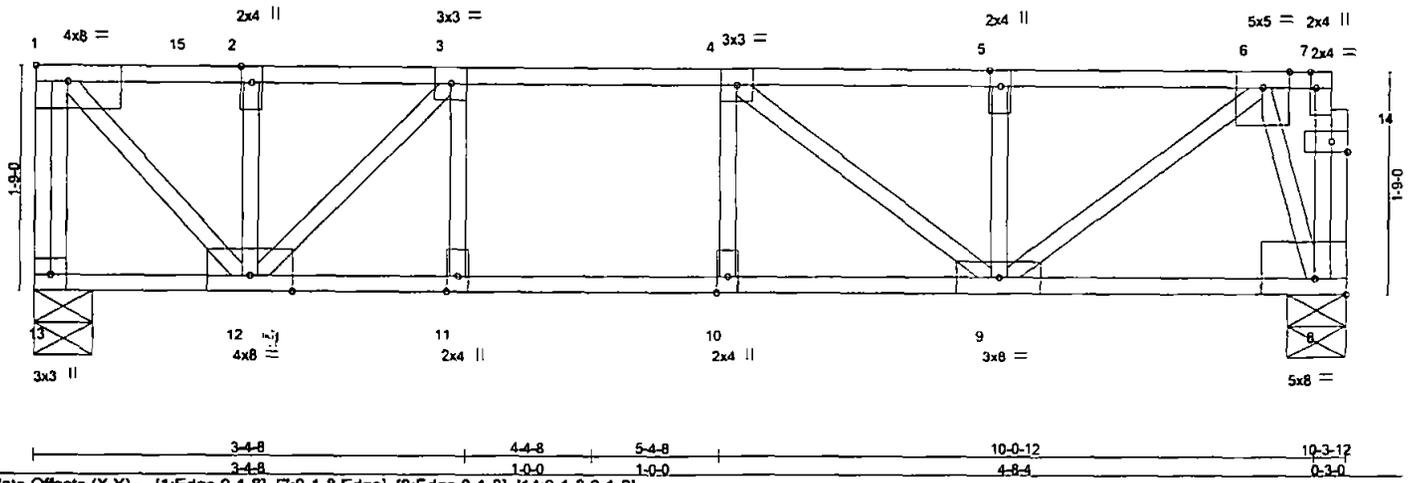


Plate Offsets (X, Y) - [1:Edge,0-1-8], [7:0-1-8,Edge], [8:Edge,0-1-8], [14:0-1-8,0-1-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.56	in (loc) l/defl L/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.28	Vert(LL) -0.02 10 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.72	Vert(TL) -0.04 10 >999 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 8 n/a n/a		
	Code IRC2012/TPI2007			Weight: 50 lb	FT = 0%F, 0%E

LUMBER-
TOP CHORD 2x4 SPF 2100F 1.8E(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 13=938/0-5-8, 8=463/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-13=-926/0, 1-15=-721/0, 2-15=-721/0, 2-3=-721/0, 3-4=-822/0, 4-5=-603/0, 5-6=-603/0
BOT CHORD 11-12=0/822, 10-11=0/822, 9-10=0/822
WEBS 1-12=0/1041, 2-12=-637/0, 4-9=-275/0, 6-9=0/569, 6-8=-495/0

- NOTES-**
- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 2) Gable studs spaced at 1-4-0 oc.
 - 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 5) CAUTION, Do not erect truss backwards.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 1-2-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 8-13=-13, 1-7=-67
Concentrated Loads (lb)
Vert: 15=-600(F)



Expires: 3-31-2019
June 7, 2017

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.</p>	 7777 Greenback Lane Suite 109 Citrus Heights, CA 95610
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Job 00416-17Y	Truss F34	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761209
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

7.840 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 08 10:48:14 2017 Page 1
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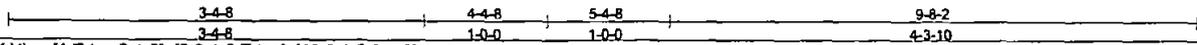
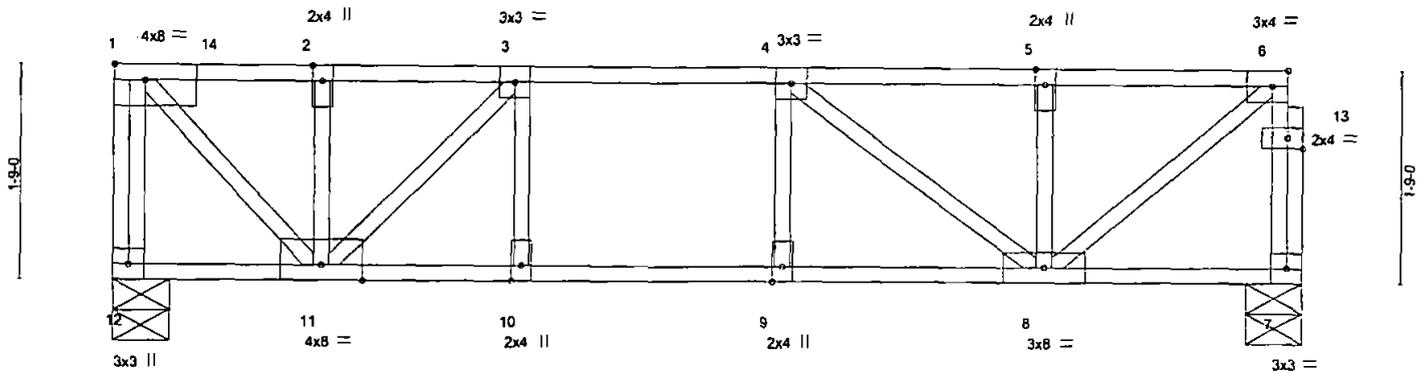


Plate Offsets (X,Y) - [1:Edge,0-1-8], [6:0-1-8,Edge], [13:0-1-8,0-1-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	2-0-0	TC 0.80	in (loc) l/defl L/d	MT20	185/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.34	Vert(LL) -0.03 9 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.74	Vert(TL) -0.05 9 >999 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 7 n/a n/a		
	Code IRC2012/TPI2007			Weight: 46 lb	FT = 0%F, 0%E

LUMBER-

TOP CHORD 2x4 SPF 2100F 1.8E(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purtins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 12=1118/0-5-8, 7=607/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-12=-1101/0, 7-13=-593/0, 6-13=-592/0, 1-14=-747/0, 2-14=-747/0, 2-3=-747/0, 3-4=-942/0, 4-5=-633/0, 5-6=-633/0
BOT CHORD 10-11=0/942, 9-10=0/942, 8-9=0/942
WEBS 1-11=0/1078, 2-11=-563/0, 3-11=-275/0, 4-8=-388/0, 6-8=0/801

NOTES-

- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 2) Gable studs spaced at 1-4-0 oc.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION. Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 0-10-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 7-12=-20, 1-6=-100
Concentrated Loads (lb)
Vert: 14=-600(F)



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

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7777 Greenbeck Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss F35	Truss Type FLOOR SUPPORTED GABL	Qty 1	Ply 1	Tony Abbott	R50761210
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Foxworth Galbraith Lumber Co, Yuma, Az 85385

Job Reference (optional)
7.840 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:15 2017 Page 1
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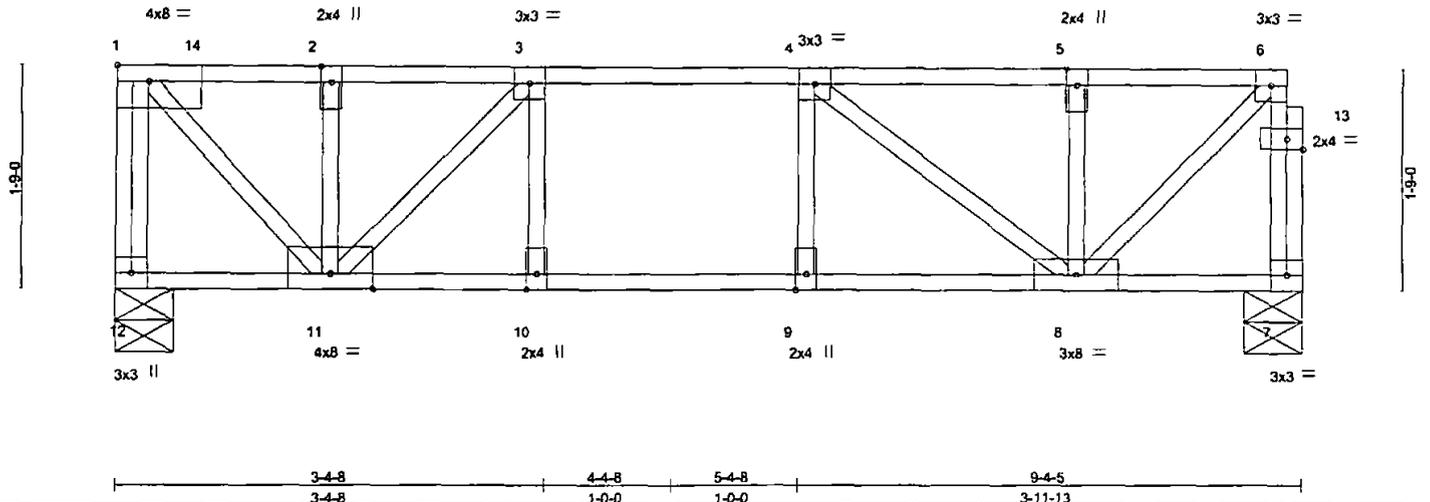


Plate Offsets (X,Y)- [1:Edge,0-1-8], [13:0-1-8,0-1-0]

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 40.0	Plate Grip DOL	1.00	TC 0.96	Vert(LL)	-0.01	10	>999	360	MT20	185/144
TCDL 10.0	Lumber DOL	1.00	BC 0.20	Vert(TL)	-0.02	9-10	>999	240		
BCLL 0.0	Rep Stress Incr	NO	WB 0.52	Horz(TL)	0.01	7	n/a	n/a		
BCDL 10.0	Code IRC2012/TP12007		(Matrix)							
									Weight: 45 lb	FT = 0%F, 0%E

LUMBER-
TOP CHORD 2x4 SPF 1650F 1.5E(flat)
BOT CHORD 2x4 SPF No.2(flat)
WEBS 2x4 HF/SPF Stud/Std(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid coiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 12=926/0-5-8, 7=399/0-5-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-12=915/0, 7-13=390/0, 6-13=390/0, 1-14=524/0, 2-14=524/0, 2-3=524/0, 3-4=607/0, 4-5=359/0, 5-6=359/0
BOT CHORD 10-11=0/607, 9-10=0/607, 8-9=0/607
WEBS 1-11=0/756, 2-11=442/0, 4-8=312/0, 6-8=0/490

- NOTES-**
- 1) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 2) Gable studs spaced at 1-4-0 oc.
 - 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 5) CAUTION, Do not erect truss backwards.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 600 lb down at 0-8-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 7-12=13, 1-6=67
Concentrated Loads (lb)
Vert: 14=600(F)



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-1473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 216 N. Lee Street, Suite 312, Alexandria, VA 22314.

7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss T	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761213
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Foxworth Galbraith Lumber Co. Yuma, Az 85365

Job Reference (optional)
7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:20 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8l-YJGHRc8cXsthV7sieLLnTeNk4BD46qvPOal0R5z9?Kp

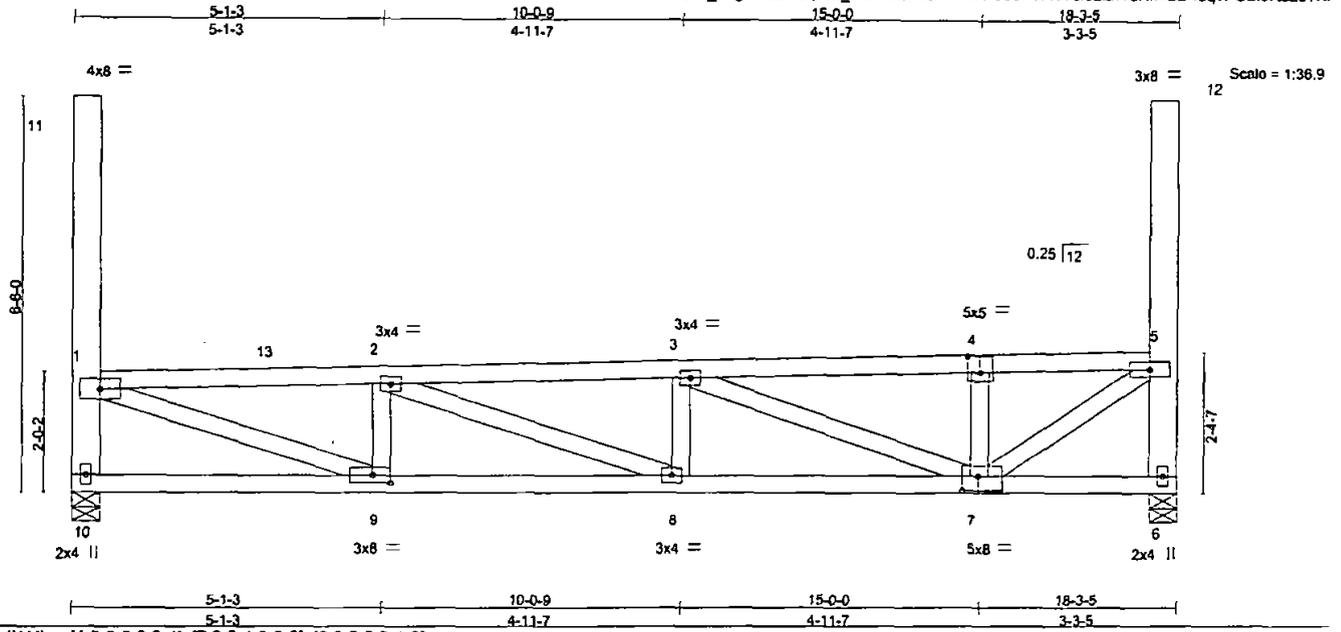


Plate Offsets (X,Y) - [4:0-2-8,0-3-4], [7:0-3-4,0-3-0], [9:0-3-8,0-1-8]

LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.31	in (loc) l/defl L/d	MT20	197/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.51	Vert(LL) -0.10 8-9 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.42	Vert(TL) -0.17 8-9 >999 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.03 6 n/a n/a		
	Code IRC2012/TPI2007			Weight: 89 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 4-2-14 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 7-3-11 oc bracing.
WEBS 2x4 SPF No.2 *Except* 10-11,6-12: 2x6 SPF 1650F 1.5E	

REACTIONS. (lb/size) 10=713/0-5-8, 6=713/0-5-8
Max Horz 10=300(LC 9)
Max Uplift 10=159(LC 8), 6=99(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-10=-675/184, 1-13=-1464/429, 2-13=-1460/429, 2-3=-1711/455, 3-4=-955/342,
4-5=-957/345, 5-6=-692/181
BOT CHORD 9-10=-656/661, 8-9=-540/1460, 7-8=-338/1707
WEBS 2-9=-431/175, 4-7=-288/91, 1-9=-407/1475, 2-8=-272/328, 3-7=-811/299,
5-7=-253/1131

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cal. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 18-0-9 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 10 and 99 lb uplift at joint 6.
- *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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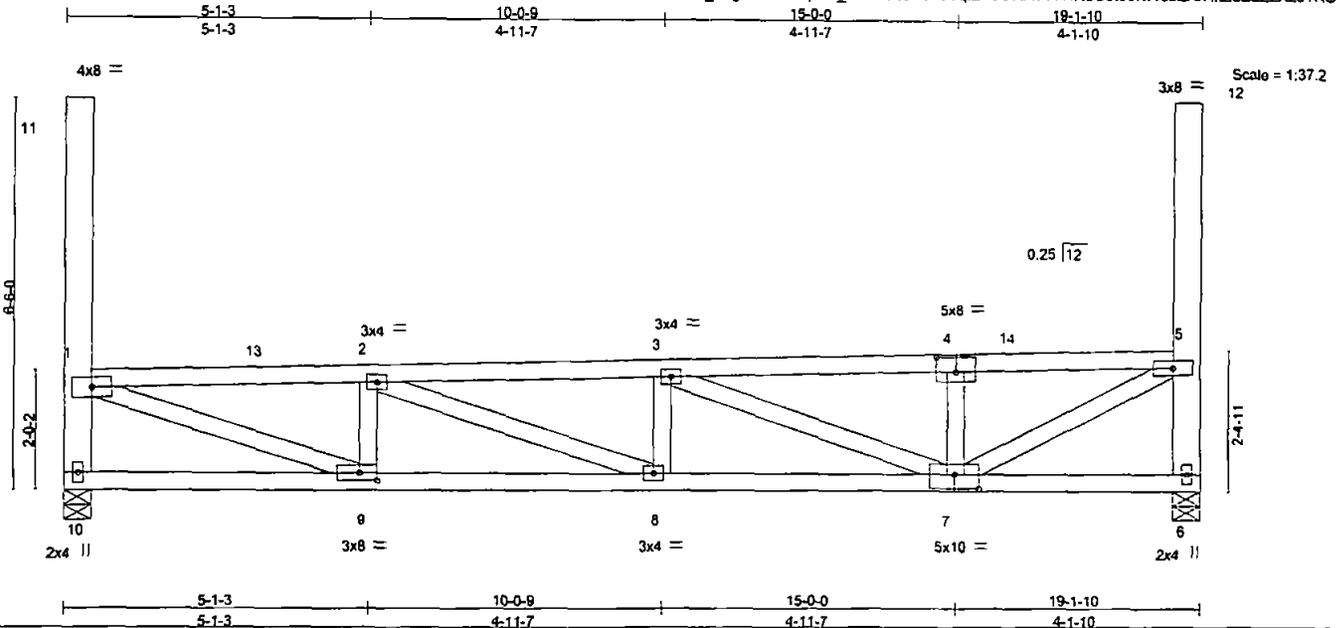


7777 Groenback Lane
Suite 109
Citrus Heights, CA 95610

Job	Truss	Truss Type	Qty	Ply	Tony Abbott	R50761214
00416-17Y	T1	SLOPING FLAT	1	1		

Foxworth Galbraith Lumber Co. Yuma, Az 85385

7,640 s Apr 19 2016 Mittek Industries, Inc. Tue Jun 08 10:48:21 2017 Page 1
 ID:Z_0Fgkz7ob1GlpCh_l4UWJzDd8i-0Vqf2Y9GHA?Y7HRuC3t00rwwsbZxrhkZdE2ZzXz97KO



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.31	in (loc) l/defl L/d	MT20	197/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.56	Vert(LL) -0.11 8-9 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.44	Vert(TL) -0.20 8-9 >999 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.03 6 n/a n/a		
	Code IRC2012/TP12007			Weight: 92 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SPF No.2
 BOT CHORD 2x4 SPF No.2
 WEBS 2x4 SPF No.2 *Except*
 10-11,6-12: 2x6 SPF 1650F 1.5E

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 4-0-3 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 7-3-15 oc bracing.

REACTIONS. (lb/size) 10=747/0-5-8, 6=747/0-5-8
 Max Horz 10=300(LC 11)
 Max Uplift 10=161(LC 8), 6=104(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-10=-709/183, 1-13=-1555/441, 2-13=-1551/442, 2-3=-1885/479, 3-4=-1209/377,
 4-14=-1211/380, 5-14=-1208/380, 5-6=-718/182
 BOT CHORD 9-10=-652/657, 8-9=-556/1552, 7-8=-362/1881
 WEBS 2-9=-463/174, 4-7=-309/95, 1-9=-403/1571, 2-8=-268/390, 3-7=-725/280,
 5-7=-292/1332

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 18-10-14 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 10 and 104 lb uplift at joint 6.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

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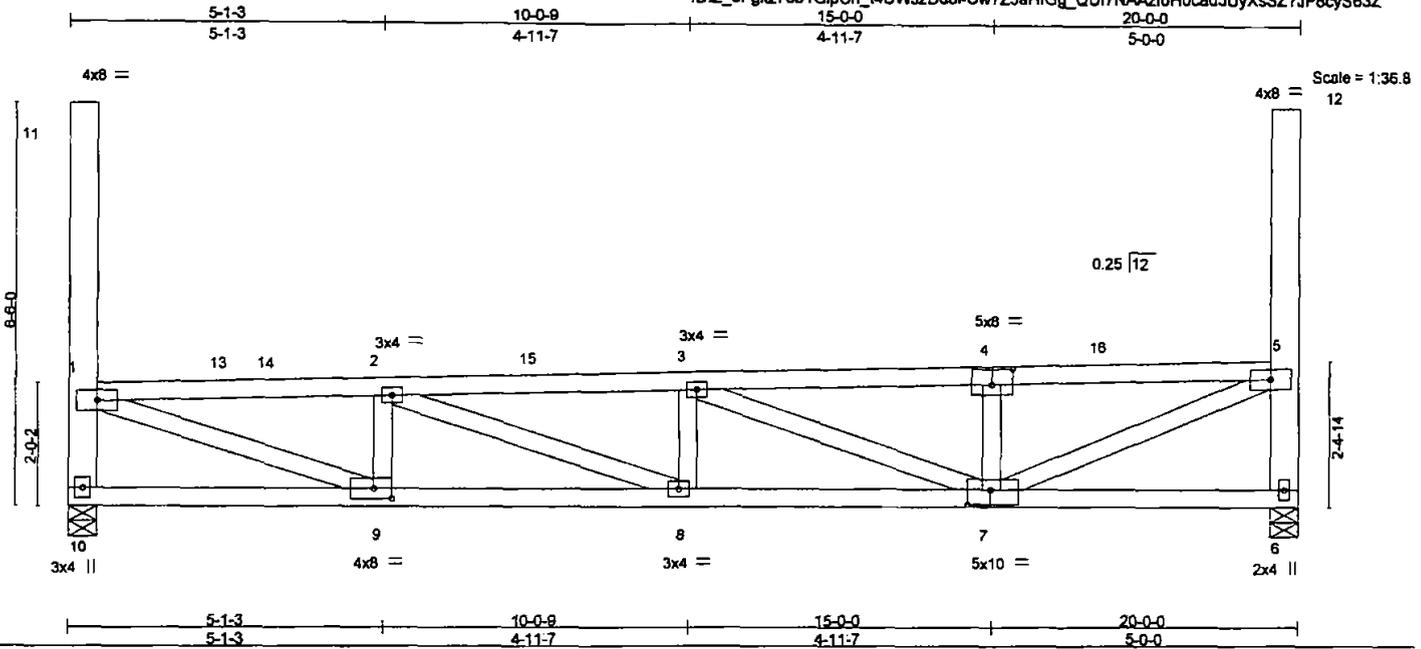


7777 Groenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss T2	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R52054349
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Foxworth Galbraith Lbr Co, Yuma, AZ - 85365.

8.130 s Sep 15 2017 MITek Industries, Inc. Wed Oct 18 17:56:26 2017 Page 1
ID_Z_OFgkz7ob1GlpCh_t4UWJzDd8l-Uw7Z5aHrGg_QUi7NAAzioH0cad5UyXsSZ7JP8cyS63Z



LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.50	in (loc) l/defl L/d	MT20	197/144
TCDL 10.0	Lumber DOL 1.00	BC 0.68	Vert(LL) -0.13 8 >999 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.55	Vert(TL) -0.28 8-9 >850 240		
BCDL 10.0	Code IRC2012/TP12007	Matrix-S	Horz(TL) 0.04 6 n/a n/a		
				Weight: 94 lb	FT = 20%

LUMBER-	BRACING-	
TOP CHORD 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 3-2-10 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except 7-5-15 oc bracing: 9-10.
WEBS 2x4 SPF No.2 "Except" 10-11,6-12: 2x6 SPF 1650F 1.5E		MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

REACTIONS.	(lb/size) 10=933/0-5-8, 6=830/0-5-8
	Max Horz 10=300(LC 9)
	Max Uplift 10=12(LC 8), 6=60(LC 12)
FORCES.	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	1-10=-892/33, 1-2=-1946/158, 2-3=-2320/243, 3-4=-1580/301, 4-5=-1583/305, 5-6=-794/135
BOT CHORD	9-10=-625/675, 8-9=-274/1942, 7-8=-135/2315
WEBS	2-9=-596/74, 4-7=-328/112, 1-9=-108/1960, 2-8=-305/410, 3-7=-792/102, 5-7=-212/1669

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 19-9-4 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - 200.0lb AC unit load placed on the top chord, 5-0-0 from left end, supported at two points, 5-0-0 apart.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 10 and 60 lb uplift at joint 6.



Expires: 3-31-2019
October 19, 2017

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.</p> <p>Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.</p>	<p>7777 Greenback Lane Suite 109 Citrus Heights, CA 95610</p>
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Job 00416-17Y	Truss T3	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R52054351
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Foxworth Galbraith Lbr Co. Yuma, AZ - 85365,

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Oct 18 17:56:28 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8i-Qi7JWFivoHE8kzHIHb?mt5xiRmiQQw1JoWCUyS63X

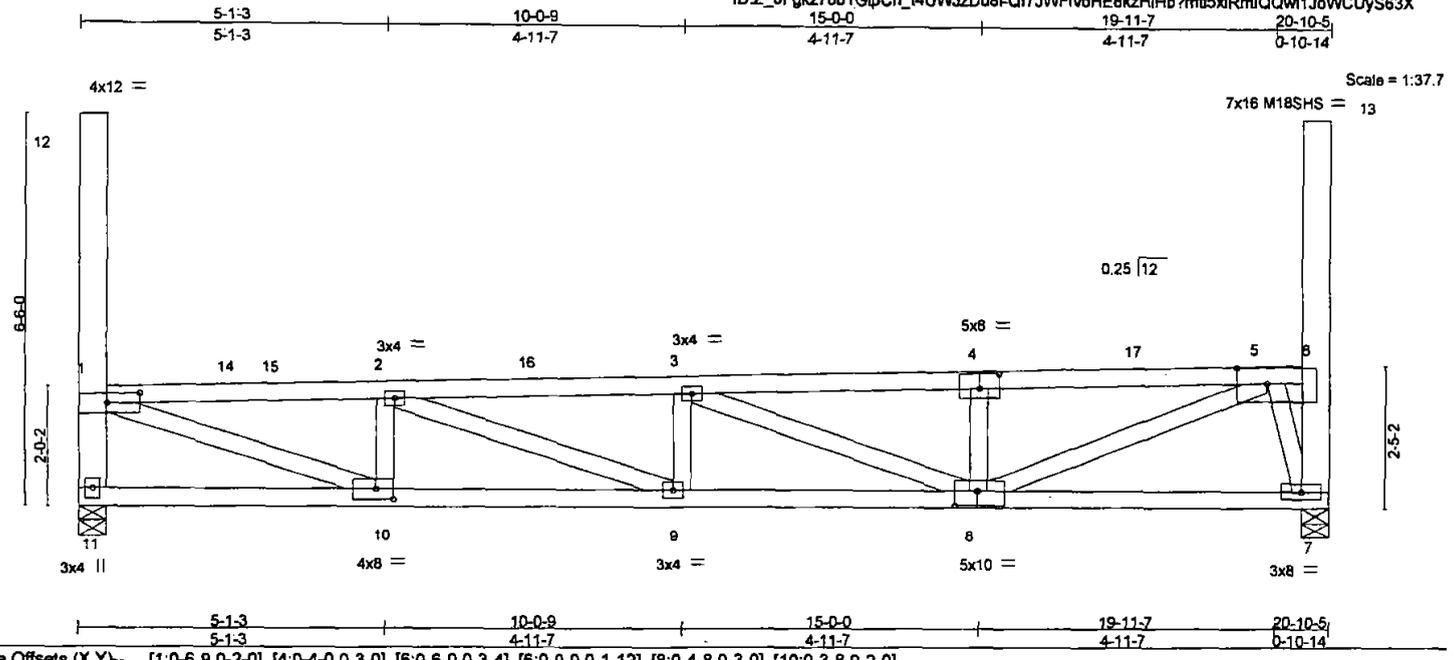


Plate Offsets (X, Y) - [1:0-6-9,0-2-0], [4:0-4-0,0-3-0], [6:0-6-0,0-3-4], [6:0-0-0,0-1-12], [8:0-4-8,0-3-0], [10:0-3-8,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL.	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.53	in (loc) l/def L/d	MT20	197/144
TCDL 10.0	Lumber DOL 1.00	BC 0.72	Vert(LL) -0.15 9 >999 360	M18SHS	197/144
BCLL 0.0 *	Rep Stress Incr YES	WB 0.58	Vert(TL) -0.31 9-10 >797 240		
BCDL 10.0	Code IRC2012/TP12007	Matrix-S	Horz(TL) 0.05 7 n/a n/a		
				Weight: 99 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-9 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 7-6-3 oc bracing: 10-11.
WEBS 2x4 SPF No.2 *Except* 11-12,7-13: 2x6 SPF 1650F 1.5E	

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 11=969/0-5-8, 7=863/0-5-8
Max Horz 11=300(LC 9)
Max Uplift 11=13(LC 8), 7=67(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-11=928/32, 1-2=2043/164, 2-3=2505/258, 3-4=1844/316, 4-5=1847/319,
5-6=337/302
BOT CHORD 10-11=621/671, 9-10=283/2038, 8-9=156/2500, 7-8=130/402
WEBS 2-10=629/74, 4-8=314/103, 1-10=107/2061, 2-9=300/491, 3-8=707/91,
5-8=198/1578, 5-7=987/182

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 20-7-9 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - 3) 200.0lb AC unit load placed on the top chord, 5-0-0 from left end, supported at two points, 5-0-0 apart.
 - 4) Provide adequate drainage to prevent water ponding.
 - 5) All plates are MT20 plates unless otherwise indicated.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 11 and 67 lb uplift at joint 7.



Expires: 3-31-2019
October 19, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.
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7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss T4	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R52054352
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Foxworth Galbraith Lbr Co, Yuma, AZ - 85365.

8.130 s Sep 15 2017 MITek Industries, Inc. Wed Oct 18 17:56:28 2017 Page 1
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Job Reference (optional)

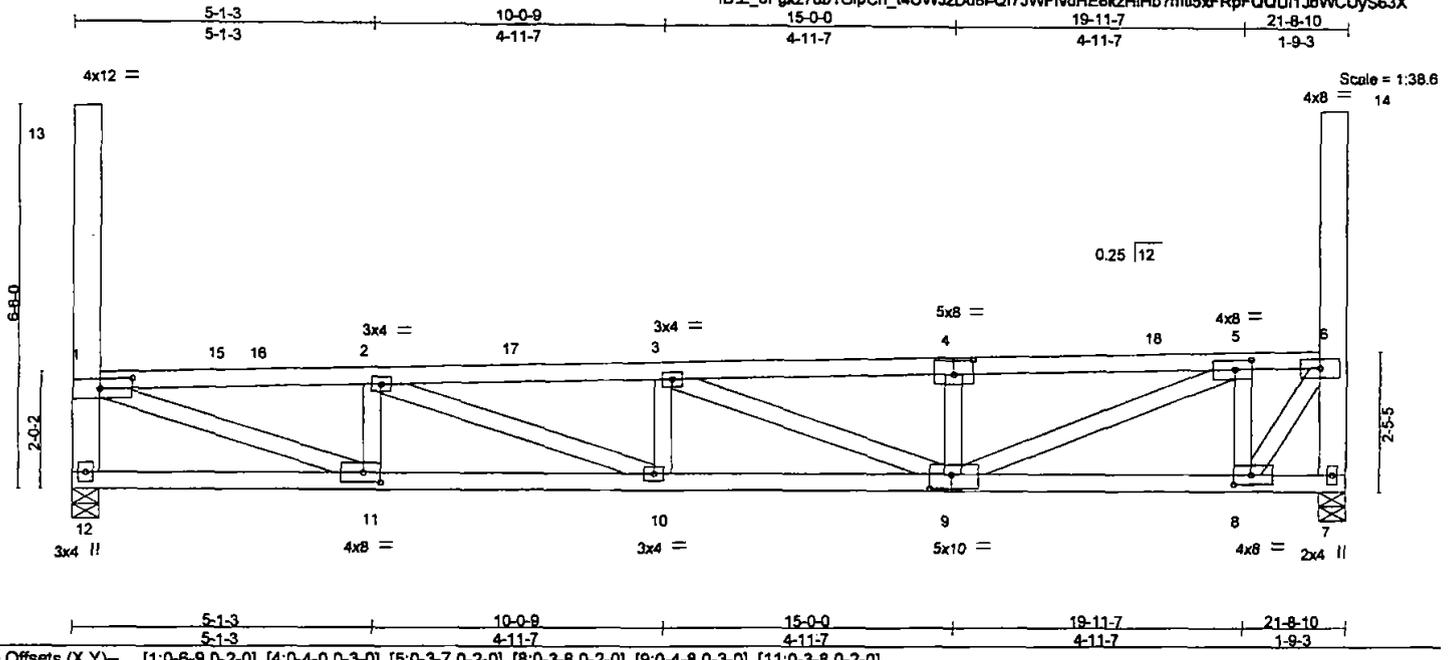


Plate Offsets (X,Y) - [1:0-6-9,0-2-0], [4:0-4-0,0-3-0], [5:0-3-7,0-2-0], [8:0-3-6,0-2-0], [9:0-4-8,0-3-0], [11:0-3-6,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL.	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.56	in (loc) l/def L/d	MT20	197/144
TCDL 10.0	Lumber DOL 1.00	BC 0.53	Vert(LL) -0.17 10 >999 360		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.61	Vert(TL) -0.34 10 >751 240		
BCDL 10.0	Code IRC2012/TPI2007	Matrix-S	Horz(TL) 0.05 7 n/a n/a		
				Weight: 103 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF No.2 "Except"
12-13,7-14: 2x6 SPF 1650F 1.5E

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-10-12 oc putrins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-2-5 oc bracing.
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 12=1006/0-5-8, 7=895/0-5-8
Max Horz 12=299(LC 9)
Max Uplift 12=13(LC 8), 7=74(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-12=963/33, 1-2=2139/172, 2-3=2690/272, 3-4=2105/340, 4-5=2108/344,
5-6=721/316, 6-7=893/133
BOT CHORD 11-12=618/668, 10-11=292/2134, 9-10=177/2684, 8-9=135/670
WEBS 2-11=664/75, 4-9=325/106, 5-8=868/136, 1-11=110/2160, 2-10=293/586,
3-9=825/78, 5-9=199/1559, 6-8=113/1136

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10: Vult=115mph (3-second gust) V(IRC2012)=91mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 21-5-14 zone; end vertical (left and right exposed); C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - 200.0lb AC unit load placed on the top chord, 5-0-0 from left end, supported at two points, 5-0-0 apart.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 12 and 74 lb uplift at joint 7.



Expires: 3-31-2019
October 19, 2017

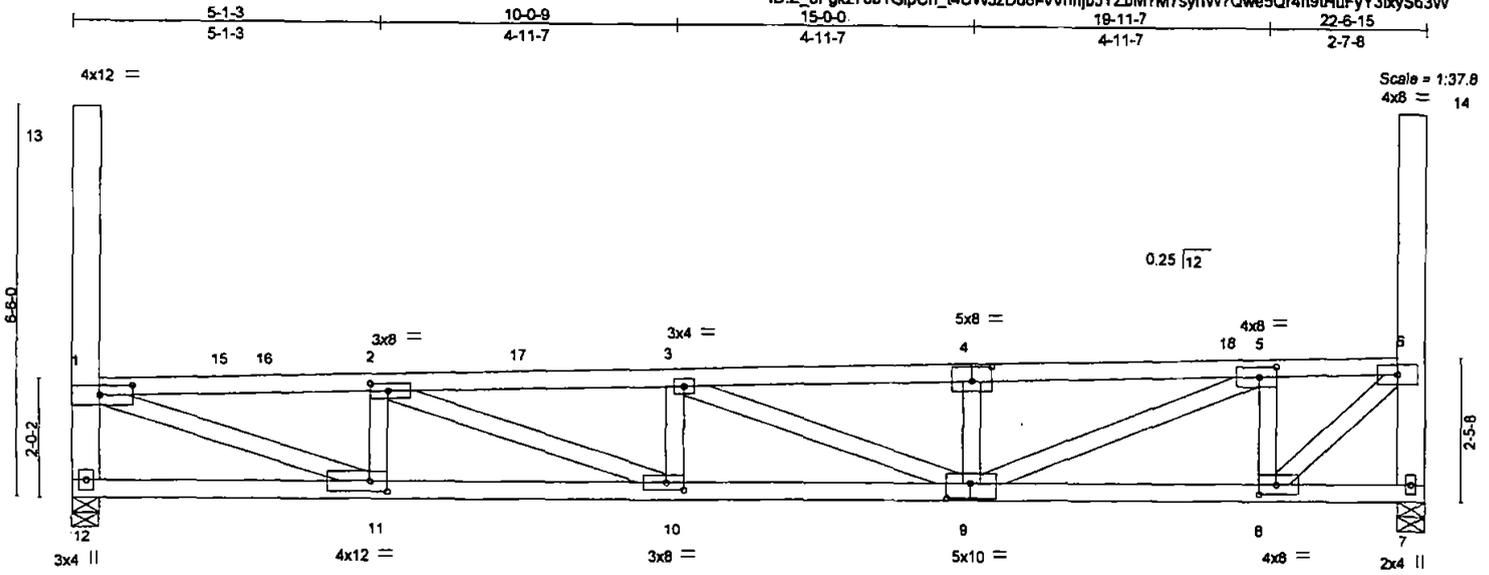
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
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7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss T5	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R52054353
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Foxworth Galbraith Lbr Co, Yuma, AZ - 85365,

8,130 s Sep 15 2017 MITek Industries, Inc. Wed Oct 18 17:56:29 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UvWzDd8f-vVhhjbjYzBM7M7syfW7Qwe5Qr4n9tHuFyY3byS63W



5-1-3	10-0-9	15-0-0	19-11-7	22-6-15
5-1-3	4-11-7	4-11-7	4-11-7	2-7-8

Plate Offsets (X,Y) -		[1:0-6-9,0-2-0], [2:0-3-8,0-1-8], [4:0-4-0,0-3-0], [5:0-3-7,0-2-0], [8:0-3-8,0-2-0], [9:0-4-12,0-3-0], [10:0-3-8,0-1-8], [11:0-3-8,0-2-0]							
LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00		TC 0.59	Vert(LL) -0.19	9-10	>999	360	MT20	197/144
TCDL 10.0	Lumber DOL 1.00		BC 0.83	Vert(TL) -0.39	9-10	>681	240		
BCLL 0.0 *	Rep Stress Incr YES		WB 0.64	Horz(TL) 0.06	7	n/a	n/a		
BCDL 10.0	Code IRC2012/TP12007		Matrix-S					Weight: 106 lb	FT = 20%

LUMBER-	BRACING-	
TOP CHORD 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 2-8-12 oc purtins, except end verticals.
BOT CHORD 2x4 SPF No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing. Except: 7-6-12 oc bracing: 11-12.
WEBS 2x4 SPF No.2 "Except" 12-13,7-14: 2x6 SPF 1650F 1.5E		MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 12=1042/0-5-8, 7=928/0-5-8
Max Horz 12=299(LC 9)
Max Uplift 12=15(LC 8), 7=81(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-12=999/34, 1-2=2235/180, 2-3=2873/287, 3-4=2370/362, 4-5=2372/366, 5-6=1002/301, 6-7=915/133

BOT CHORD 11-12=614/664, 10-11=300/2230, 9-10=196/2867, 8-9=163/999

WEBS 2-11=697/76, 4-9=327/106, 5-8=838/122, 1-11=113/2262, 2-10=289/678, 3-9=539/68, 5-9=188/1489, 6-8=138/1332

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 22-4-3 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - 3) 200.0lb AC unit load placed on the top chord, 5-0-0 from left end, supported at two points, 5-0-0 apart.
 - 4) Provide adequate drainage to prevent water ponding.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 12 and 81 lb uplift at joint 7.



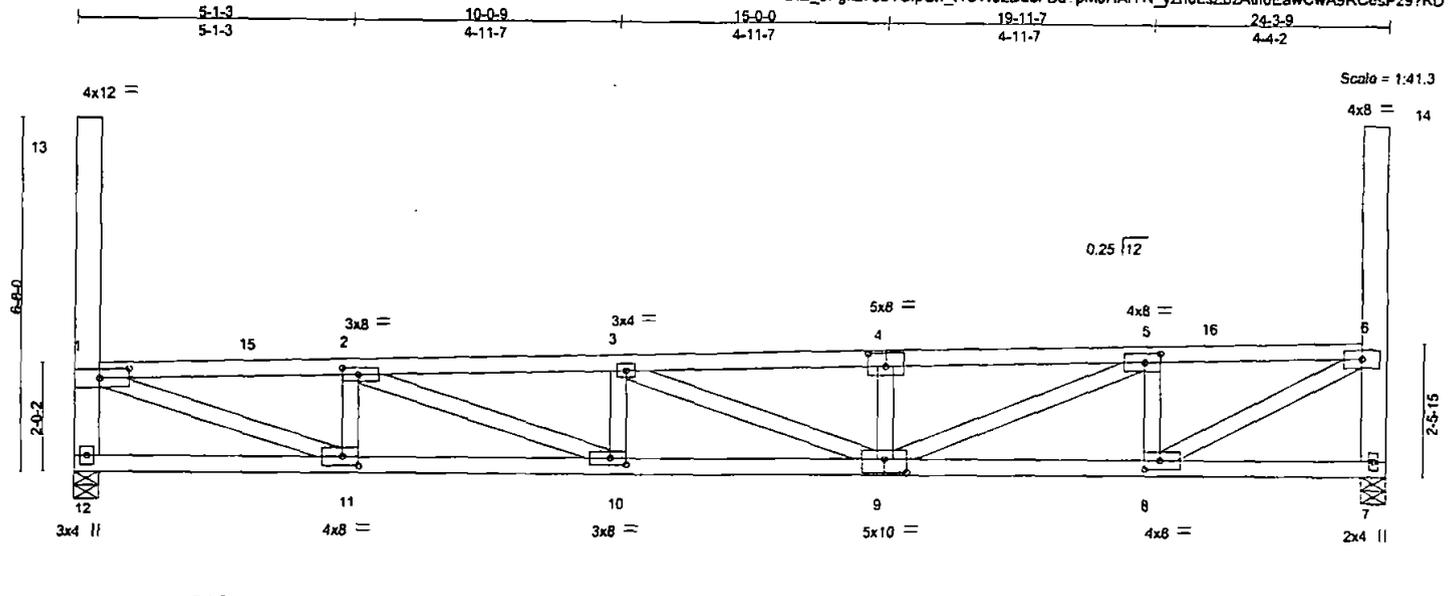
Expires: 3-31-2019
October 19, 2017

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE</p> <p>Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPH Quality Criteria, DSB-69 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.</p>	 <p>7777 Greenback Lane Suite 108 Citrus Heights, CA 95610</p>
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Job 00416-17Y	Truss T7	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761220
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Forworth Galbraith Lumber Co. Yuma, Az 85365

Job Reference (optional)
7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 08 10:48:32 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_4UJWJzDd8i-Bd?pmJHAIYN_yzn0LsZbzAtIf0EawCwA9RCesPz9?KD



5-1-3	10-0-9	15-0-0	19-11-7	24-3-9
5-1-3	4-11-7	4-11-7	4-11-7	4-4-2

LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL	40.0	Plate Grip DOL	1.00	TC	0.40	in (loc)	l/defl	L/d	MT20	197/144	
TCDL	10.0	Lumber DOL	1.00	BC	0.85	Vert(LL)	-0.25	9-10	>999	360	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.61	Vert(TL)	-0.44	9-10	>644	240	
BCDL	10.0	Code IRC2012/TPI2007		(Matrix)		Horz(TL)	0.06	7	n/a	n/a	
										Weight: 111 lb FT = 20%	

LUMBER-
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF No.2 *Except*
12-13,7-14: 2x6 SPF 1650F 1.5E

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-0-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-4-13 oc bracing.

REACTIONS. (lb/size) 12=954/0-5-8, 7=954/0-5-8
Max Horz 12=299(LC 9)
Max Uplift 12=177(LC 8), 7=134(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-12=-913/195, 1-15=-2103/517, 2-15=-2099/517, 2-3=-2932/624, 3-4=-2714/584.
4-5=-2717/587, 5-16=-1590/423, 6-16=-1587/423, 6-7=-923/182
BOT CHORD 11-12=-629/635, 10-11=-638/2100, 9-10=-540/2928, 8-9=-302/1587
WEBS 2-11=-655/187, 4-9=-332/95, 5-8=-773/171, 1-11=-438/2144, 2-10=-266/882.
5-9=-271/1225, 6-8=-287/1758

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 24-0-13 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 177 lb uplift at joint 12 and 134 lb uplift at joint 7.
 - *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



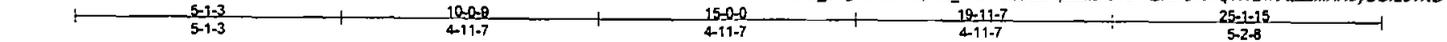
7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss T8	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761221
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

Job Reference (optional)
7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:33 2017 Page 1

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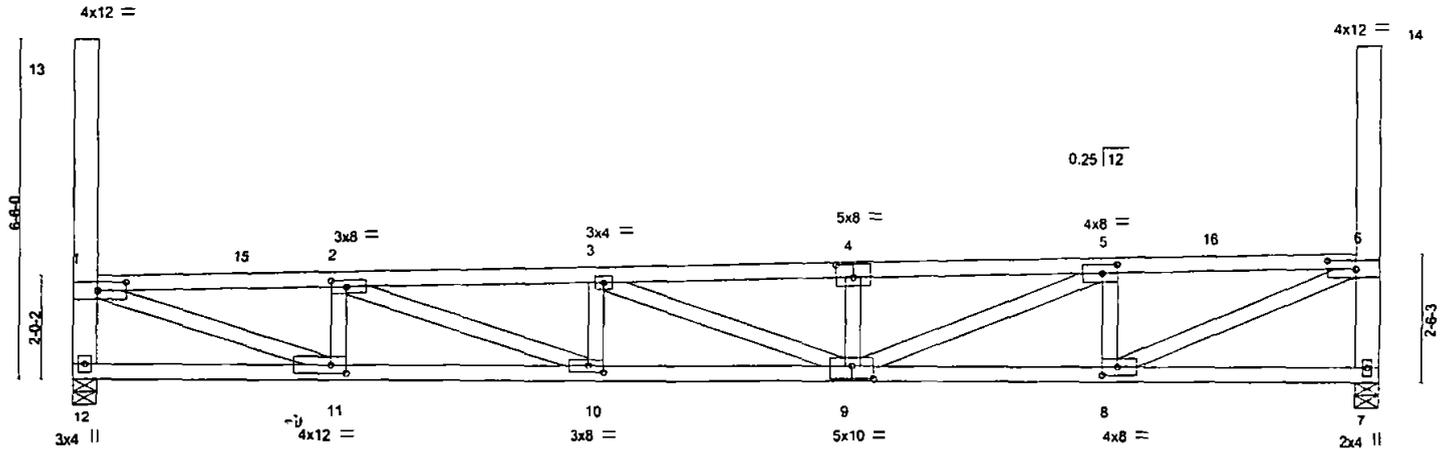


Plate Offsets (X, Y)	[1:0-6-9,0-2-0], [2:0-3-8,0-1-8], [4:0-4-0,0-3-0], [5:0-3-7,0-2-0], [6:0-6-9,0-2-0], [8:0-3-8,0-2-0], [9:0-5-0,0-3-0], [10:0-3-8,0-1-8], [11:0-3-8,0-2-0]
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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.44	in (loc) l/defl L/d	MT20	197/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.90	Vert(LL) -0.29 9-10 >999 360		
BCLL 0.0 *	Lumber DOL 1.00	WB 0.63	Vert(TL) -0.51 9-10 >585 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.07 7 n/a n/a		
	Code IRC2012/TPI2007			Weight: 114 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 2-11-5 oc purlins, except end verticals.
BOT CHORD 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 7-4-0 oc bracing.
WEBS 2x4 SPF No.2 *Except 12-13,7-14: 2x6 SPF 1650F 1.5E	

REACTIONS. (lb/size) 12=988/0-5-8, 7=988/0-5-8
 Max Horz 12=299(LC 9)
 Max Uplift 12=181(LC 8), 7=139(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-12=-947/198, 1-15=-2195/529, 2-15=-2191/530, 2-3=3108/648, 3-4=-2962/618, 4-5=-2965/622, 5-16=-1920/469, 6-16=-1916/469, 6-7=-950/185
 BOT CHORD 11-12=-626/632, 10-11=-649/2191, 9-10=-566/3104, 8-9=-348/1916
 WEBS 2-11=-688/190, 3-10=-268/132, 4-9=-325/93, 5-8=-769/175, 1-11=-447/2240, 2-10=-268/972, 5-9=-257/1137, 6-8=-328/2037

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BC DL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 24-11-3 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 12 and 139 lb uplift at joint 7.
 - *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI Quality Criteria, DSB-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss T9	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761222
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Foxworth Galbraith Lumber Co, Yuma, Az 85365 Job Reference (optional) 7,640 a Apr 19 2016 MiTek Industries, Inc. Tue Jun 08 10:48:33 2017 Page 1
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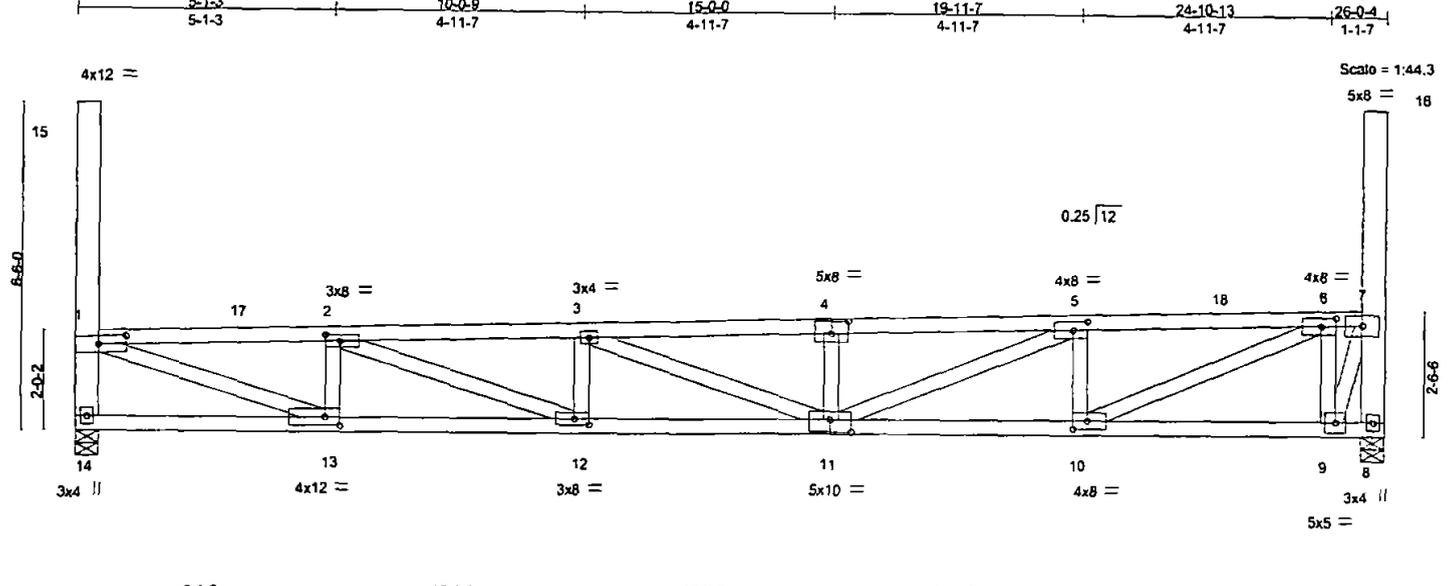


Plate Offsets (X,Y)	[1:0-6-9,0-2-0], [2:0-3-8,0-1-8], [4:0-4-0,0-3-0], [5:0-3-7,0-2-0], [6:0-3-7,0-2-0], [10:0-3-8,0-2-0], [11:0-5-0,0-3-0], [12:0-3-8,0-1-8], [13:0-3-8,0-2-0]
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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.48	in (loc) l/defl L/d	MT20	197/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.95	Vert(LL) -0.32 11-12 >963 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.66	Vert(TL) -0.56 11-12 >543 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.08 8 n/a n/a		
	Code IRC2012/TPI2007			Weight: 121 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SPF No.2
 BOT CHORD 2x4 SPF No.2
 WEBS 2x4 SPF No.2 *Except
 14-15,8-16: 2x6 SPF 1650F 1.5E

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 2-9-13 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS. (lb/size) 14=1023/0-5-8, 8=1023/0-5-8
 Max Horz 14=299(LC 9)
 Max Uplift 14=184(LC 8), 8=143(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-14=-981/201, 1-17=-2286/542, 2-17=-2282/543, 2-3=-3283/672, 3-4=-3213/653,
 4-5=-3216/656, 5-18=-2233/511, 6-18=-2229/512, 6-7=-554/335, 7-8=-1001/183
 BOT CHORD 13-14=-623/628, 12-13=-661/2282, 11-12=-591/3278, 10-11=-392/2229, 9-10=-135/457
 WEBS 2-13=-720/193, 3-12=-299/134, 4-11=-328/95, 5-10=-734/172, 6-9=-1032/248,
 1-13=-456/2335, 2-12=-276/1061, 5-11=-244/1068, 6-10=-318/1943, 7-9=-197/1139

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10: Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 25-9-8 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 184 lb uplift at joint 14 and 143 lb uplift at joint 8.
 - *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek
 7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss T10	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761223
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Foxworth Galbraith Lumber Co, Yuma, Az 85365
 7,640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:22 2017 Page 1
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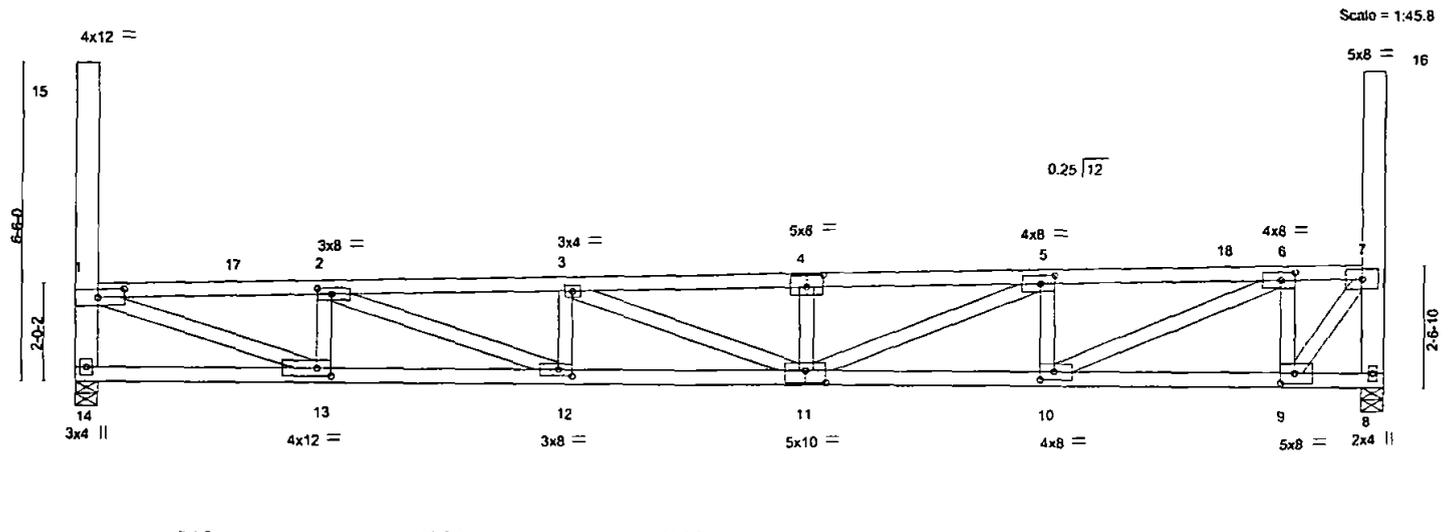


Plate Offsets (X, Y) -	[1:0-6-9,0-2-0], [2:0-3-8,0-1-8], [4:0-4-0,0-3-0], [5:0-3-7,0-2-0], [6:0-3-7,0-2-0], [9:0-3-8,0-2-8], [10:0-3-8,0-2-0], [11:0-5-0,0-3-0], [12:0-3-8,0-1-8], [13:0-3-8,0-2-0]
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LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.53	Vert(LL)	-0.35 11-12	>914	360	MT20	197/144
TCDL 10.0	Lumber DOL	1.00	BC 0.68	Vert(TL)	-0.61 11-12	>517	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.69	Horz(TL)	0.08 8	n/a	n/a		
BCDL 10.0	Code IRC2012/TPI2007		(Matrix)					Weight: 123 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SPF No.2
 BOT CHORD 2x4 SPF 1650F 1.5E
 WEBS 2x4 SPF No.2 "Except"
 14-15,8-16: 2x6 SPF 1650F 1.5E

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 2-7-11 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 8-9-10 oc bracing.

REACTIONS. (lb/size) 14=1057/0-5-8, 8=1057/0-5-8
 Max Horz 14=299(LC 9)
 Max Uplift 14=187(LC 8), 8=148(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-14=1014/205, 1-17=2377/555, 2-17=2372/555, 2-3=3457/696, 3-4=3460/687,
 4-5=3463/691, 5-18=2551/555, 6-18=2546/556, 6-7=837/347, 7-8=1050/180
 BOT CHORD 13-14=620/626, 12-13=672/2373, 11-12=615/3453, 10-11=436/2547, 9-10=188/835
 WEBS 2-13=752/196, 3-12=329/137, 4-11=327/95, 5-10=707/168, 6-9=1010/205,
 1-13=466/2429, 2-12=284/1151, 5-11=233/992, 6-10=305/1877, 7-9=206/1345

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 26-7-13 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 14 and 148 lb uplift at joint 8.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
 June 7, 2017

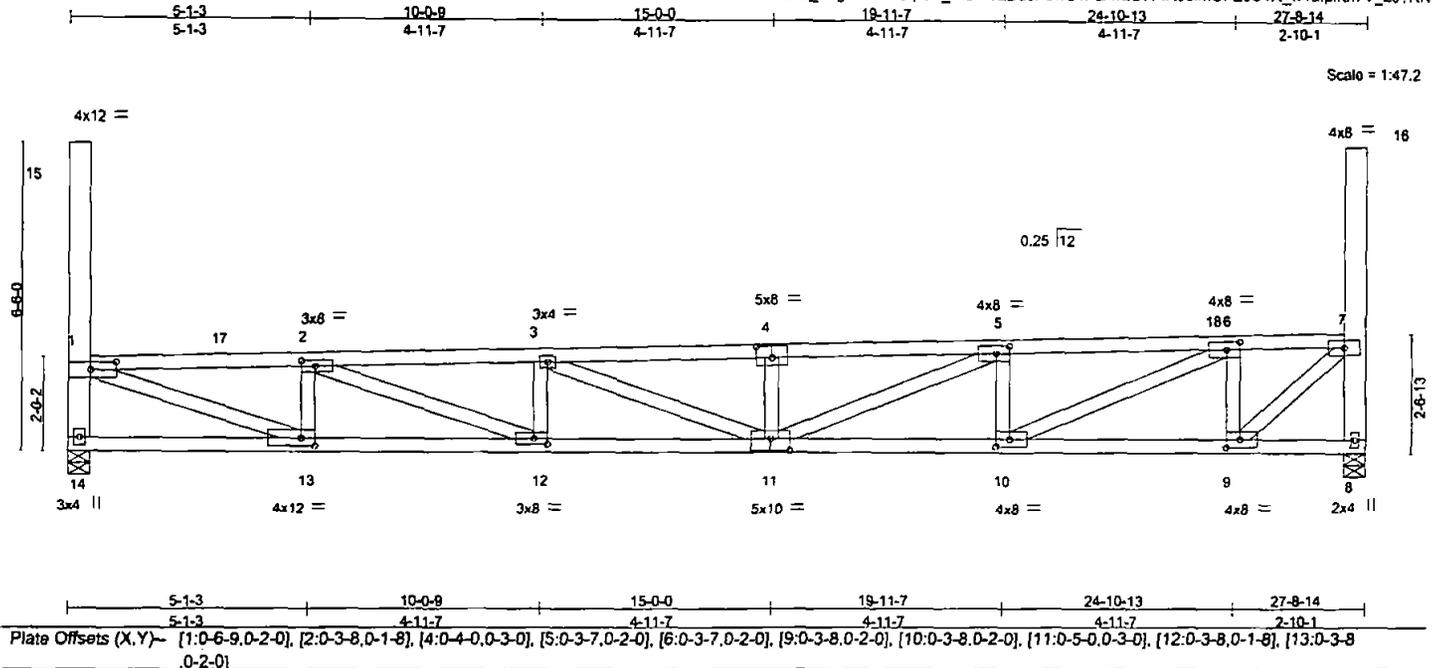
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek
 7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss T11	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbot	R50761224
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Foxworth Galbraith Lumber Co. Yuma, Az 85385

7.640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:22 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8l-UhO1FuAv2U7PIR05ImOFZ3S1R_w1afpirtn7V_zs9?KN



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1-4-0 1.00	TC 0.39	in (loc) l/defl L/d	MT20	197/144
TCDL 10.0	Lumber DOL 1.00	BC 0.48	Vert(LL) -0.35 11-12 >925 360		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.71	Vert(TL) -0.62 11-12 >525 240		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)	Horz(TL) 0.08 8 n/a n/a		
				Weight: 126 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SPF 1650F 1.5E
BOT CHORD 2x4 SPF 2100F 1.8E
WEBS 2x4 SPF No.2 *Except*
14-15,8-16: 2x6 SPF 1650F 1.5E

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-4-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-6-9 oc bracing.

REACTIONS. (lb/size) 14=1091/0-5-8, 8=1091/0-5-8
Max Horz 14=299(LC 9)
Max Uplift 14=191(LC 8), 8=153(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-14=-1046/208, 1-17=-2465/567, 2-17=-2461/567, 2-3=-3632/721, 3-4=-3706/721,
4-5=-3709/725, 5-18=-2869/600, 6-18=-2862/600, 6-7=-1213/362, 7-8=-1074/183
BOT CHORD 13-14=-618/623, 12-13=-682/2462, 11-12=-639/3628, 10-11=-481/2865, 9-10=-241/1211
WEBS 2-13=-784/199, 3-12=-359/140, 4-11=-325/94, 5-10=-679/163, 6-9=-989/194,
1-13=-475/2518, 2-12=-294/1242, 5-11=-221/913, 6-10=-293/1814, 7-9=-235/1585

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCCL=6.0psf; BCOL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 27-6-2 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 14 and 153 lb uplift at joint 8.
 - 7) *Semi-rigid pitchbreaks including heels* Member and fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPH1 Quality Criteria, DSB-69 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job 00416-17Y	Truss T12	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761225
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

7.640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:23 2017 Page 1
ID:Z_OFgkz7ob1GlpCh_t4UWJzDd8-yuyPTEAXpnFGMbbHJTvU5G78809DJ6cs4XXg1Qz97KM



Scale = 1:48.7

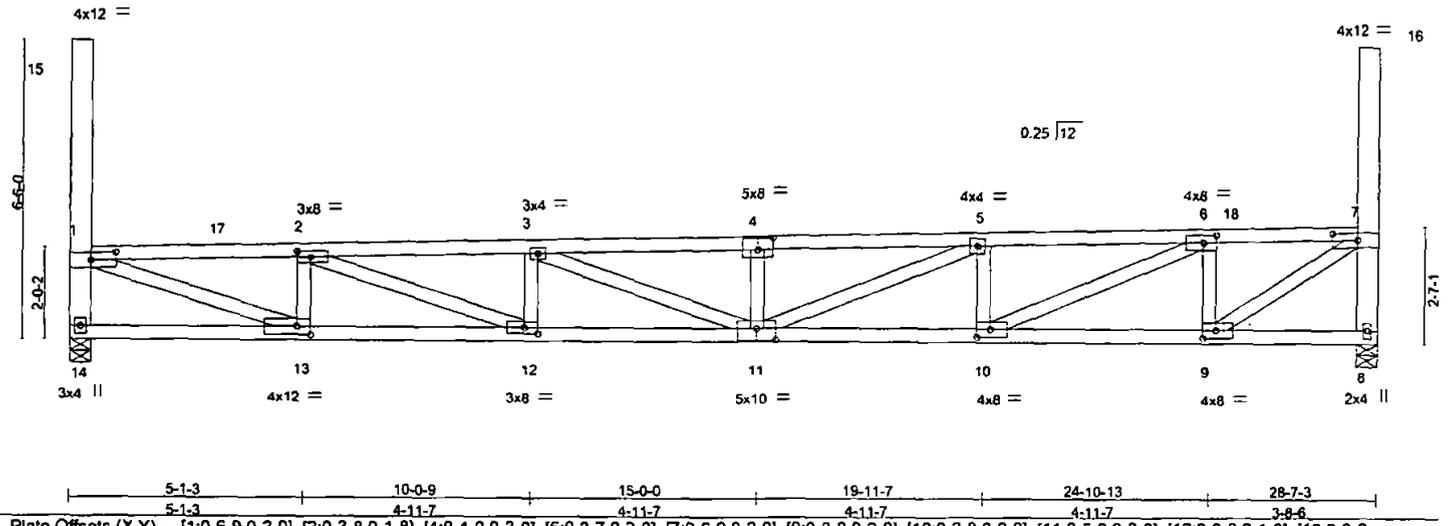


Plate Offsets (X, Y) - [1:0-6-9,0-2-0], [2:0-3-8,0-1-8], [4:0-4-0,0-3-0], [6:0-3-7,0-2-0], [7:0-6-9,0-2-0], [9:0-3-8,0-2-0], [10:0-3-8,0-2-0], [11:0-5-0,0-3-0], [12:0-3-8,0-1-8], [13:0-3-8-0-2-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.65	in (loc) l/defl L/d	MT20	197/144
TGDL 10.0	Plate Grip DOL 1.00	BC 0.93	Vert(LL) -0.43 11-12 >785 360		
BCLL 0.0 *	Lumber DOL 1.00	WB 0.74	Vert(TL) -0.76 11-12 >446 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.10 8 n/a n/a		
	Code IRC2012/TPI2007			Weight: 129 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF 1650F 1.5E "Except"
8-11: 2x4 SPF No.2
WEBS 2x4 SPF No.2 "Except"
14-15,8-16: 2x6 SPF 1650F 1.5E

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-3-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS. (lb/size) 14=1126/0-5-8, 8=1126/0-5-8
Max Horz 14=298(LC 9)
Max Uplift 14=195(LC 8), 8=-158(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-14=-1082/212, 1-17=-2559/580, 2-17=-2555/580, 2-3=-3807/745, 3-4=-3959/756,
4-5=-3962/760, 5-6=-3186/644, 6-18=-1592/414, 7-18=-1592/414, 7-8=-1100/185
BOT CHORD 13-14=-614/620, 12-13=-693/2555, 11-12=-662/3802, 10-11=-526/3182, 9-10=-294/1589
WEBS 2-13=-816/203, 3-12=-390/143, 4-11=-327/95, 5-10=-650/158, 6-9=-980/192,
1-13=-485/2620, 2-12=-303/1329, 5-11=-211/843, 6-10=-283/1747, 7-9=-275/1874

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C: enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 28-4-7 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 14 and 158 lb uplift at joint 8.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPM Quality Criteria, DSB-89 and BCSI Building Component Safety information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss T13	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761226
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Foxworth Galbraith Lumber Co, Yuma, Az 85385
 7,640 a Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:24 2017 Page 1
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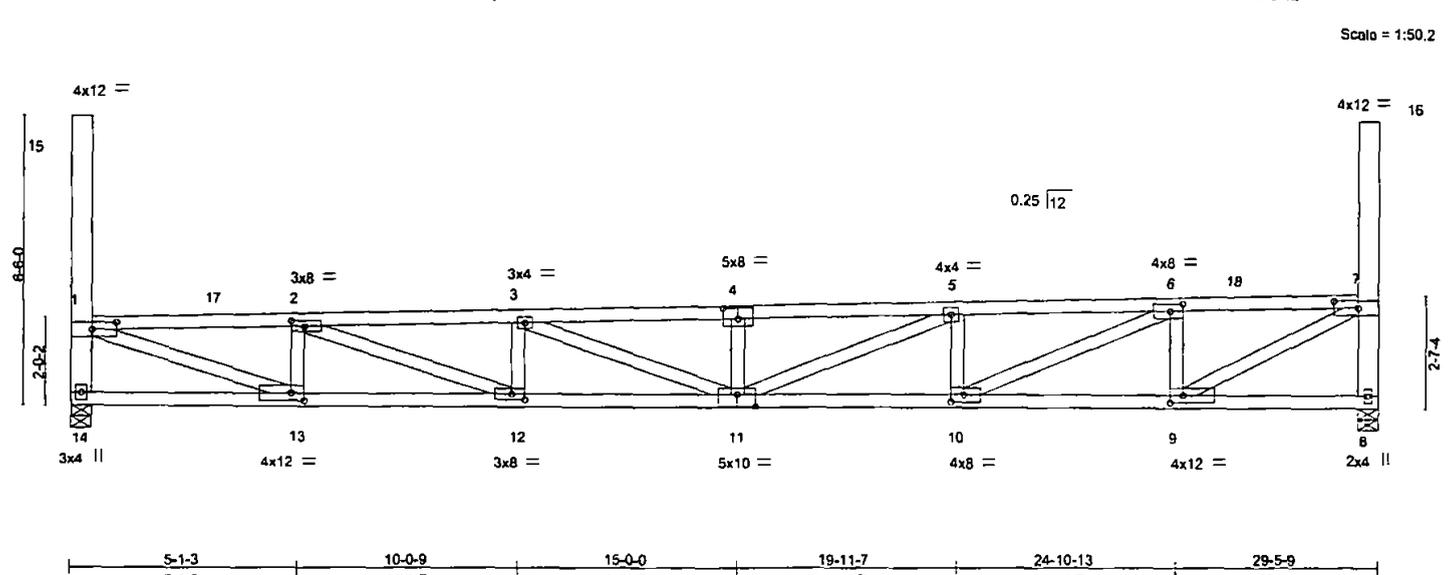


Plate Offsets (X,Y) - [1:0-6-9,0-2-0], [2:0-3-8,0-1-8], [4:0-4-0,0-3-0], [6:0-3-7,0-2-0], [7:0-6-9,0-2-0], [9:0-3-8,0-2-0], [10:0-3-8,0-2-0], [11:0-5-0,0-3-4], [12:0-3-8,0-1-8], [13:0-3-8,0-2-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.70	in (loc) l/dofl L/d	MT20	197/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.78	Vert(LL) -0.48 11-12 >730 360		
BCLL 0.0 *	Lumber DOL 1.00	WB 0.77	Vert(TL) -0.84 11-12 >416 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.11 8 n/a n/a		
	Code IRC2012/TPI2007			Weight: 131 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SPF No.2
 BOT CHORD 2x4 SPF 1650F 1.5E
 WEBS 2x4 SPF No.2 *Except*
 14-15,8-16: 2x6 SPF 1650F 1.5E

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 8-7-3 oc bracing.

REACTIONS. (lb/size) 14=1160/0-5-8, 8=1160/0-5-8
 Max Horz 14=298(LC 9)
 Max Uplift 14=198(LC 8), 8=163(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-14=-1116/215, 1-17=-2650/593, 2-17=-2646/593, 2-3=-3983/769, 3-4=-4209/791,
 4-5=-4212/795, 5-6=-3504/688, 6-18=-1978/467, 7-18=-1975/468, 7-8=-1126/189
 BOT CHORD 13-14=-612/617, 12-13=703/2647, 11-12=-685/3978, 10-11=-570/3500, 9-10=-348/1975
 WEBS 2-13=-848/206, 3-12=-421/147, 4-11=-328/95, 5-10=-619/152, 6-9=-975/193,
 1-13=-496/2715, 2-12=-312/1418, 3-11=-156/258, 5-11=-200/770, 6-10=-272/1673,
 7-9=-319/2191

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 29-2-13 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 198 lb uplift at joint 14 and 163 lb uplift at joint 8.
 - *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.



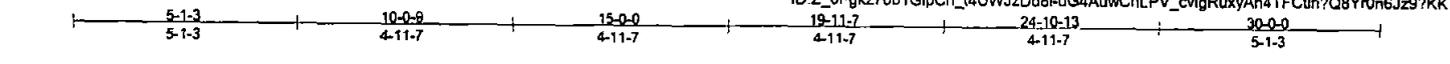
Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

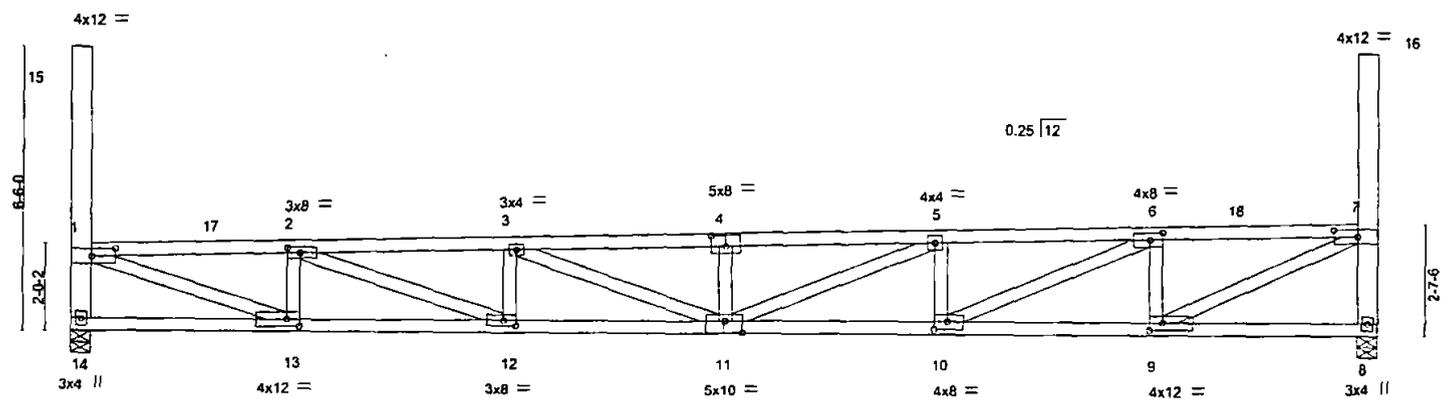
MITEK
 7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss T14	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761227
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Foxworth Galbraith Lumber Co., Yuma, Az 85385
 Job Reference (optional)
 7.640 s Apr 19 2016 MITek Industries, Inc. Tue Jun 06 10:48:25 2017 Page 1
 ID-Z_OFgkz7ob1GlpCh_t4UWJzDd8l-uG4AuwCnLPV_cvlgruXyAh4TFClm?Q8Yr0n6Jz97Kk



Scale = 1:51.2



5-1-3	10-0-8	15-0-0	19-11-7	24-10-13	30-0-0
5-1-3	4-11-7	4-11-7	4-11-7	4-11-7	5-1-3

Plate Offsets (X,Y)- [1:0-6-9,0-2-0], [2:0-3-8,0-1-8], [4:0-4-0,0-3-0], [6:0-3-7,0-2-0], [7:0-6-9,0-2-0], [9:0-3-8,0-2-0], [10:0-3-8,0-2-0], [11:0-5-0,0-3-4], [12:0-3-8,0-1-8], [13:0-3-8,0-2-0]

LOADING (psf)	SPACING-	CSI.	DEFL	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.73	in (loc) l/defl L/d	MT20	197/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.80	Vert(LL) -0.51 11 >695 360		
BCLL 0.0 *	Lumber DOL 1.00	WB 0.78	Vert(TL) -0.89 11-12 >397 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.11 8 n/a n/a		
	Code IRC2012/TPI2007			Weight: 133 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SPF No.2
 BOT CHORD 2x4 SPF 1650F 1.5E
 WEBS 2x4 SPF No.2 *Except*
 14-15,8-16: 2x6 SPF 1650F 1.5E

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

REACTIONS. (lb/size) 14=1182/0-5-8, 8=1182/0-5-8
 Max Horz 14=298(LC 11)
 Max Uplift 14=201(LC 8), 8=166(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-14=-1137/218, 1-17=-2707/600, 2-17=-2703/601, 2-3=-4092/784, 3-4=-4365/813, 4-5=-4368/816, 5-6=-3701/715, 6-18=-2220/501, 7-18=-2217/501, 7-8=-1143/192
 BOT CHORD 13-14=-610/615, 12-13=-710/2703, 11-12=-698/4087, 10-11=-598/3697, 9-10=-381/2216
 WEBS 2-13=-868/208, 3-12=-440/149, 4-11=-329/95, 5-10=-598/149, 6-9=-974/196, 1-13=-502/2775, 2-12=-318/1474, 3-11=-158/295, 5-11=-194/725, 6-10=-265/1623, 7-9=-348/2397

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10: Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCCL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 29-9-4 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 201 lb uplift at joint 14 and 166 lb uplift at joint 8.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
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Job 00416-17Y	Truss T15	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761228
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:25 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8i-uG4AuwCnLPV_cvtgRuxyAh4UeCrhn068Yr0n6Jz9?KK



Scale = 1:48.7

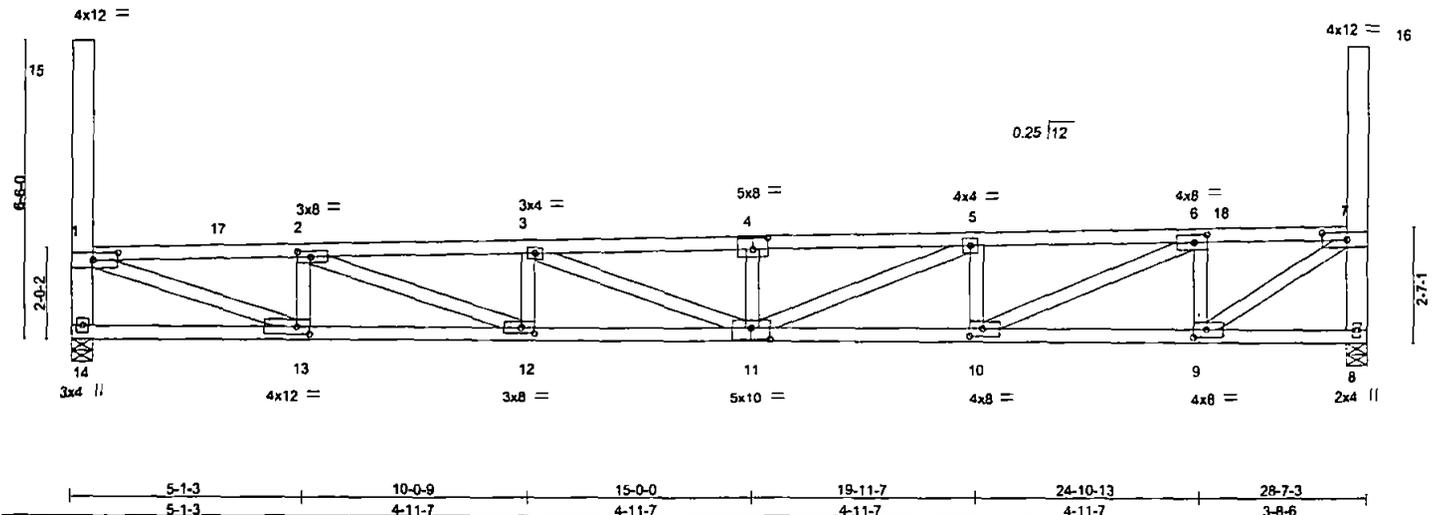


Plate Offsets (X,Y) - [1:0-6-9,0-2-0], [2:0-3-8,0-1-8], [4:0-4-0,0-3-0], [6:0-3-7,0-2-0], [7:0-6-9,0-2-0], [9:0-3-8,0-2-0], [10:0-3-8,0-2-0], [11:0-5-0,0-3-0], [12:0-3-8,0-1-8], [13:0-3-8,0-2-0]

LOADING (psf)	SPACING-	1-4-0	CSI.	DEFL.	in (loc)	l/d	l/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.65	Vert(LL)	-0.43 11-12	>785	360	MT20	197/144
TCDL 10.0	Lumber DOL	1.00	BC 0.93	Vert(TL)	-0.76 11-12	>446	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.74	Horz(TL)	0.10 8	n/a	n/a		
BCDL 10.0	Code IRC2012/TPI2007		(Matrix)					Weight: 129 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SPF No.2
 BOT CHORD 2x4 SPF 1650F 1.5E "Except"
 8-11: 2x4 SPF No.2
 WEBS 2x4 SPF No.2 "Except"
 14-15,8-16: 2x6 SPF 1650F 1.5E

BRACING-

TOP CHORD Structural wood sheathing directly applied or 2-3-5 oc purfins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS.

(lb/size) 14=1126/0-5-8, 8=1126/0-5-8
 Max Horz 14=298(LC 9)
 Max Uplift 14=195(LC 8), 8=158(LC 12)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-14=-1082/212, 1-17=-2559/580, 2-17=-2555/580, 2-3=-3807/745, 3-4=-3959/756,
 4-5=-3962/760, 5-6=-3186/644, 6-18=-1592/414, 7-18=-1592/414, 7-8=-1100/185
 BOT CHORD 13-14=-614/620, 12-13=-693/2555, 11-12=-662/3802, 10-11=-526/3182, 9-10=-294/1589
 WEBS 2-13=-816/203, 3-12=-390/143, 4-11=-327/95, 5-10=-650/158, 6-9=-980/192,
 1-13=-485/2620, 2-12=-303/1329, 5-11=-211/843, 6-10=-283/1747, 7-9=-275/1874

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C: enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 28-4-7 zone; end vertical left and right exposed:C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 14 and 158 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss webs and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, OSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss T16	Truss Type GABLE	Qty 1	Ply 1	Tony Abbott	R50761229
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

7.640 s Apr 19 2018 MiTek Industries, Inc. Tue Jun 06 10:48:26 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8i-NTeY5GDP6idrE2Js_cSBjvdgwbKOWallmViKelz9?KJ

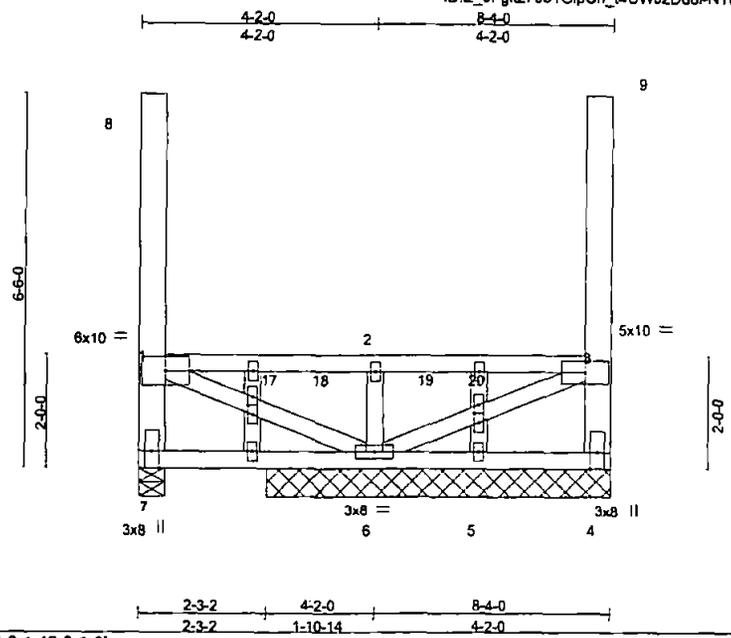


Plate Offsets (X,Y)-- [10:0-1-13,0-1-0], [14:0-1-13,0-1-0]

LOADING (psf)	SPACING-	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	2-0-0	TC 0.61	Vert(LL)	-0.01	6-7	>999	MT20	197/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.26	Vert(TL)	-0.02	6-7	>999		
BCLL 0.0	Lumber DOL 1.00	WB 0.27	Horz(TL)	0.01	6	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix)						
	Code IRC2012/TPI2007						Weight: 53 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SPF No.2
 BOT CHORD 2x4 SPF No.2
 WEBS 2x4 SPF No.2 *Except*
 7-8,4-9: 2x6 SPF 1650F 1.5E
 OTHERS 2x4 SPF No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-6-5 oc bracing.

REACTIONS.

All bearings 6-0-14 except (jt=length) 7=0-5-8.
 (lb) - Max Horz 7=-454(LC 8)
 Max Uplift All uplift 100 lb or less at joint(s) except 7=-258(LC 8), 4=-264(LC 9), 6=-233(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 5 except 7=273(LC 20), 4=259(LC 19), 6=517(LC 1)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-7=-336/465, 3-4=-335/462
 BOT CHORD 6-7=-1148/1208, 5-6=-376/412, 4-5=-376/412
 WEBS 2-6=-406/148, 1-6=-871/826, 3-6=-875/829

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 8-1-4 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- Provide adequate drainage to prevent water ponding.
- All plates are 2x4 MT20 unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 258 lb uplift at joint 7, 264 lb uplift at joint 4 and 233 lb uplift at joint 6.
- *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

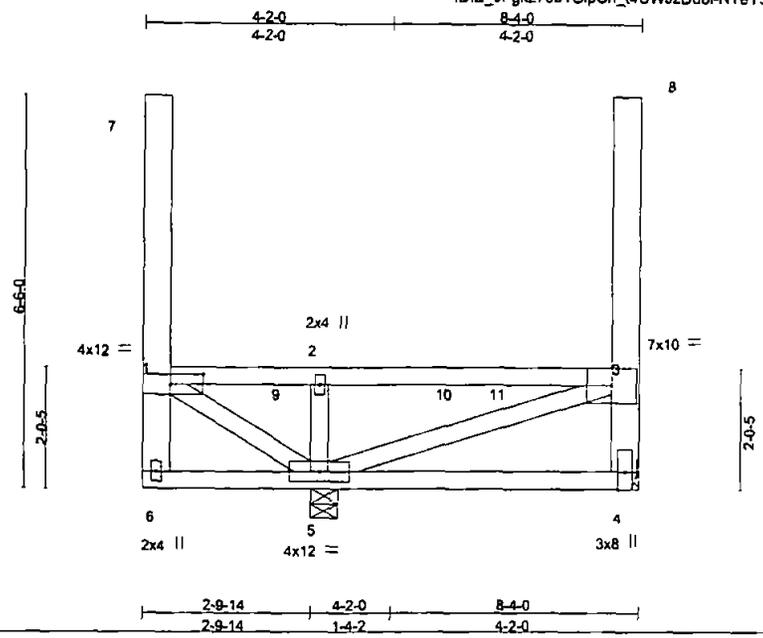


7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job	Truss	Truss Type	Qty	Ply	Tony Abbott	R50761230
00416-17Y	T17	FLAT	1	1		

Foxworth Galbraith Lumber Co, Yuma, Az 85385

7,840 s Apr 19 2018 MITok Industries, Inc. Tue Jun 06 10:48:26 2017 Page 1
 ID:Z_0Fgkz7ob1GlpCh_4UWJzDd8i-NTeY5GDP6idrE2Js_cSBjvdg2bMYWTwlmViketz97KJ



Scale = 1:37.1

Plate Offsets (X,Y) - [1:0-6-8,0-2-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 40.0	Plate Grip DOL	1.00	TC 0.60	Vert(LL)	-0.02	4-5	>999	360	MT20	197/144
TCDL 10.0	Lumber DOL	1.00	BC 0.19	Vert(TL)	-0.06	4-5	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.77	Horz(TL)	0.00	4	n/a	n/a		
BCDL 10.0	Code IRC2012/TPI2007		(Matrix)							
									Weight: 51 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SPF No.2
 BOT CHORD 2x4 SPF No.2
 WEBS 2x4 SPF No.2 *Except*
 6-7,4-8: 2x6 SPF 1650F 1.5E

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-10-4 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

(lb/size) 4=220/Mechanical, 5=725/0-5-8
 Max Horz 5=-453(LC 10)
 Max Uplift 4=-347(LC 9), 5=-417(LC 8)
 Max Grav 4=380(LC 19), 5=725(LC 1)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-9=-935/707, 2-9=-935/707, 2-10=-935/707, 10-11=-935/707, 3-11=-935/707,
 3-4=-520/646
 BOT CHORD 5-6=-386/422, 4-5=-348/373
 WEBS 2-5=-451/161, 3-5=-1575/1748

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 2-11-10, Interior(1) 2-11-10 to 8-1-4 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 347 lb uplift at joint 4 and 417 lb uplift at joint 5.
- *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with Mitek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPI® Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

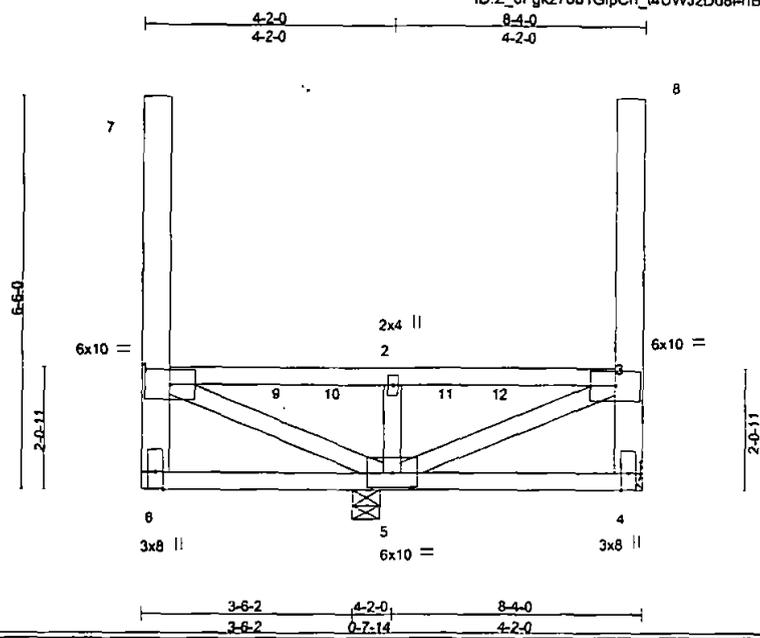


7777 Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss T18	Truss Type FLAT	Qty 1	Ply 1	Tony Abbar	R50761231
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Foxworth Galbreith Lumber Co, Yuma, Az 85365

Job Reference (optional)
7.640 s Apr 19 2018 MITek Industries, Inc. Tue Jun 06 10:48:27 2017 Page 1
ID:Z_OFgkz7ob1GlpCh_4UWJzDd8l-rfBwJcE110lhrCu2YJ_QG6Arx7ISF_XR79VuABz97KI



Scale = 1:37.1

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 40.0	2-0-0	TC 0.59	in (loc) l/defl L/d	MT20	197/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.15	Vert(LL) -0.01 4-5 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.49	Vert(TL) -0.01 4-5 >999 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 4 n/a n/a		
	Code IRC2012/TPI2007			Weight: 51 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF No.2 "Except"
6-7,4-8: 2x6 SPF 1650F 1.5E

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-8-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-6-11 oc bracing.

REACTIONS. (lb/size) 4=0/Mechanical, 5=945/0-5-8
Max Horz 5=-451(LC 10)
Max Uplift 4=-411(LC 9), 5=-543(LC 8)
Max Grav 4=411(LC 8), 5=945(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-9=-987/827, 9-10=-987/827, 2-10=-987/827, 2-11=-987/827, 11-12=-987/827,
3-12=-987/827, 3-4=-741/779
BOT CHORD 5-6=-370/403, 4-5=-350/372
WEBS 2-5=-423/152, 1-5=-510/175, 3-5=-1588/1831

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 8-1-4 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Refer to girder(s) for truss connections.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 411 lb uplift at joint 4 and 543 lb uplift at joint 5.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

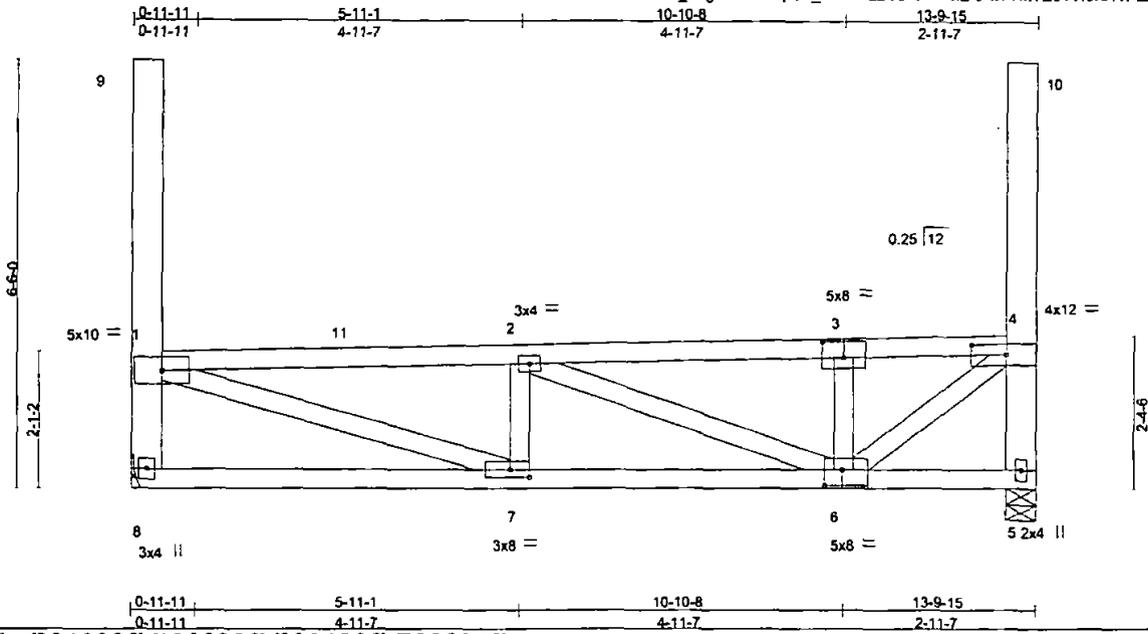


7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss T19	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761232
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Foxworth Galbraith Lumber Co., Yuma, Az 85385

Job Reference (optional)
7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:28 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8i-JrlVwxEfoKIYTMTE61VfoK1?PzL_QLbEpERjdz9?KH



Scale = 1:34.1

Plate Offsets (X,Y) - [3:0-4-0,0-3-0], [4:0-6-9,0-2-0], [6:0-3-4,0-3-0], [7:0-3-8,0-1-8]

LOADING (psf)	SPACING-	CSL	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	2-0-0	TC 0.57	Vort(LL)	-0.06	6-7	>999	MT20	197/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.49	Vert(TL)	-0.11	6-7	>999		
BCLL 0.0	Lumber DOL 1.00	WB 0.45	Horz(TL)	0.02	5	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix)						
	Code IRC2012/TPI2007						Weight: 72 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SPF No.2
 BOT CHORD 2x4 SPF No.2
 WEBS 2x4 SPF No.2 *Except*
 8-9,5-10: 2x6 SPF 1650F 1.5E

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-9-14 oc purtins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-10-7 oc bracing.

REACTIONS. (lb/size) 8=802/Mechanical, 5=802/0-5-8
 Max Horz 8=447(LC 9)
 Max Uplift 8=231(LC 8), 5=115(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-8=-741/325, 1-11=-1519/531, 2-11=-1513/532, 2-3=-957/444, 3-4=-957/448,
 4-5=-777/317
 BOT CHORD 7-8=-1009/1029, 6-7=-578/1513, 5-6=-277/261
 WEBS 2-7=-339/296, 3-6=-397/130, 1-7=-759/1463, 2-6=-641/552, 4-6=-408/1158

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCCL=6.0psf; BCCL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 4-4-4 to 7-4-4, Interior(1) 7-4-4 to 17-8-11 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 8 and 115 lb uplift at joint 5.
- *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

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TTTT Greenback Lane
 Suite 109
 Citrus Heights, CA 95610

Job 00416-17Y	Truss T20	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761233
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

Job Reference (optional)
7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:29 2017 Page 1
ID:Z_OFgkz7ob1GlpCh_4UWJzDd8i-n2JhjHFIPd?P5W2Rgk0uLXFAapMjJyKST_F4z9?KG

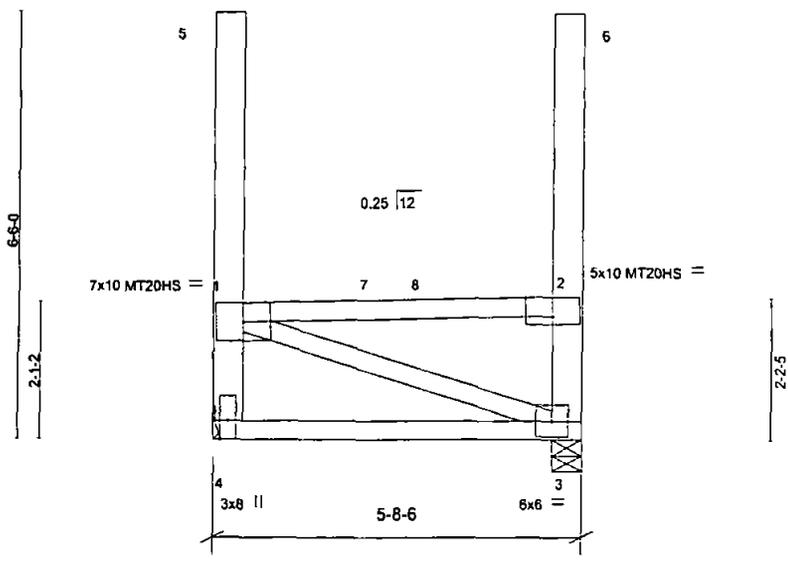


Plate Offsets (X,Y) [2:0-4-15,Edge]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	2-0-0	TC 0.65	Vert(LL)	-0.05	3-4	>999	MT20	197/144
TCDL 10.0	Plate Grip DOL 1.00	BC 0.31	Vert(TL)	-0.11	3-4	>556	MT20HS	148/108
BCLL 0.0	Lumber DOL 1.00	WB 0.77	Horz(TL)	0.01	3	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix)						
	Code IRC2012/TPI2007						Weight: 40 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SPF No.2
 BOT CHORD 2x4 SPF No.2
 WEBS 2x6 SPF 1650F 1.5E *Except*
 1-3: 2x4 SPF No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-4-2 oc bracing.

REACTIONS.

(lb/size) 4=314/Mechanical, 3=314/0-5-8
 Max Horz 4=449(LC 9)
 Max Uplift 4=350(LC 8), 3=306(LC 9)
 Max Grav 4=471(LC 20), 3=435(LC 19)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-4=-503/713, 1-7=-774/666, 7-8=-773/667, 2-8=-773/667, 2-3=-259/207
 BOT CHORD 3-4=-1175/1203
 WEBS 1-3=-1567/1568

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCCL=6.0psf; BCCL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 4-4-4 to 7-4-4, Interior(1) 7-4-4 to 9-7-2 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 350 lb uplift at joint 4 and 306 lb uplift at joint 3.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Expires: 3-31-2019
June 7, 2017

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7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss TGE	Truss Type GABLE	Qty 1	Ply 1	Tony Abbott	R50761234
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

Job Reference (optional)
7,640 s Nov 10 2015 MITek Industries, Inc. Wed Jun 07 15:13:25 2017 Page 1
ID:Z_0Fgkz7ob1GlpCh_14UWJzDdBi-6KossztS7DTFn8xGtx_MKsrUGbQgmd1IXq2yAz8e6O

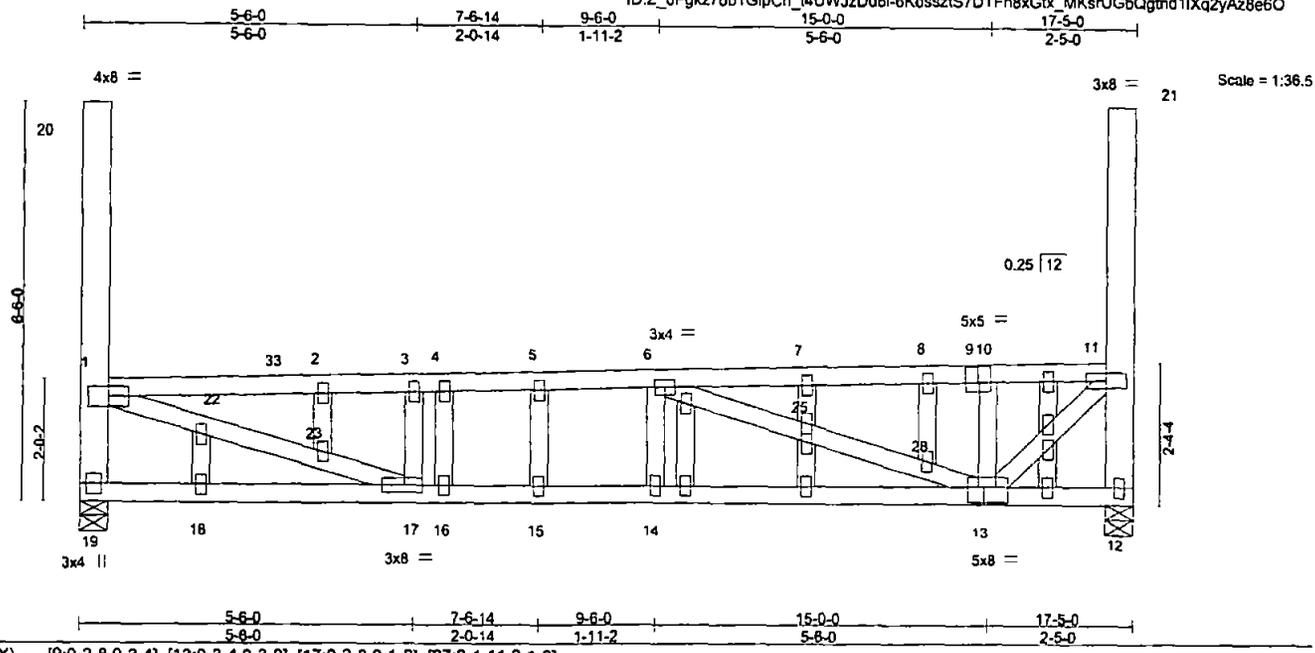


Plate Offsets (X,Y) - [9:0-2-8,0-3-4], [13:0-3-4,0-3-0], [17:0-3-8,0-1-8], [27:0-1-11,0-1-0]

LOADING (psf)	SPACING-	CSL	DEFL	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.36	in (loc) l/defl L/d	MT20	197/144
TCDL 10.0	Plato Grip DOL 1.00	BC 0.55	Vert(LL) -0.12 13-14 >999 360		
BCLL 0.0	Lumber DOL 1.00	WB 0.64	Vert(TL) -0.21 13-14 >955 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.03 12 n/a n/a		
	Code IRC2012/TPI2007			Weight: 94 lb	FT = 20%

LUMBER-	BRACING-	
TOP CHORD 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 4-4-3 oc purtins, except end verticals.
BOT CHORD 2x4 SPF No.2	BOT CHORD	Rigid ceiling directly applied or 6-10-5 oc bracing.
WEBS 2x4 SPF No.2 *Except*		
19-20,12-21: 2x6 SPF 1650F 1.5E		
OTHERS 2x4 SPF No.2		

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 19=678/0-5-8 (min. 0-1-8), 12=678/0-5-8 (min. 0-1-8)
Max Horz 19=300(LC 9)
Max Uplift 19=157(LC 8), 12=94(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-19=623/203, 1-33=1480/423, 2-33=1476/424, 2-3=1476/425, 3-4=1481/426.
 4-5=1480/426, 5-6=1478/427, 6-7=756/361, 7-8=754/362, 8-9=752/361,
 9-10=751/361, 10-11=762/369, 11-12=683/166
BOT CHORD 18-19=690/687, 17-18=690/687, 16-17=404/1477, 15-16=404/1477, 14-15=404/1477,
 13-14=404/1477
WEBS 10-13=258/152, 1-22=567/1459, 22-23=571/1469, 17-23=582/1458, 6-25=781/465,
 25-28=803/456, 13-28=819/477, 11-13=163/991, 3-17=356/201

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 17-2-4 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33, plate grip DOL=1.33
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 157 lb uplift at joint 19 and 94 lb uplift at joint 12.
 - This truss is designed in accordance with the 2012 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



Expires: 3-31-2019
June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



7777 Greenback Lane
Suite 109
Citrus Heights, CA 95610

Job 00416-17Y	Truss TGR	Truss Type SLOPING FLAT	Qty 1	Ply 1	Tony Abbott	R50761235
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Foxworth Galbraith Lumber Co, Yuma, Az 85365

Job Reference (optional)
7.640 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:35 2017 Page 1
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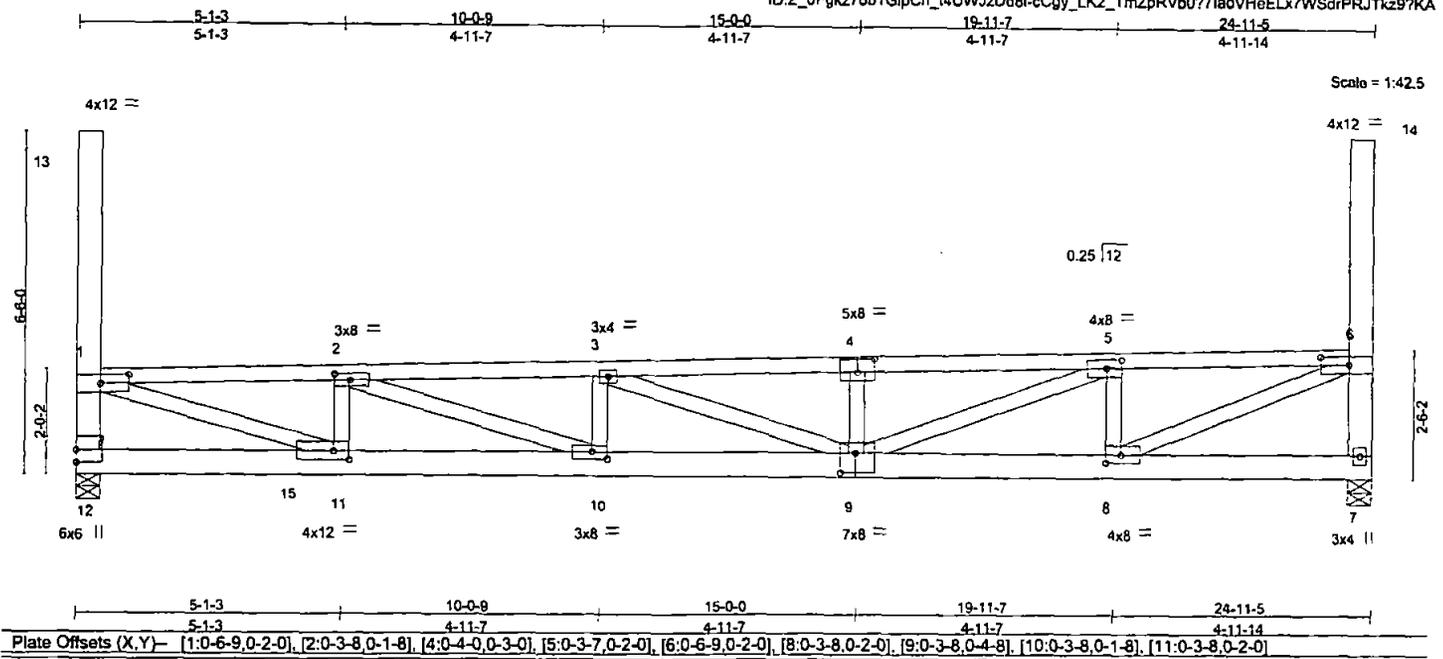


Plate Offsets (X,Y)	[1:0-6-9,0-2-0], [2:0-3-8,0-1-8], [4:0-4-0,0-3-0], [5:0-3-7,0-2-0], [6:0-6-9,0-2-0], [8:0-3-8,0-2-0], [9:0-3-8,0-4-8], [10:0-3-8,0-1-8], [11:0-3-8,0-2-0]							
LOADING (psf)	SPACING- 1-4-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.36	Vert(LL)	-0.26	9-10	>999	MT20	197/144
TCDL 10.0	Lumber DOL 1.00	BC 0.55	Vert(TL)	-0.46	9-10	>639		
BCLL 0.0	Rep Stress Incr NO	WB 0.81	Horz(TL)	0.05	7	n/a		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)					Weight: 128 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SPF 2100F 1.8E
BOT CHORD 2x6 SPF 1650F 1.5E
WEBS 2x4 SPF No.2 *Except*
12-13,7-14: 2x6 SPF 1650F 1.5E

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-9-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 12=1683/0-5-8, 7=1040/0-5-8
Max Horz 12=297(LC 5)
Max Uplift 12=367(LC 4), 7=154(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-12=-1144/258, 1-2=-2996/742, 2-3=-3631/784, 3-4=-3289/705, 4-5=-3292/708,
5-6=-2029/503, 6-7=-985/161
BOT CHORD 12-15=-465/598, 11-15=-465/598, 10-11=-634/2992, 9-10=-673/3627, 8-9=-384/2025
WEBS 2-11=-553/152, 4-9=-323/92, 5-8=-829/184, 1-11=-632/2877, 2-10=-268/672,
3-9=-402/157, 5-9=-224/1366, 6-8=-325/2137

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10: Vult=115mph (3-second gust) V(IRC2012)=91mph: TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) automatic zone; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 367 lb uplift at joint 12 and 154 lb uplift at joint 7.
 - Girdar carries tie-in span(s): 8-4-0 from 0-0-0 to 4-1-8
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 12-15=-210(B=-196), 7-15=-13, 1-6=-67

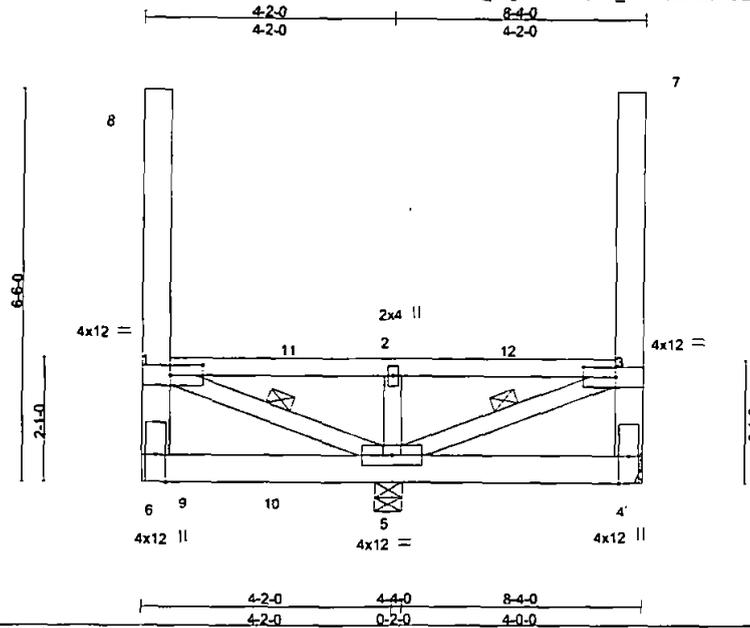


Expires: 3-31-2019
June 7, 2017

Job	Truss	Truss Type	Qty	Ply	Tony Abbott	R50761236
00416-17Y	TGR2	FLAT	1	1		

Foxworth Galbreath Lumber Co. Yuma, Az 85385

7,840 s Apr 19 2016 MiTek Industries, Inc. Tue Jun 06 10:48:36 2017 Page 1
 ID:Z_0Fgkz7ob1GlpCh_t4UWJzDd8i-4OEKChLhnuQQb4naieX702MAdDsyBm33As7Az97K9



Scale = 1:37.1

Plate Offsets (X,Y)-- [1:0-6-8,0-2-0], [3:0-6-8,0-2-0]

LOADING (psf)	SPACING-	CSL	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.75	Vert(LL)	0.02	4-5	>999	MT20	137/130
TCDL 10.0	Lumber DOL 1.00	BC 0.30	Vert(TL)	0.03	4-5	>999		
BCLL 0.0	Rep Stress Incr NO	WB 0.84	Horz(TL)	0.00	4	n/a		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)					Weight: 60 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SPF 2100F 1.8E³
 BOT CHORD 1.5X5.5 LP LVL 3100Fb 2.0E
 WEBS 2x4 HF/SPF Stud/Std *Except*
 4-7,6-8: 2x6 SPF 1650F 1.5E

BRACING-

TOP CHORD Structural wood sheathing directly applied or 8-4-0 oc purins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 3-5, 1-5

REACTIONS.

(lb/size) 4=523/Mechanical, 5=2167/0-5-8
 Max Horz 5=447(LC 26)
 Max Uplift 4=1380(LC 34), 5=860(LC 4)
 Max Grav 4=541(LC 4), 5=3623(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=481/1240, 1-11=820/2690, 2-11=820/2690, 2-12=820/2690, 3-12=820/2690,
 1-6=-294/1046
 WEBS 2-5=-553/184, 3-5=-3178/1270, 1-5=-2922/826

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1380 lb uplift at joint 4 and 860 lb uplift at joint 5.
- 8) Girder carries tie-in span(s): 13-10-12 from 0-0-0 to 2-2-8
- 9) *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.
- 10) This truss has large uplift reaction(s) from gravity load case(s). Proper connection is required to secure truss against upward movement at the bearings. Building designer must provide for uplift reactions indicated.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 802 lb down and 231 lb up at -9-1-12, and 786 lb down and 247 lb up at 0-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 6-10=-373(B=-353), 4-10=-20, 1-3=-100



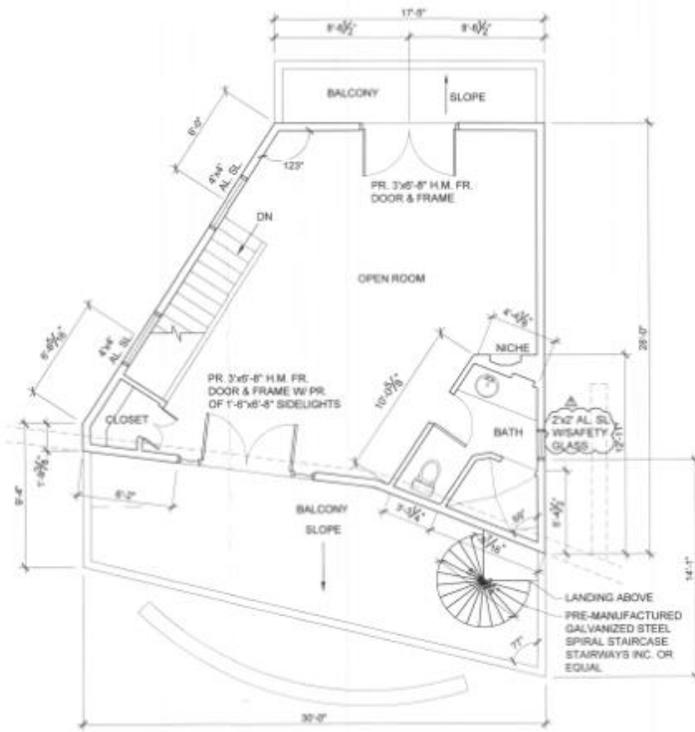
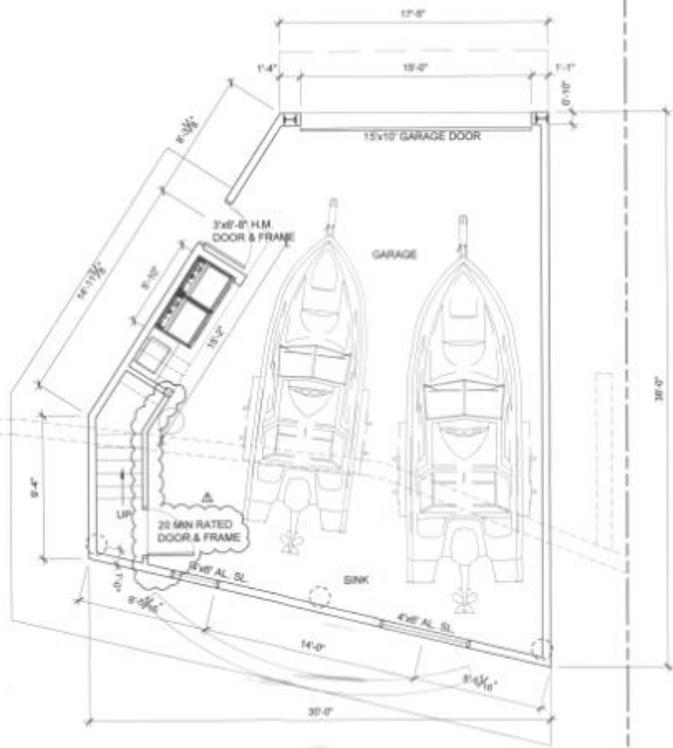
Expires: 3-31-2019
 June 7, 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/02/2015 BEFORE USE.

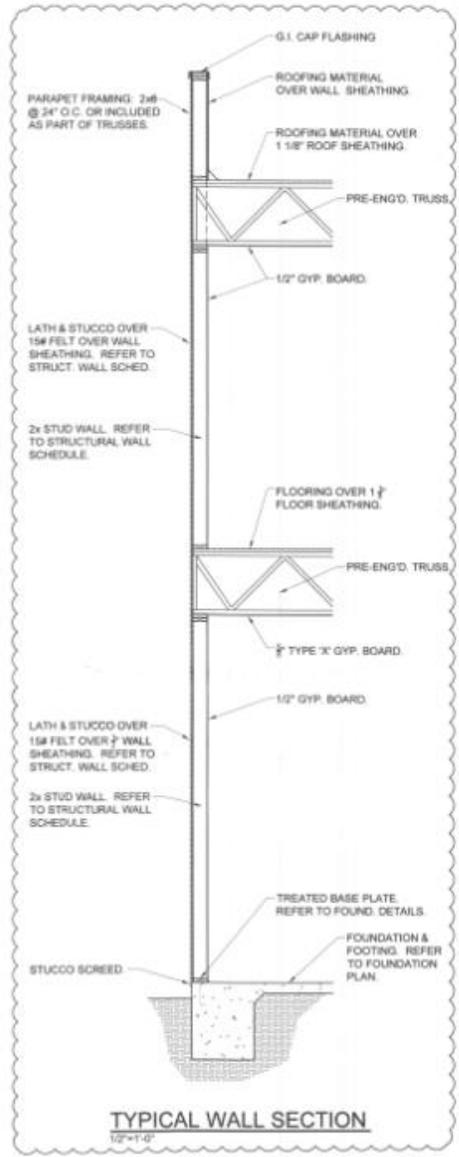
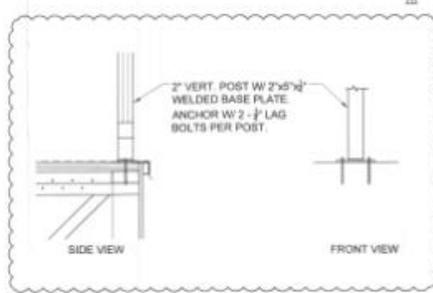
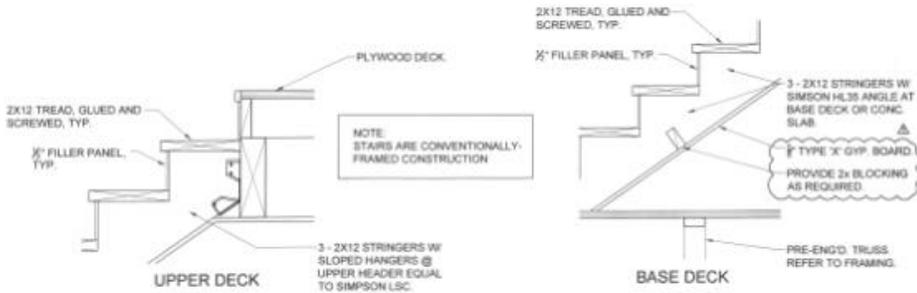
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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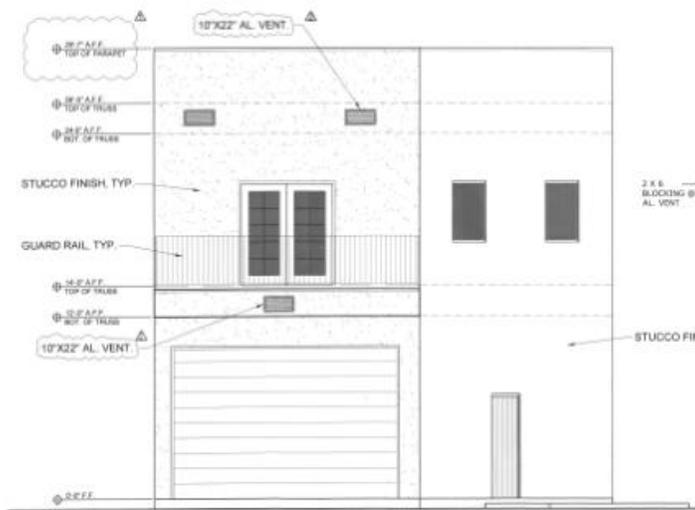


NOTES:
ALL CEILINGS BETWEEN GARAGE AND HABITABLE SPACE TO RECEIVE 5/8" TYPE 'X' GYPSUM BOARD.
ALL UNDER SIDE OF INTERIOR STAIRS TO RECEIVE 5/8" TYPE 'X' GYPSUM BOARD FOR PROTECTION.

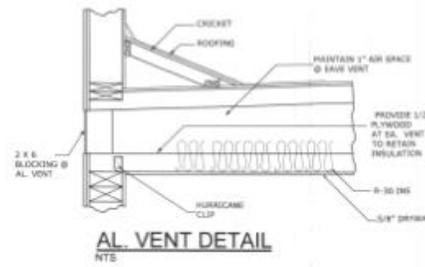


OSMAN ENGINEERING, INC. 183 E 24TH STREET, SUITE 6 YUMA, AZ 85364 Tel: (928) 314-1737 FAX: (928) 314-1738
FLOOR PLANS
NEW GARAGE TONY ABBOTT RESIDENCE 10343 E NORTH MARTINEZ LAKE ROAD YUMA, ARIZONA
I.D. 8090917 R.L. SD 2-17 SCALE AS SHOWN SHEET A2 OF 12

Yuma County Flood Control District
Flood Control Plan Approved
 Date: 11/11/17
 PP 17-0007
 PPL17-0.75

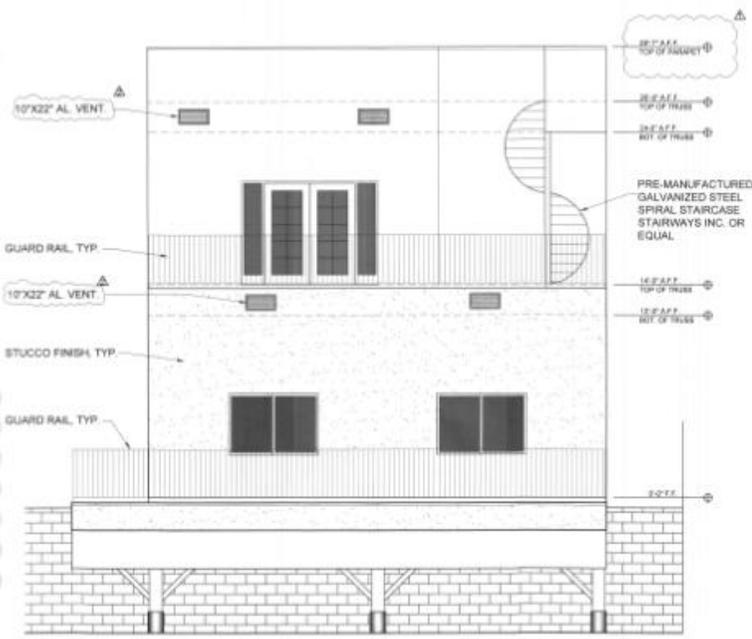


NORTH ELEVATION
1/4"=1'-0"

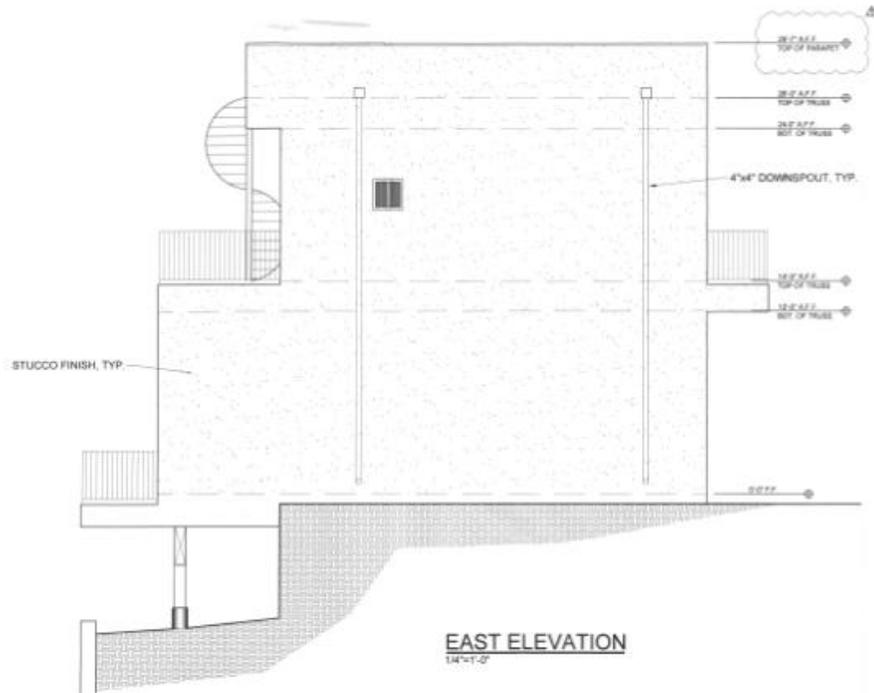


VENTILATION CALCULATIONS

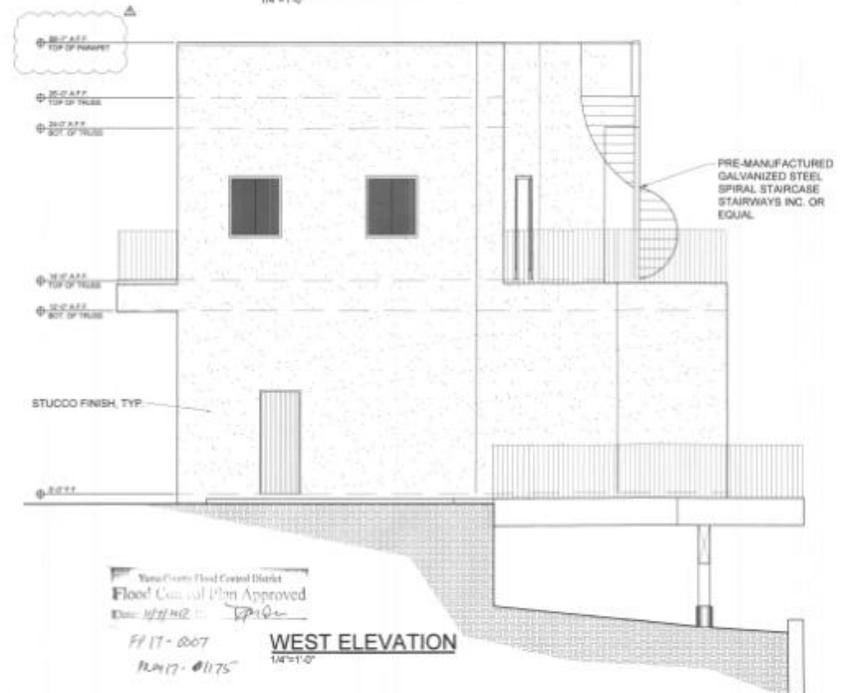
TOTAL ROOFED AREA:	1458.28 S.F.
REQUIRED AREA :	2628.74 X 1/150 = 9.72 S.F.
PROVIDED AREA:	(7)10"X22" GBL VENT 1.51 SF EA. 10.57 S.F.
TOTAL :	10.57 S.F.



SOUTH ELEVATION
1/4"=1'-0"



EAST ELEVATION
1/4"=1'-0"



WEST ELEVATION
1/4"=1'-0"

Yuma County Flood Control District
Flood Control Plan Approved
 Date: 11/17/07
 11/17-007
 12/17-0175

NO.	DATE	DESCRIPTION
1	11/07/07	CONTRACT SETS 1 & 2017
2	11/07/07	CONTRACTS 1 & 2017



OSMAN ENGINEERING SERVICES PLLC
 183 E 24TH STREET, SUITE 6
 YUMA, AZ 85364
 Tel: (928) 314-1737 FAX: (928) 314-1738

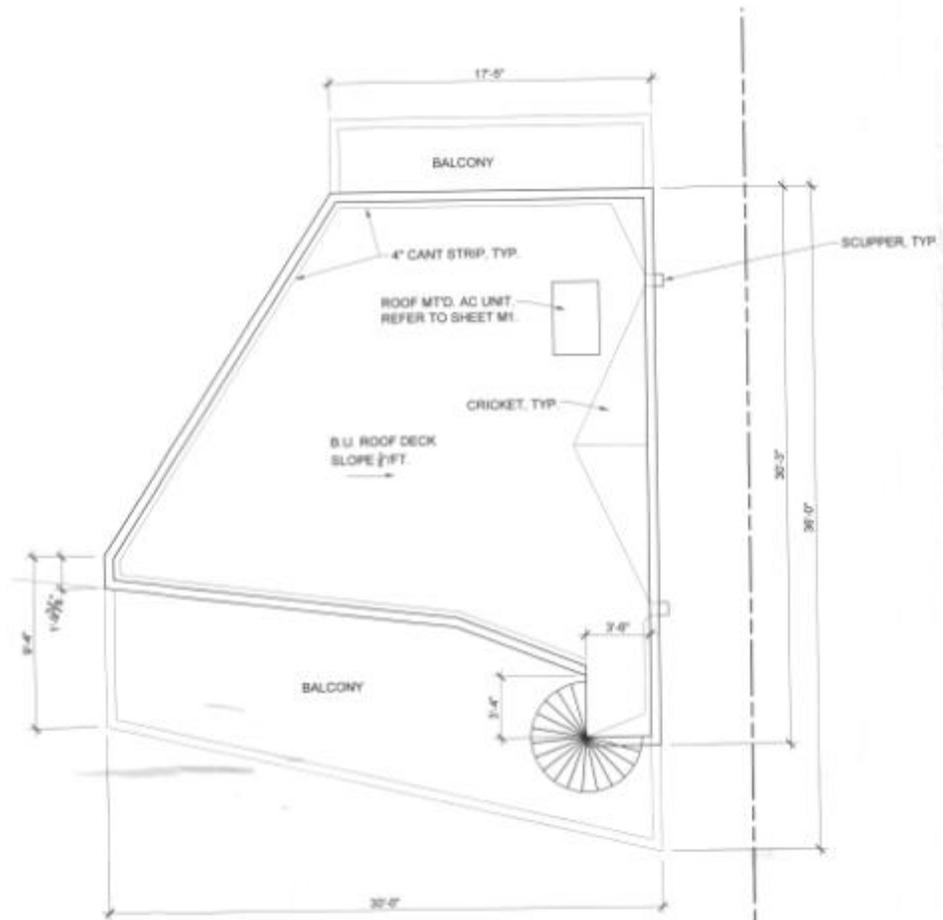
EXTERIOR ELEVATIONS

REV 02 2017

BY: **NEW GARAGE**
TONY ABBOTT RESIDENCE
10343 E NORTH MARTINEZ LAKE ROAD
YUMA, ARIZONA

NO.	DATE	DESCRIPTION
1	11/17/07	3088017
2	11/17/07	SD 2-17
3	11/17/07	

SCALE: AS SHOWN
 SHEET **A3** OF **12**



ROOF PLAN
1/8"=1'-0"

